DRAINAGE ANALYSIS

Prepared for Joanna Hills LLC Joanna Hills Estates

Joanna Road Avon, Massachusetts 02322

August 08, 2019 Revised thru October 27, 2020



MBL Land Development & Permitting Corp.
770 Broadway Suite 6
Raynham, MA 02767
Phone 508.297.2746
Fax 508.297.2756

Email: info@MBLLandDevelopment.com Website: www.MBLLandDevelopment.com

TABLE OF CONTENTS

SEC	CTION	INTRODUCTION		
СН	ECKLIS	ST FOR STORMWATER REPORT		
1.	INTRO	DUCTION	1-1	
2.	SITE DI	ESCRIPTION	2-1	
	2.1 2.2 2.3 2.4	Proposed ConditionsSoils	2-1 2-1	
3.	DRAINA	AGE CALCULATIONS	3-1	
	3.1 3.1.1 3.1.2 3.1.3 3.1.4 3.2	Design Points Existing Hydrology Proposed Hydrology		
4.	BEST N	MANAGEMENT PRACTICES	4-1	
	4.1 4.2 4.3 4.4	Deep Sump Hooded Catch Basins	4-1 4-1	
5.	STORM	IWATER MANAGEMENT STANDARDS COMPLIANCE		
6.	LOW IN	MPACT DESIGN (LID) MEASURES	6-1	
7.		.USION		
		LIST OF TABLES		
TA	BLE		PAGE NO.	
Tak	ole 1: Ta	ble Design Rainfall Data	3-1	
Tab	ole 2: Pre	e-Development Peak Discharge Rates	3-2	
Tab	ole 3: Po	st-Development Peak Discharge Rates	3-3	

LIST OF FIGURES

Figure 1: Aerial Map

Figure 2: USGS Topographic Map

Figure 3: Flood Insurance Rate Map

Figure 4: Natural Heritage Map

Figure 5: Critical Areas

Figure 6: Soils Map

Figure 7: Existing Conditions Drainage Map

Figure 8: Proposed Conditions Drainage Map

APPENDICES

APPENDIX A: HYDROLOGIC ANALYSIS

APPENDIX B: HYDRAULIC ANALYSIS

PIPE SIZING CALCULATIONS

APPENDIX C: GROUNDWATER RECHARGE CALCULATIONS & 72-HR

DRAWDOWN CALCULATIONS

APPENDIX D: REQUIRED WATER QUALITY VOLUME & TSS REMOVAL

CALCULATIONS

APPENDIX E: CONSTRUCTION PERIOD POLLUTION PREVENTION PLAN

APPENDIX F: LONG TERM POLLUTION PREVENTION PLAN

APPENDIX G: OPERATION & MAINTENANCE PLAN

APPENDIX H: ILLICIT DISCHARGE COMPLIANCE STATEMENT

APPENDIX I: GROUNDWATER MOUNDING ANALYSIS

APPENDIX J: SOIL LOGS & SIEVE ANALYSIS RESULTS

APPENDIX K: RIPRAP SIZING CALCULATIONS

APPENDIX L: TURF MANAGEMENT PLAN



Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.





A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the Massachusetts Stormwater Handbook. The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals. This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8²
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

¹ The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

² For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature



10/27/2020

Signature and Date

Checklist

	expject Type: Is the application for new development, redevelopment, or a mix of new and evelopment?
\boxtimes	New development
	Redevelopment
	Mix of New Development and Redevelopment



Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

\boxtimes	No disturbance to any Wetland Resource Areas
	Site Design Practices (e.g. clustered development, reduced frontage setbacks)
	Reduced Impervious Area (Redevelopment Only)
	Minimizing disturbance to existing trees and shrubs
	LID Site Design Credit Requested:
	Credit 1
	Credit 2
	☐ Credit 3
	Use of "country drainage" versus curb and gutter conveyance and pipe
	Bioretention Cells (includes Rain Gardens)
	Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
	Treebox Filter
	Water Quality Swale
	Grass Channel
	Green Roof
	Other (describe):
Sta	ndard 1: No New Untreated Discharges
\boxtimes	No new untreated discharges
\boxtimes	Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
\boxtimes	Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



Massachusetts Department of Environmental ProtectionBureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

Cł	necklist (continued)				
Sta	ndard 2: Peak Rate Attenuation				
	Standard 2 waiver requested because the project is located in land subject to coastal storm flow				
\boxtimes	and stormwater discharge is to a wetland subject to coastal flooding. Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.				
	Calculations provided to show that post-development peak discharge rates do not exceed pre- development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24- hour storm.				
Sta	indard 3: Recharge				
\boxtimes	Soil Analysis provided.				
\boxtimes	Required Recharge Volume calculation provided.				
	Required Recharge volume reduced through use of the LID site Design Credits.				
\boxtimes	Sizing the infiltration, BMPs is based on the following method: Check the method used.				
	☐ Static ☐ Dynamic Field¹				
\boxtimes	Runoff from all impervious areas at the site discharging to the infiltration BMP.				
	Runoff from all impervious areas at the site is <i>not</i> discharging to the infiltration BMP and calculation are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.				
\boxtimes	Recharge BMPs have been sized to infiltrate the Required Recharge Volume.				
	Recharge BMPs have been sized to infiltrate the Required Recharge Volume <i>only</i> to the maximum extent practicable for the following reason:				
	☐ Site is comprised solely of C and D soils and/or bedrock at the land surface				
	M.G.L. c. 21E sites pursuant to 310 CMR 40.0000				
	☐ Solid Waste Landfill pursuant to 310 CMR 19.000				
	Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.				
\boxtimes	Calculations showing that the infiltration BMPs will drain in 72 hours are provided.				
	Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.				

¹ 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

Checklist (continued)

Standard 3: Recharge (continued)

- ☐ The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;
- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management system;
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- 🔘 A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
- Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for

	calculating the water quality volume are included, and discharge:
	is within the Zone II or Interim Wellhead Protection Area
	is near or to other critical areas
	is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
	involves runoff from land uses with higher potential pollutant loads.
	The Required Water Quality Volume is reduced through use of the LID site Design Credits.
\boxtimes	Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if



Critical areas and BMPs are identified in the Stormwater Report.

Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

Checklist (continued) Standard 4: Water Quality (continued) The BMP is sized (and calculations provided) based on: The ½" or 1" Water Quality Volume or The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume. ☐ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs. A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided. Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs) ☐ The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report. The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted *prior* to the discharge of stormwater to the post-construction stormwater BMPs. The NPDES Multi-Sector General Permit does *not* cover the land use. LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan. All exposure has been eliminated. All exposure has *not* been eliminated and all BMPs selected are on MassDEP LUHPPL list. The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent. Standard 6: Critical Areas The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.



Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

Checklist (continued)

ent practicable
The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:
☐ Limited Project
 Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area. Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
☐ Bike Path and/or Foot Path
Redevelopment Project
Redevelopment portion of mix of new and redevelopment.
Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.
The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.
- A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

Checklist (continued)

	Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)				
	The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has <i>not</i> been included in the Stormwater Report but will be submitted <i>before</i> land disturbance begins.				
	The project is <i>not</i> covered by a NPDES Construction General Permit.				
	The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.				
	The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.				
Sta	ndard 9: Operation and Maintenance Plan				
	The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:				
	Name of the stormwater management system owners;				
	□ Party responsible for operation and maintenance;				
	Schedule for implementation of routine and non-routine maintenance tasks;				
	☐ Plan showing the location of all stormwater BMPs maintenance access areas;				
	☐ Description and delineation of public safety features;				
	Operation and Maintenance Log Form.				
	The responsible party is not the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:				
	A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;				
	A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.				
Sta	andard 10: Prohibition of Illicit Discharges				
\boxtimes	The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;				
\boxtimes	An Illicit Discharge Compliance Statement is attached;				
	NO Illicit Discharge Compliance Statement is attached but will be submitted <i>prior to</i> the discharge of any stormwater to post-construction BMPs.				

1. INTRODUCTION

MBL Land Development and Permitting Corporation (MBL) has prepared this Drainage Analysis for the proposed development of the project site located at Joanna Road, Avon, Massachusetts. The purpose of the analysis is to quantitatively understand the impacts of the proposed site development on the existing hydrologic conditions and to mitigate these impacts through the implementation of a proposed stormwater management system that utilizes best management practices (BMPs) and is supported by an Operations & Maintenance Plan as well as a Long Term Pollution Prevention Plan.

2. SITE DESCRIPTION

2.1 EXISTING CONDITIONS

The project is located at the end of Joanna Road, off of Page Street, in Avon, Massachusetts. The site is referenced as Block 3, Plot 15 on Assessors Map C7 and contains a total of 29.37 acres. The existing site consists woodlands and forested wetland areas. A minority of the site is located within a Flood Zone A, and the majority of the site is located within a Flood Zone X, as shown on FEMA flood insurance FIRM Map No. 25021C0218E, having an effective date of July 17, 2012. The site is not located within an Estimated Habitat of Rare Wildlife, Priority Habitat of Rare Species or near a Certified Vernal Pool according to the Massachusetts Natural Heritage Atlas, 14th Edition, effective August 2017. The site is not located within an ACEC, Outstanding Resource Water or any other Critical Area. Please refer to Figures 1-6.

2.2 PROPOSED CONDITIONS

The proposed project is a Chapter 40B development containing a total of 76 proposed units with associated site access, parking, grading, drainage and utilities including on-site subsurface disposal systems. These units are comprised of 9 single family dwellings, 11 duplexes, one 7-unit condominium building, one 10-unit condominium building and two 14-unit condominium buildings.

2.3 SOILS

The Soil Conservation Survey for Norfolk County indicates that the site is located within Soil Map Units 420B: Canton Fine Sandy Loam (Hydrologic Soil Group (HSG) B), 424D: Canton Fine Sandy Loam (HSG A), 424B: Canton Fine Sandy Loam (HSG A), 302C: Montauk Fine Sandy Loam (HSG C), 302B: Montauk Fine Sandy Loam (HSG C), and 71B: Ridgebury Fine Sandy Loam (HSG D). Please refer to Figure 6 – Soils Map.

2.4 SITE TOPOGRAPHY

The existing topography slopes easterly from Page Street down Joanna Road and into the site. Within the site, stormwater runoff splits from the center of the site and flows either northerly or southerly towards the bordering vegetated wetlands on-site. Slopes on-site vary anywhere from 1 to 25%.

3. DRAINAGE CALCULATIONS

To mitigate the quality and quantity of stormwater runoff discharging from the site, a stormwater management system has been designed to collect, treat and control flows leaving the site. The proposed stormwater management system consists a series of deep sump hooded catch basins, pipes and manholes, which will collect and convey stormwater runoff to proposed water quality units and vegetated infiltration basins. The proposed infiltration basins have been designed to store the required recharge volume and have riprap overflow spillways. Also where feasible roof recharge systems were added to recharge roof runoff. These system consist of Cultec 330XLHD Chambers designed to recharge to entire 100-year storm. The stormwater management system has been designed to reduce peak flow rates from existing to the proposed conditions for all storm events. From an environmentally sensitive perspective, the aforementioned measures result in a design that promotes on-site groundwater recharge while preserving the natural hydrologic conditions.

A detailed hydrologic analysis of the stormwater management system was completed to evaluate its performance and document compliance with the Massachusetts Stormwater Standards. MBL has prepared the following drainage system calculations for the proposed site development.

3.1 HYDROLOGIC ANALYSIS

The Soil Conservation Service (SCS) Unit Hydrograph methodology was utilized to develop a hydrologic model of the site. MBL utilized HydroCAD Version 10.0 software, developed by HydroCAD Software Solutions, LLC to analyze the site hydrology. The program calculates peak rates of runoff and runoff volume based on selected rainfall events. Contributing watershed areas were identified and soils, surface cover, watershed slope, and flow paths were evaluated to develop the necessary HydroCAD model input parameters. A minimum Time of Concentration (Tc) of (6) minutes was used in the calculations.

Drainage calculations were performed for the Existing and Proposed Conditions for the 24-hour 2, 10, 25 and 100-year Type III storm events. The total rainfall for each of the storm events was based upon TP-40 data. The total rainfall values used in the hydrologic modeling are shown in the following table:

Table 1: Table Design Rainfall Data			
2-year, 24-hour storm	10-year, 24-hour storm	25-year, 24-hour storm	100-year, 24-hour storm
3.20 inches	4.70 inches	5.50 inches	6.70 inches

3.1.1 Design Points

To compare the difference between the existing and proposed peak flow rates, the existing and proposed watershed areas were delineated. Design points for each watershed were determined by flow paths from the hydraulically most distance point of the watershed. These parameters were utilized to calculate the times of concentration which were modeled. The same Design Points were analyzed for both the existing and proposed conditions. For this project, the design points that were identified are as follows:

- Wetlands North
- Wetlands South
- Wetlands South East
- ILSF

3.1.2 Existing Hydrology

The existing site was broken into watershed areas, each discharging to one of the design points. The existing watershed areas are shown on the attached Figure 7 entitled, "Existing Conditions Drainage Map". The hydrographs for each watershed were generated to develop the peak discharge rates for the 24-hour, 2, 10, 25 and 100-year storm events for the existing site conditions.

3.1.3 Proposed Hydrology

The site was broken into watersheds for the proposed analysis, each discharging to one of the same design points. The proposed watershed areas, are shown on the attached Figure 8 entitled "Proposed Conditions Drainage Map". The hydrographs for each watershed were generated and routed through the proposed BMPs to develop the peak discharge rates for the 24-hour 2, 10, 25 and 100-year storm events for the proposed site conditions.

3.1.4 Peak Discharge Rates

Tables 2 and 3 below summarizes the Pre- and Post-Development peak discharge rates for each Design Point. As depicted in the tables, the Post-Development peak rate of discharge does not increase over Pre-Development peak rate for all storm events analyzed.

Table 2: Pre-Development Peak Discharge Rates				
Storm	Existing Peak Runoff (cfs)			
Frequency	Wetlands North	Wetlands South	ILSF	Wetlands Southeast
2	0.18	3.14	0.00	0.09
10	2.95	7.12	0.07	1.44
25	5.88	9.47	0.18	2.89
100	11.70	13.16	0.46	5.81

Table 3: Post-Development Peak Discharge Rates				
Storm	Proposed Peak Runoff (cfs)			
Frequency	Wetlands North	Wetlands South	ILSF	Wetlands Southeast
2	0.18	2.80	0.00	0.09
10	1.73	5.40	0.06	0.70
25	3.23	6.85	0.13	1.21
100	10.29	9.08	0.29	5.20

3.2 HYDRAULIC ANALYSIS

Portions of the stormwater closed (underground piping) drainage system discharging to the BMPs were designed to convey the 25-year storm event. Pipe capacity and peak discharge rates for the closed drainage system were calculated using a Manning's Formula spreadsheet.

The closed drainage system, as designed, is capable of conveying the design flow as calculated, as well as maintaining a design velocity of between 2.0 feet per second (fps) and 10.0 fps for pipe capacity full conditions. The closed drainage system analysis for the proposed system is depicted in Appendix B.

4. BEST MANAGEMENT PRACTICES

The Massachusetts Stormwater Standards requires 80% removal rate over an average annual basis, for Total Suspended Solids (TSS) contained in stormwater runoff. The water quality volume or "first flush" is defined as the volume obtained by multiplying one-half inch ($\frac{1}{2}$ ") times the impervious surface area of the contributing drainage area. Water quality volume calculations are provided in Appendix D. When this volume is incorporated into properly designed BMPs an 80% reduction of average annual TSS loading will result. The following Best Management Practices will be employed for the project.

4.1 DEEP SUMP HOODED CATCH BASINS

Deep sump hooded catch basins are proposed for pretreatment. 25% TSS credit has been taken for the deep sump hooded catch basins.

4.2 WATER QUALITY UNITS

Stormceptor STC 2400 Water Quality Units are proposed for TSS removal. Please see Appendix D for the manufacturer's TSS removal calculations for the proposed water quality units.

4.3 INFILTRATION BASINS

The proposed infiltration basins will achieve a TSS removal rate of 80% while promoting groundwater recharge on-site.

4.4 SUBSUFRACE RECHARGE CHAMBERS

Cultec 330XLHD Subsurface Recharge Chambers are proposed to recharge roof runoff for the 100-yr storm on-site.

5. STORMWATER MANAGEMENT STANDARDS COMPLIANCE

The proposed best management practices (BMPs) selection and their placement within the treatment train of the stormwater management system has been strategically planned and designed as prescribed by the Massachusetts Stormwater Management Handbook. The following addresses how the project complies with Standards 1-10 as set forth in the Massachusetts Stormwater Handbook:

Standard 1

No new stormwater conveyances (e.g. outfalls) may discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.

There are no new untreated discharges to or that will cause erosion in wetlands or waters of the Commonwealth. All stormwater runoff from impervious surfaces is designed to be collected and treated prior to discharging.

Standard 2

Stormwater management systems shall be designed so that the post-development peak discharge rates do not exceed pre-development peak discharge rates.

The stormwater management system has been designed so that the proposed peak discharge rates are less than the existing peak discharge rates for the 2-year, 10-year, 25-year and 100-year 24-hour storm events (see Tables 2 & 3 of this report). Supporting documentation such as HydroCAD reports and required computations are located in Appendix A.

Standard 3

Loss of annual recharge to groundwater shall be eliminated or minimized with environmentally sensitive site design, low impact development techniques, stormwater best management practices, and good operation and maintenance.

The proposed stormwater management system has been designed to collect stormwater runoff and recharge it back into the ground on-site. The soils on-site are conducive to recharge. Supporting documentation such as Required Recharge Volume Calculations can be found in Appendix C and Soil Logs can be found in Appendix J.

Standard 4

Stormwater management systems shall be designed to remove 80% of the average annual post-construction load of Total Suspended Solids (TSS).

The required removal of 80% TSS has been achieved by a series of BMP's including deep sump hooded catch basins, stormceptor water quality units and vegetated infiltration basins. Computations and documentation are provided in Appendix D.

Standard 5

For land uses with higher potential loads, source control and pollution prevention shall be implemented in accordance with the Massachusetts Stormwater Handbook to eliminate or reduce the discharge of stormwater runoff from such land uses to the maximum extent practicable.

The site is not a land use with higher potential pollutant load, per the regulations.

Standard 6

Stormwater discharges within the Zone II or Interim Wellhead Protection Area of a public water supply and stormwater discharges near or to any other critical area require the use of the specific source control and pollution prevention measures and the specific structural stormwater best management practices determined by the Department to be suitable for managing discharges to such areas, as provided in the Massachusetts Stormwater Handbook.

The site does not discharge to a Zone II, IPWA or other critical area.

Standard 7

A redevelopment project is required to meet the following Stormwater Management Standards only to the maximum extent practicable: Standard 2, Standard 3, and the pretreatment and structural stormwater best management practice requirements of Standards 4, 5, and 6. Existing Stormwater discharges shall comply with Standard 1 only to the maximum extent practicable. This project could be considered a redevelopment.

The site is not a redevelopment project.

Standard 8

A plan to control construction-related impacts, including erosion, sedimentation, and other pollutant sources during construction and land disturbance activities (construction period erosion, sedimentation, and pollution prevention plan) shall be developed and implemented.

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan is located in Appendix E.

Standard 9

A Long-Term Operation and Maintenance (O&M) Plan shall be developed and implemented to ensure that stormwater management systems function as designed.

A Long-Term Operation and Maintenance Plan (O&M Plan) for the site stormwater management facilities can be found in Appendix G and a Long-Term Pollution Prevention Plan (LPPP) is located in Appendix F.

Standard 10

All illicit discharges to the stormwater management system are prohibited.

An Illicit Discharge Compliance Statement has been provided in Appendix H.

6. LOW IMPACT DESIGN (LID) MEASURES

In accordance with the Massachusetts Stormwater Standards, Low Impact Design (LID) measures were evaluated and incorporated into the on-site drainage design where feasible.

No Disturbance to any Wetland Resource Areas

No proposed work is located within the existing BVW.

Site Design Practices

In order to minimize disturbance, multi-family condominium and duplex buildings are proposed instead of single-family homes where possible. This allows for housing units to be constructed in close proximity which reducing the length of the associated roadway.

Reduced Impervious Area

This LID measure does not apply because this site is not a redevelopment.

Minimizing Disturbance to Existing Trees and Shrubs

This LID measure could not be implemented due to the disturbance of the existing woods on-site.

Use of "Country Drainage" Versus Curb and Gutter Conveyance and Pipe

This LID measure could not be implemented due the project's reduced lot areas and close proximity to wetlands.

Bioretention Cells

This LID measure was not implemented because of the amount of fill that would be required to construct the Bioretention Cell as specified while also maintaining the necessary 2 feet of groundwater separation.

Constructed Stormwater Wetlands

This LID measure was not implemented due to the necessary area required to construct the Stormwater Wetlands.

Tree Box Filter

This LID measure was not implemented because there is no room for street trees within the proposed right-of-way for the project.

Water Quality Swale

This LID measure was not implemented because of the projects reduced lot areas and proximity to the wetlands and given the design requirements of water quality swales, there is no location on-site that is feasible for constructing a swale.

Grass Channel

This LID measure was not implemented because of the projects reduced lot areas and proximity to the wetlands and given the design requirements of water quality swales, there is no location on-site that is feasible for constructing a swale.

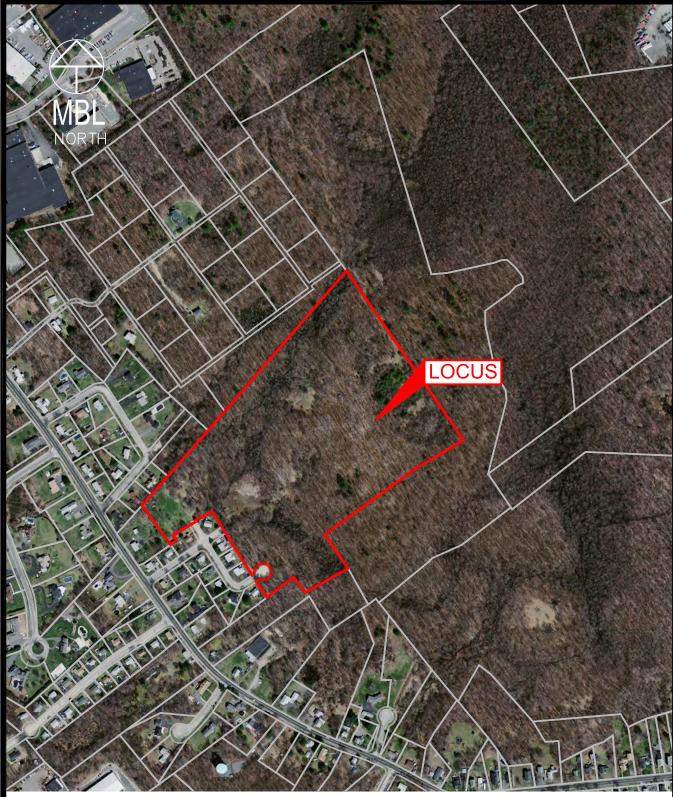
Green Roof

This LID measure was not implemented because the proposed single family, duplex, and multifamily buildings have designs that are not conducive to green roof construction.

7. CONCLUSION

The proposed site development will not increase the peak rate of runoff from the existing conditions for all modeled storm events. This has been accomplished by implementing BMPs that will enhance the quality of stormwater runoff while also introducing infiltration to a previously uncontrolled site. The proposed stormwater management system has been designed to meet all 10 of the Massachusetts Stormwater Management Standards.

Figure 1: Aerial Map



NOTE: INFORMATION ON THIS PLAN OBTAINED FROM MASSGIS USGS COLOR ORTHO IMAGERY 2008/2009.

MBL

LAND DEVELOPMENT & PERMITTING, CORP.
LAND DEVELOPMENT, TRANSPORTATION AND ENVIRONMENTAL SOLUTIONS
770 BROADWAY, SUITE No. 6
RAYNHAM, MA 02767
P.508.297.2746 F.508.297.2756
EMAIL.info@MBLLandDevelopment.com
WEB: www.MBLLand Development.com

AERIAL MAP

JOANNA HILLS ESTATES ASSESSORS MAP C7-3 & PLOT 15

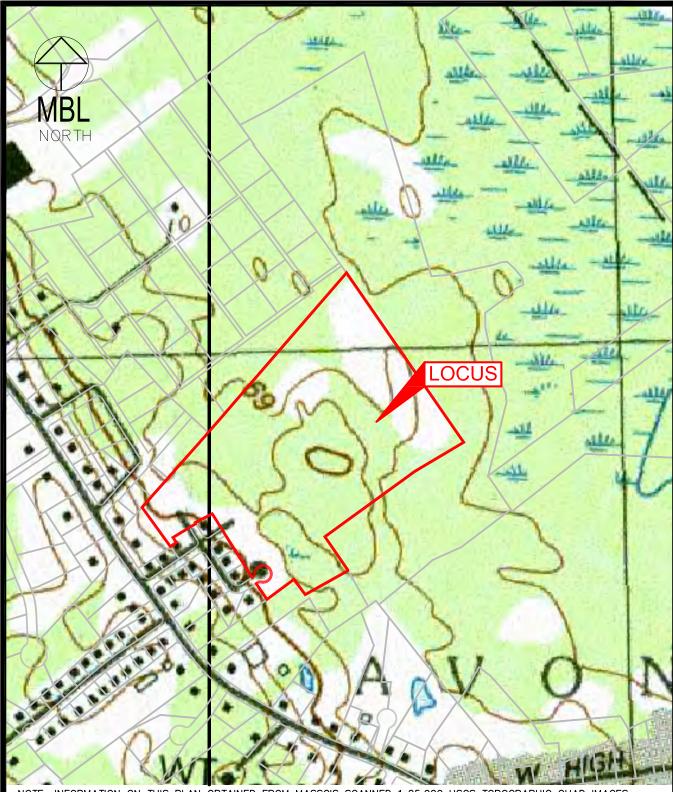
MASSACHUSETTS

AVON

PROJ. No: 2016-002

DATE: 8/8/2019 SCALE: 1"=500'

Figure 2: USGS Topographic Map



NOTE: INFORMATION ON THIS PLAN OBTAINED FROM MASSGIS SCANNED 1:25,000 USGS TOPOGRAPHIC QUAD IMAGES, JUNE 2001.

MBL

LAND DEVELOPMENT & PERMITTING, CORP.
LAND DEVELOPMENT. TRANSPORTATION AND ENVIRONMENTAL SOLUTIONS
770 BROADWAY, SUITE No. 6
RAYNHAM, MA 02767
P.508.297.2746 F.508.297.2756
EMAIL:info@MBLLandDevelopment.com
WEB: www.MBLLand Development.com

USGS TOPOGRAPHIC MAP

JOANNA HILLS ESTATES ASSESSORS MAP C7-3 & PLOT 15

AVON MASSACHUSETTS

PROJ. No: 2016-002
DATE: 8/8/2019

SCALE:

FIGURE 2

1"=500'

Figure 3: Flood Insurance Rate Map



MIQI

LAND DEVELOPMENT & PERMITTING, CORP.
LAND DEVELOPMENT, TRANSPORTATION AND ENVIRONMENTAL SOLUTIONS
770 BROADWAY, SUITE No. 6
RAYNHAM, MA 02767
P.508.297.2746 F.508.297.2756
EMAIL:info@MBLLandDevelopment.com
WEB: www.MBLLand Development.com

FLOOD INSURANCE RATE MAP

JOANNA HILLS ESTATES ASSESSORS MAP C7-3 & PLOT 15

AVON MASSACHUSETTS

PROJ. No: 2016-002

DATE: 8/8/2019 SCALE: 1"=500'

Figure 4: Natural Heritage Map



NOTE: INFORMATION ON THIS PLAN OBTAINED FROM THE 14TH EDITION OF THE MASSACHUSETTS NATURAL HERITAGE ATLAS DATED AUGUST 1, 2017.

LAND DEVELOPMENT & PERMITTING, CORPLAND DEVELOPMENT, TRANSPORTATION AND ENVIRONMENTAL SOLUTIONS 770 BROADWAY, SUITE No. 6 RAYNHAM, MA 02767 P.508.297.2746 F.508.297.2756 EMAIL:info@MBLLandDevelopment.com WEB: www.MBLLand Development.com

NATURAL HERITAGE MAP

JOANNA HILLS ESTATES ASSESSORS MAP C7-3 & PLOT 15

AVON MASSACHUSETTS PROJ. No: 2016-002 DATE: 8/8/2019

1"=500' SCALE:

Figure 5: Critical Areas



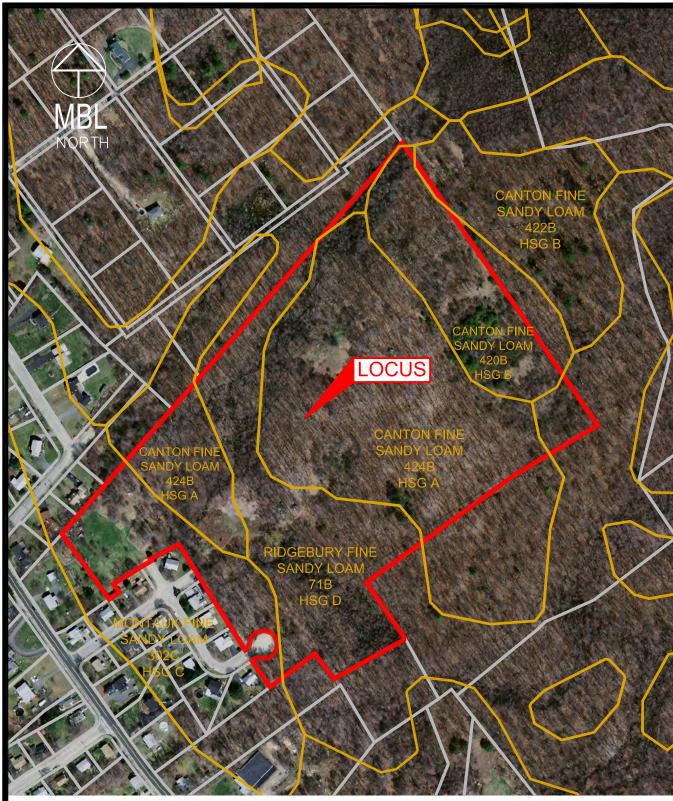
LAND DEVELOPMENT & PERMITTING, CORPLAND DEVELOPMENT, TRANSPORTATION AND ENVIRONMENTAL SOLUTIONS 770 BROADWAY, SUITE No. 6 PAYNHAM, MA 02767 P.508.297.2746 F.508.297.2756 EMAIL:info@MBLLandDevelopment.com WEB: www.MBLLand Development.com

JOANNA HILLS ESTATES ASSESSORS MAP C7-3 & PLOT 15

AVON MASSACHUSETTS DATE: 8/8/2019

1"=500' SCALE:

Figure 6: Soils Map



NOTE: INFORMATION ON THIS PLAN OBTAINED FROM MASS GIS NRCS SSURGO-CERTIFIED SOILS DATA LAYER.

MBL

LAND DEVELOPMENT & PERMITTING, CORP.
LAND DEVELOPMENT. TRANSPORTATION AND EXVIRONMENTAL SOLUTIONS
770 BROADWAY, SUITE No. 6
RAYNHAM, MA 02767
P.508.297.2746 F.508.297.2756
EMAIL:info@MBLLandDevelopment.com
WEB: www.MBLLand Development.com

SOILS MAP

JOANNA HILLS ESTATES ASSESSORS MAP C7-3 & PLOT 15

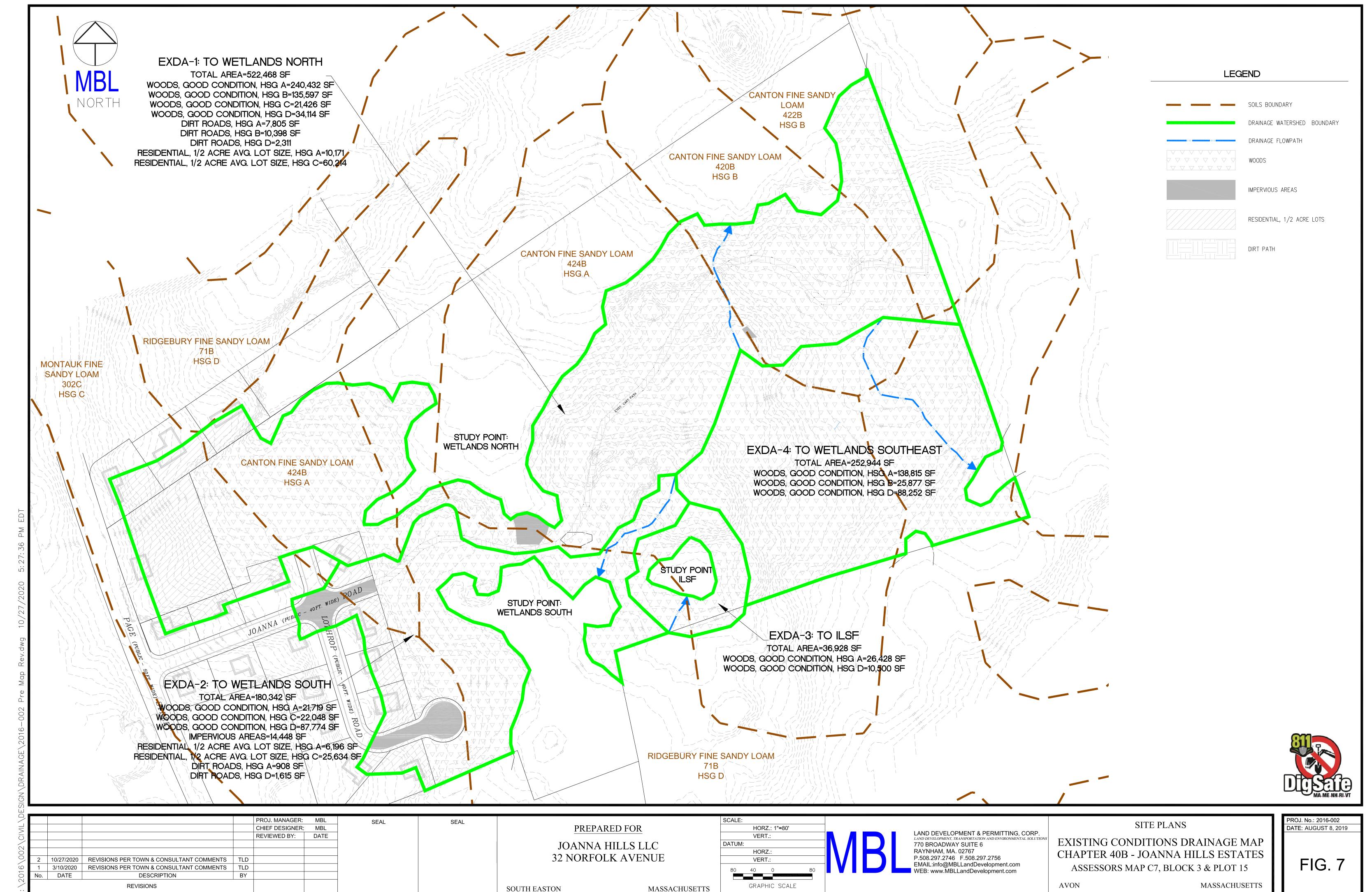
AVON

MASSACHUSETTS

PROJ. No: 2016-002

DATE: 8/8/2019 SCALE: 1"=300'

Figure 7: Existing Conditions Drainage Map



SOUTH EASTON

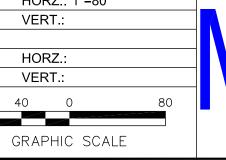
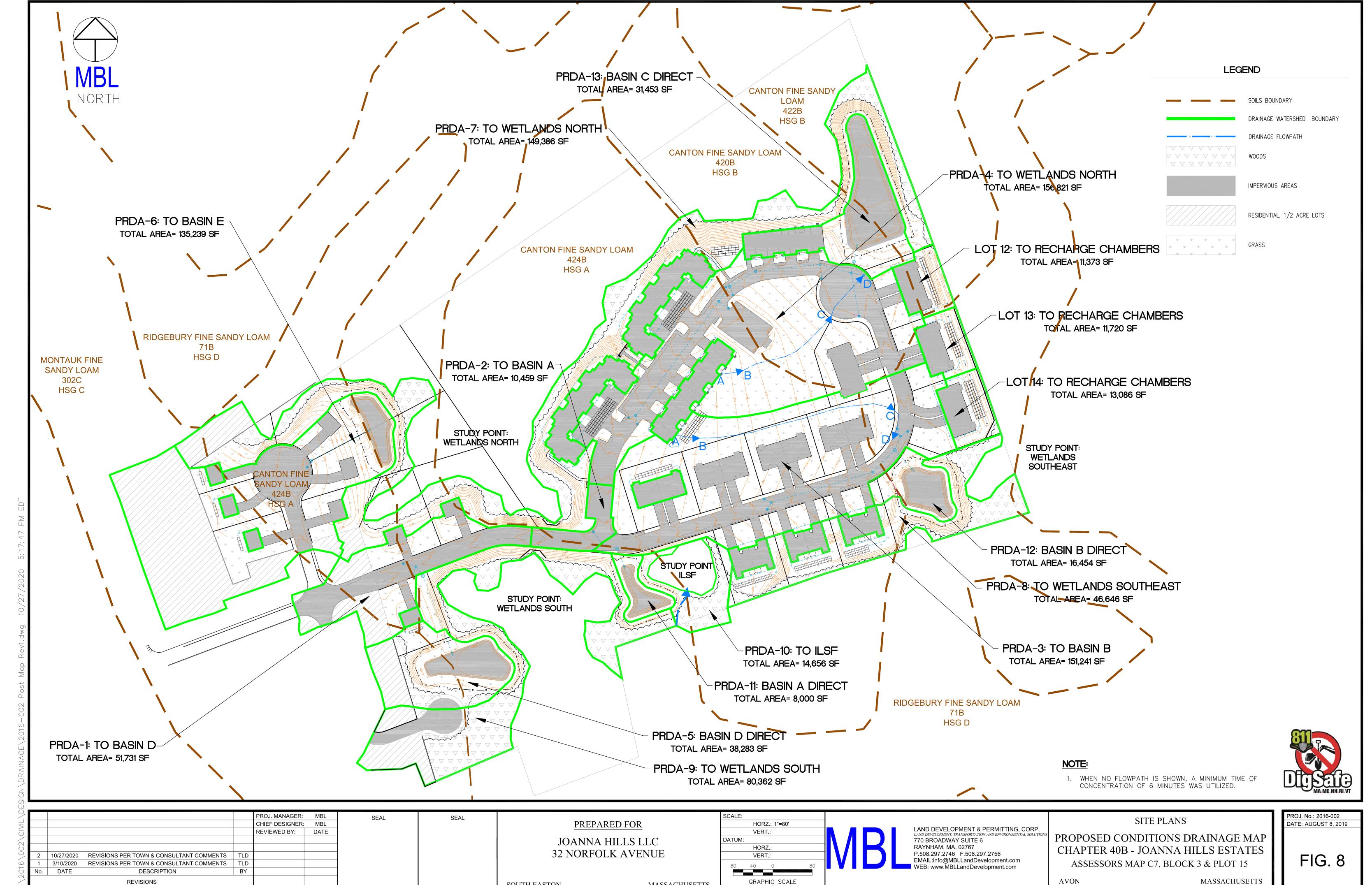


Figure 8: Proposed Conditions Drainage Map



REVISIONS

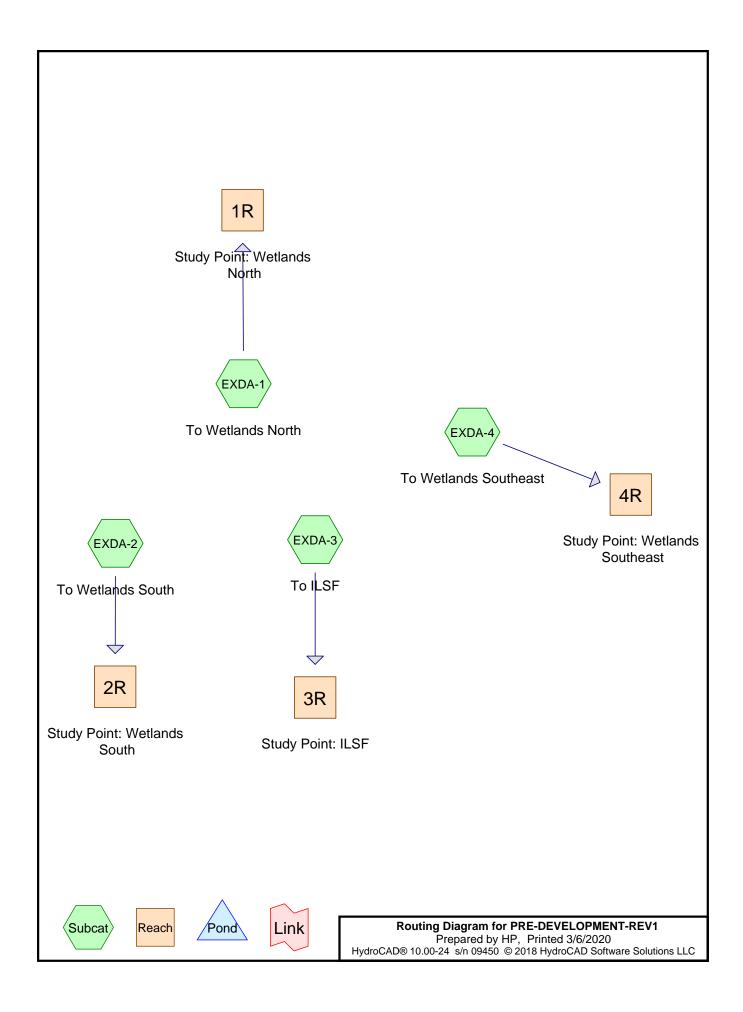
GRAPHIC SCALE MASSACHUSETTS

SOUTH EASTON

MASSACHUSETTS

APPENDIX A: HYDROLOGIC ANALYSIS

Pre-Development HydroCAD Analysis



Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/6/2020

Page 2

Project Notes

Rainfall events imported from "TP-40-Rain.txt" for 446 MA Norfolk Rainfall events imported from "TP-40-Rain.txt" for 446 MA Norfolk Defined 10 rainfall events from MA_AVON_JOANNA-HILL IDF Rainfall events imported from "TP-40-Rain.txt" for 446 MA Norfolk

Printed 3/6/2020 Page 3

Area Listing (all nodes)

Area	CN	Description	
(sq-ft)		(subcatchment-numbers)	
16,367	54	1/2 acre lots, 25% imp, HSG A (EXDA-1, EXDA-2)	
85,848	80	1/2 acre lots, 25% imp, HSG C (EXDA-1, EXDA-2)	
8,713	72	Dirt roads, HSG A (EXDA-1, EXDA-2)	
10,398	82	Dirt roads, HSG B (EXDA-1)	
3,926	89	Dirt roads, HSG D (EXDA-1, EXDA-2)	
14,448	98	Imprevious (EXDA-2)	
427,394	30	Woods, Good, HSG A (EXDA-1, EXDA-2, EXDA-3, EXDA-4)	
161,474	55	Woods, Good, HSG B (EXDA-1, EXDA-4)	
43,474	70	Woods, Good, HSG C (EXDA-1, EXDA-2)	
220,640	77	Woods, Good, HSG D (EXDA-1, EXDA-2, EXDA-3, EXDA-4)	
992,682	53	TOTAL AREA	

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/6/2020 Page 4

Soil Listing (all nodes)

Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
452,474	HSG A	EXDA-1, EXDA-2, EXDA-3, EXDA-4
171,872	HSG B	EXDA-1, EXDA-4
129,322	HSG C	EXDA-1, EXDA-2
224,566	HSG D	EXDA-1, EXDA-2, EXDA-3, EXDA-4
14,448	Other	EXDA-2
992,682		TOTAL AREA

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/6/2020

Page 5

Subcat Numbe

Ground Covers (all nodes)

HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover
16,367	0	85,848	0	0	102,215	1/2 acre lots, 25% imp
8,713	10,398	0	3,926	0	23,037	Dirt roads
0	0	0	0	14,448	14,448	Imprevious
427,394	161,474	43,474	220,640	0	852,982	Woods, Good
452,474	171,872	129,322	224,566	14,448	992,682	TOTAL AREA

Type III 24-hr 2-Year Rainfall=3.20" Printed 3/6/2020

Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 6

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EXDA-1: To Wetlands North Runoff Area=522,468 sf 3.37% Impervious Runoff Depth>0.08"

Flow Length=277' Tc=17.1 min CN=49 Runoff=0.18 cfs 3,605 cf

Subcatchment EXDA-2: To Wetlands Runoff Area=180,342 sf 12.42% Impervious Runoff Depth>0.84"

Flow Length=281' Tc=14.7 min CN=72 Runoff=3.14 cfs 12,629 cf

Subcatchment EXDA-3: To ILSF Runoff Area=36,928 sf 0.00% Impervious Runoff Depth>0.01"

Flow Length=85' Tc=14.4 min CN=43 Runoff=0.00 cfs 37 cf

Subcatchment EXDA-4: To Wetlands Runoff Area=252,944 sf 0.00% Impervious Runoff Depth>0.08"

Flow Length=445' Tc=16.2 min CN=49 Runoff=0.09 cfs 1,747 cf

Reach 1R: Study Point: Wetlands North Inflow=0.18 cfs 3,605 cf

Outflow=0.18 cfs 3,605 cf

Reach 2R: Study Point: Wetlands South Inflow=3.14 cfs 12,629 cf

Outflow=3.14 cfs 12,629 cf

Reach 3R: Study Point: ILSF Inflow=0.00 cfs 37 cf

Outflow=0.00 cfs 37 cf

Reach 4R: Study Point: Wetlands Southeast Inflow=0.09 cfs 1,747 cf

Outflow=0.09 cfs 1,747 cf

Total Runoff Area = 992,682 sf Runoff Volume = 18,018 cf Average Runoff Depth = 0.22" 95.97% Pervious = 952,680 sf 4.03% Impervious = 40,002 sf

Page 7

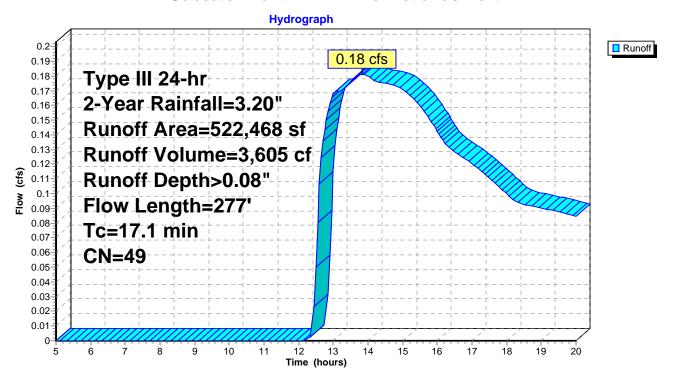
Summary for Subcatchment EXDA-1: To Wetlands North

Runoff = 0.18 cfs @ 13.83 hrs, Volume= 3,605 cf, Depth> 0.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

A	rea (sf)	CN [Description		
2	240,432	30 \	Noods, Go	od, HSG A	
1	35,597	55 \	Noods, Go	od, HSG B	
	21,426	70 \	Noods, Go	od, HSG C	
	34,114	77 \	Noods, Go	od, HSG D	
	60,214	80 ′	1/2 acre lots	s, 25% imp	, HSG C
	10,171	54 <i>′</i>	1/2 acre lots	s, 25% imp	, HSG A
	7,805	72 [Dirt roads, I	HSG A	
	10,398	82 [Dirt roads, I	HSG B	
	2,311	89 [Dirt roads, I	HSG D	
5	522,468	49 \	Neighted A	verage	
5	04,872	Ç	96.63% Pei	vious Area	
	17,596	3	3.37% Impe	ervious Area	a
Tc	Length	Slope	•	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
16.3	50	0.0100	0.05		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.20"
0.8	227	0.0802	4.56		Shallow Concentrated Flow,
-					Unpaved Kv= 16.1 fps
17.1	277	Total			

Subcatchment EXDA-1: To Wetlands North



Printed 3/6/2020 Page 9

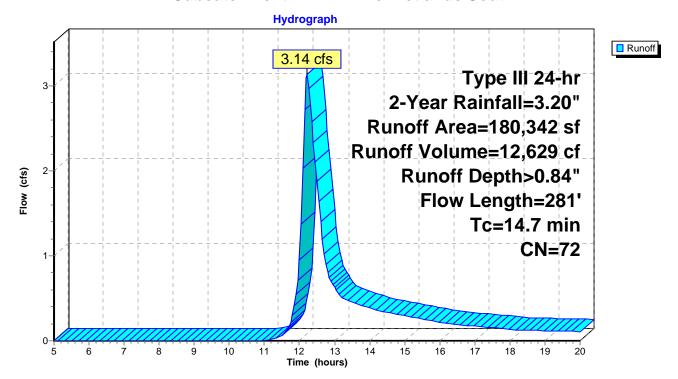
Summary for Subcatchment EXDA-2: To Wetlands South

Runoff = 3.14 cfs @ 12.22 hrs, Volume= 12,629 cf, Depth> 0.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Α	rea (sf)	CN	Description		
		21,719	30	Woods, Go	od, HSG A	
		22,048	70	Woods, Go	od, HSG C	
		87,774	77	Woods, Go	od, HSG D	
*		14,448	98	mprevious		
		6,196	54	1/2 acre lot	s, 25% imp	, HSG A
		25,634	80	1/2 acre lots	s, 25% imp	, HSG C
		908	72	Dirt roads, I	HSG A	
		1,615	89	Dirt roads, I	HSG D	
	1	80,342	72	Weighted A	verage	
	1	57,937		37.58% Pei	vious Area	
		22,406		12.42% lmp	pervious Are	ea
	Тс	Length	Slope	•	Capacity	Description
((min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	13.5	50	0.0160	0.06		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.20"
	1.2	231	0.0397	3.21		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	14.7	281	Total			

Subcatchment EXDA-2: To Wetlands South



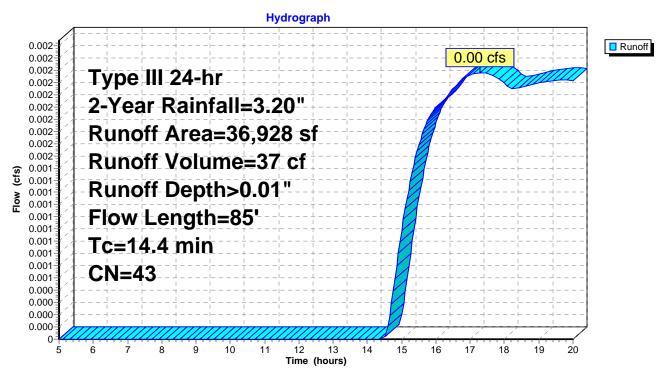
Summary for Subcatchment EXDA-3: To ILSF

Runoff = 0.00 cfs @ 17.28 hrs, Volume= 37 cf, Depth> 0.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

A	rea (sf)	CN	<u>Description</u>		
	26,428	30	Woods, Go	od, HSG A	
	10,500	77	Woods, Go	od, HSG D	
	36,928	43	Weighted A	verage	
	36,928		100.00% Pe	ervious Are	a
Tc	Length	Slope	,	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
14.2	50	0.0140	0.06		Sheet Flow, AB
					Woods: Light underbrush n= 0.400 P2= 3.20"
0.2	35	0.0350	3.01		Shallow Concentrated Flow, BC
					Unpaved Kv= 16.1 fps
14 4	85	Total			

Subcatchment EXDA-3: To ILSF



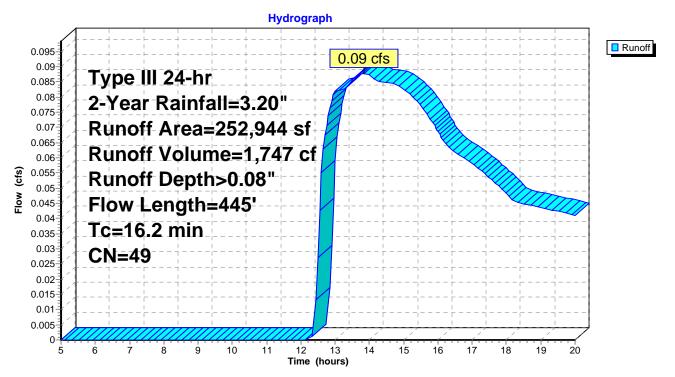
Summary for Subcatchment EXDA-4: To Wetlands Southeast

Runoff = 0.09 cfs @ 13.83 hrs, Volume= 1,747 cf, Depth> 0.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

_	Α	rea (sf)	CN I	Description		
	1	38,815	30 \	Noods, Go	od, HSG A	
		25,877	55 \	Noods, Go	od, HSG B	
_		88,252	77 \	Noods, Go	od, HSG D	
	2	52,944	49 \	Neighted A	verage	
	2	52,944	•	100.00% Pe	ervious Are	a
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	12.9	50	0.0180	0.06		Sheet Flow, AB
						Woods: Light underbrush n= 0.400 P2= 3.20"
	3.3	395	0.0150	1.97		Shallow Concentrated Flow, BC
						Unpaved Kv= 16.1 fps
	16.2	445	Total			

Subcatchment EXDA-4: To Wetlands Southeast



Page 13

Summary for Reach 1R: Study Point: Wetlands North

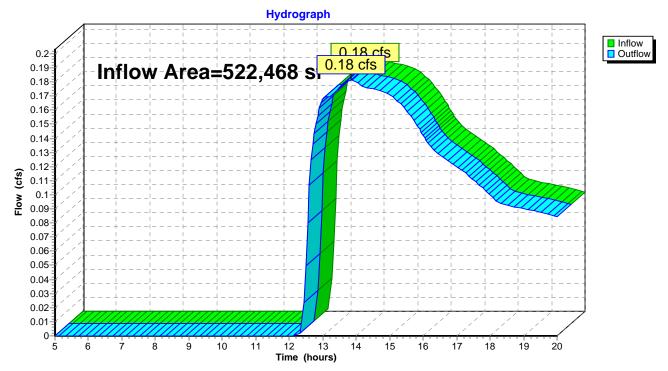
Inflow Area = 522,468 sf, 3.37% Impervious, Inflow Depth > 0.08" for 2-Year event

Inflow = 0.18 cfs @ 13.83 hrs, Volume= 3,605 cf

Outflow = 0.18 cfs @ 13.83 hrs, Volume= 3,605 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 1R: Study Point: Wetlands North



Page 14

Summary for Reach 2R: Study Point: Wetlands South

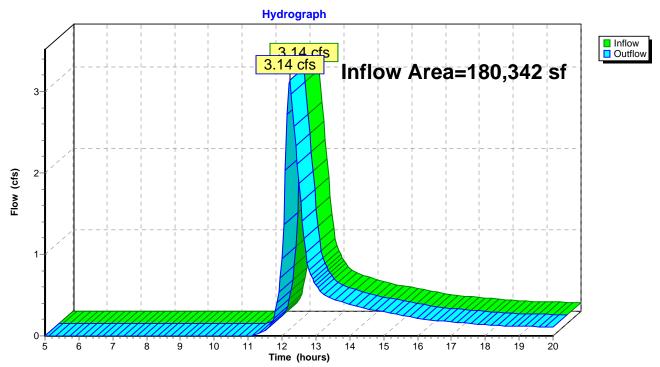
Inflow Area = 180,342 sf, 12.42% Impervious, Inflow Depth > 0.84" for 2-Year event

Inflow = 3.14 cfs @ 12.22 hrs, Volume= 12,629 cf

Outflow = 3.14 cfs @ 12.22 hrs, Volume= 12,629 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 2R: Study Point: Wetlands South



Page 15

Summary for Reach 3R: Study Point: ILSF

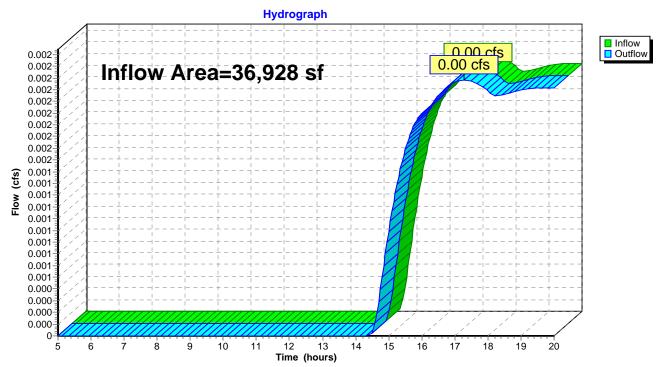
Inflow Area = 36,928 sf, 0.00% Impervious, Inflow Depth > 0.01" for 2-Year event

Inflow = 0.00 cfs @ 17.28 hrs, Volume= 37 cf

Outflow = 0.00 cfs @ 17.28 hrs, Volume= 37 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 3R: Study Point: ILSF



Summary for Reach 4R: Study Point: Wetlands Southeast

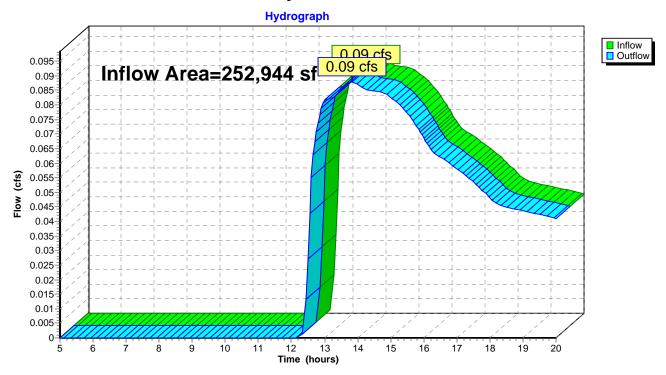
Inflow Area = 252,944 sf, 0.00% Impervious, Inflow Depth > 0.08" for 2-Year event

Inflow = 0.09 cfs @ 13.83 hrs, Volume= 1,747 cf

Outflow = 0.09 cfs @ 13.83 hrs, Volume= 1,747 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 4R: Study Point: Wetlands Southeast



Type III 24-hr 10-Year Rainfall=4.70" Printed 3/6/2020

Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 17

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EXDA-1: To Wetlands North Runoff Area=522,468 sf 3.37% Impervious Runoff Depth>0.45" Flow Length=277' Tc=17.1 min CN=49 Runoff=2.95 cfs 19,601 cf

Subcatchment EXDA-2: To Wetlands

Runoff Area=180,342 sf 12.42% Impervious Runoff Depth>1.81"
Flow Length=281' Tc=14.7 min CN=72 Runoff=7.12 cfs 27,195 cf

Subcatchment EXDA-3: To ILSF

Runoff Area=36,928 sf 0.00% Impervious Runoff Depth>0.22"
Flow Length=85' Tc=14.4 min CN=43 Runoff=0.07 cfs 686 cf

Subcatchment EXDA-4: To Wetlands

Runoff Area=252,944 sf 0.00% Impervious Runoff Depth>0.45"

Flow Length=445' Tc=16.2 min CN=49 Runoff=1.44 cfs 9,496 cf

Reach 1R: Study Point: Wetlands North Inflow=2.95 cfs 19,601 cf

Outflow=2.95 cfs 19,601 cf

Reach 2R: Study Point: Wetlands South Inflow=7.12 cfs 27,195 cf

Outflow=7.12 cfs 27,195 cf

Reach 3R: Study Point: ILSF Inflow=0.07 cfs 686 cf

Outflow=0.07 cfs 686 cf

Reach 4R: Study Point: Wetlands Southeast Inflow=1.44 cfs 9,496 cf

Outflow=1.44 cfs 9,496 cf

Total Runoff Area = 992,682 sf Runoff Volume = 56,977 cf Average Runoff Depth = 0.69" 95.97% Pervious = 952,680 sf 4.03% Impervious = 40,002 sf

Page 18

Summary for Subcatchment EXDA-1: To Wetlands North

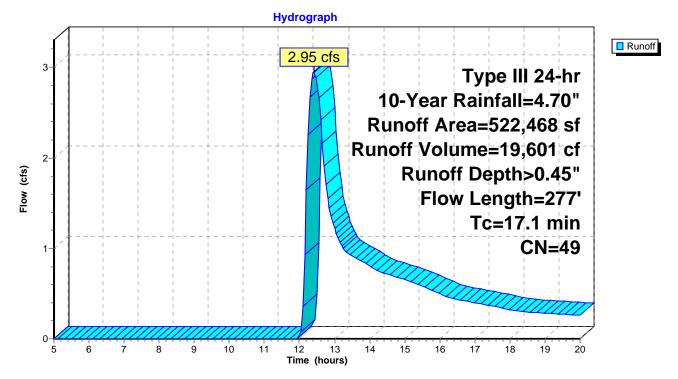
Runoff = 2.95 cfs @ 12.42 hrs, Volume= 19,601 cf, Depth> 0.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

_	Α	rea (sf)	CN	Description		
	2	40,432	30	Woods, Go	od, HSG A	
	1	35,597	55	Woods, Go	od, HSG B	
		21,426	70	Woods, Go	od, HSG C	
		34,114	77	Woods, Go	od, HSG D	
		60,214	80	1/2 acre lot	s, 25% imp	, HSG C
		10,171	54	1/2 acre lot	s, 25% imp	, HSG A
		7,805	72	Dirt roads, l	HSG A	
		10,398		Dirt roads, l		
_		2,311	89	Dirt roads, l	HSG D	
	5	22,468	49	Weighted A	verage	
	5	04,872		96.63% Pei	rvious Area	
		17,596		3.37% Impe	ervious Are	a
	_					
	Tc	Length	Slope	•	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	16.3	50	0.0100	0.05		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.20"
	8.0	227	0.0802	4.56		Shallow Concentrated Flow,
_						Unpaved Kv= 16.1 fps
	17.1	277	Total			

Page 19

Subcatchment EXDA-1: To Wetlands North



Page 20

Summary for Subcatchment EXDA-2: To Wetlands South

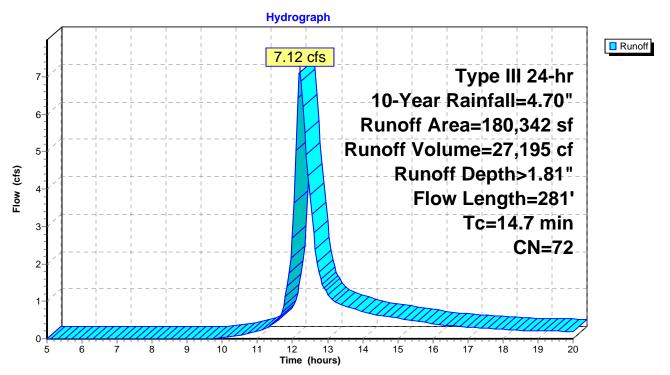
Runoff = 7.12 cfs @ 12.21 hrs, Volume= 27,195 cf, Depth> 1.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Α	rea (sf)	CN	Description		
		21,719	30	Woods, Go	od, HSG A	
		22,048	70	Woods, Go	od, HSG C	
		87,774	77	Woods, Go	od, HSG D	
*		14,448	98	mprevious		
		6,196	54	1/2 acre lot	s, 25% imp	, HSG A
		25,634	80	1/2 acre lots	s, 25% imp	, HSG C
		908	72	Dirt roads, I	HSG A	
		1,615	89	Dirt roads, I	HSG D	
	1	80,342	72	Weighted A	verage	
	1	57,937		37.58% Pei	vious Area	
		22,406		12.42% lmp	pervious Are	ea
	Тс	Length	Slope	•	Capacity	Description
((min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	13.5	50	0.0160	0.06		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.20"
	1.2	231	0.0397	3.21		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	14.7	281	Total			

Page 21

Subcatchment EXDA-2: To Wetlands South



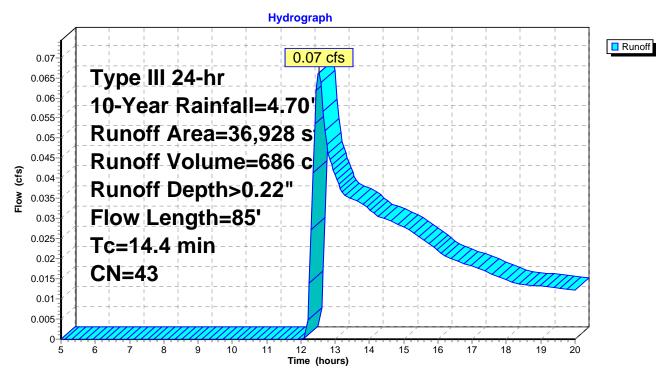
Summary for Subcatchment EXDA-3: To ILSF

Runoff = 0.07 cfs @ 12.52 hrs, Volume= 686 cf, Depth> 0.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

A	rea (sf)	CN	<u>Description</u>		
	26,428	30	Woods, Go	od, HSG A	
	10,500	77	Woods, Go	od, HSG D	
	36,928	43	Weighted A	verage	
	36,928		100.00% Pe	ervious Are	a
Tc	Length	Slope	,	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
14.2	50	0.0140	0.06		Sheet Flow, AB
					Woods: Light underbrush n= 0.400 P2= 3.20"
0.2	35	0.0350	3.01		Shallow Concentrated Flow, BC
					Unpaved Kv= 16.1 fps
14 4	85	Total			

Subcatchment EXDA-3: To ILSF



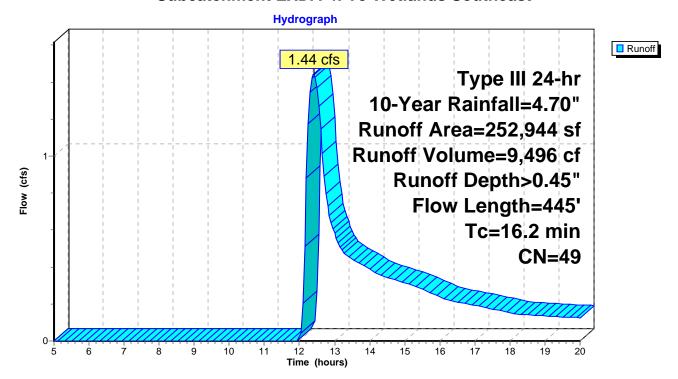
Summary for Subcatchment EXDA-4: To Wetlands Southeast

Runoff = 1.44 cfs @ 12.40 hrs, Volume= 9,496 cf, Depth> 0.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

_	Α	rea (sf)	CN [Description		
	1	38,815	30 \	Voods, Go	od, HSG A	
		25,877		,	od, HSG B	
_		88,252	77 \	Noods, Go	od, HSG D	
	2	52,944	49 \	Neighted A	verage	
	2	52,944	1	100.00% Pe	ervious Are	a
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	12.9	50	0.0180	0.06		Sheet Flow, AB
						Woods: Light underbrush n= 0.400 P2= 3.20"
	3.3	395	0.0150	1.97		Shallow Concentrated Flow, BC
						Unpaved Kv= 16.1 fps
	16.2	445	Total			

Subcatchment EXDA-4: To Wetlands Southeast



Summary for Reach 1R: Study Point: Wetlands North

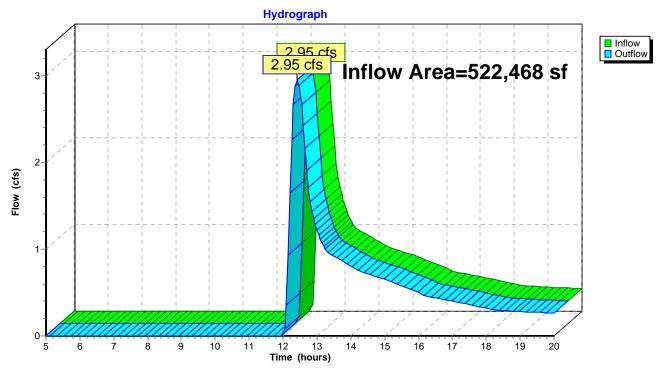
Inflow Area = 522,468 sf, 3.37% Impervious, Inflow Depth > 0.45" for 10-Year event

Inflow = 2.95 cfs @ 12.42 hrs, Volume= 19,601 cf

Outflow = 2.95 cfs @ 12.42 hrs, Volume= 19,601 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 1R: Study Point: Wetlands North



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 25

Summary for Reach 2R: Study Point: Wetlands South

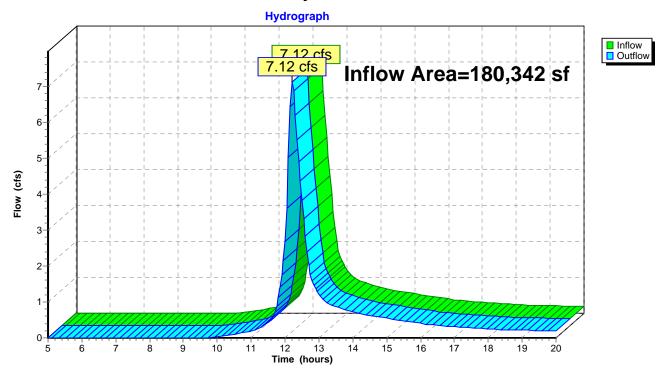
Inflow Area = 180,342 sf, 12.42% Impervious, Inflow Depth > 1.81" for 10-Year event

Inflow = 7.12 cfs @ 12.21 hrs, Volume= 27,195 cf

Outflow = 7.12 cfs @ 12.21 hrs, Volume= 27,195 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 2R: Study Point: Wetlands South



Summary for Reach 3R: Study Point: ILSF

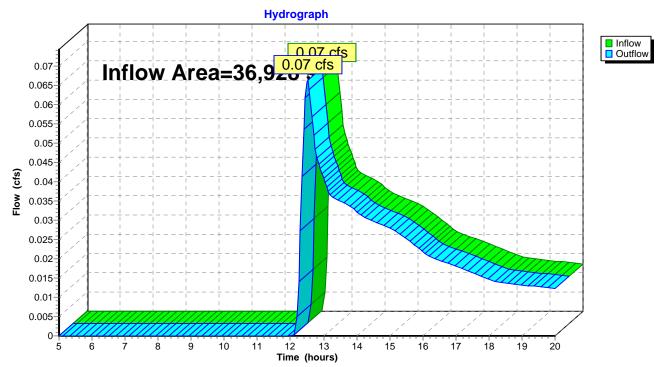
Inflow Area = 36,928 sf, 0.00% Impervious, Inflow Depth > 0.22" for 10-Year event

Inflow = 0.07 cfs @ 12.52 hrs, Volume= 686 cf

Outflow = 0.07 cfs @ 12.52 hrs, Volume= 686 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 3R: Study Point: ILSF



Summary for Reach 4R: Study Point: Wetlands Southeast

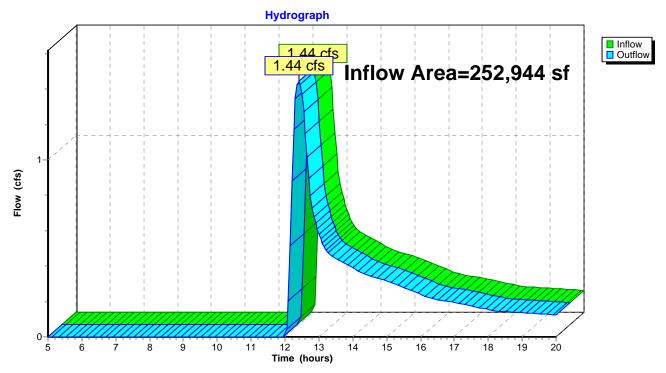
Inflow Area = 252,944 sf, 0.00% Impervious, Inflow Depth > 0.45" for 10-Year event

Inflow = 1.44 cfs @ 12.40 hrs, Volume= 9,496 cf

Outflow = 1.44 cfs @ 12.40 hrs, Volume= 9,496 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 4R: Study Point: Wetlands Southeast



Type III 24-hr 100-Year Rainfall=6.70"

Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 28

Printed 3/6/2020

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EXDA-1: To Wetlands North Runoff Area=522,468 sf 3.37% Impervious Runoff Depth>1.26" Flow Length=277' Tc=17.1 min CN=49 Runoff=11.70 cfs 54,931 cf

Flow Length=277 TC=17.1 min CN=49 Runon=11.70 ds 54,931 d

Subcatchment EXDA-2: To Wetlands

Runoff Area=180,342 sf 12.42% Impervious Runoff Depth>3.32"
Flow Length=281' Tc=14.7 min CN=72 Runoff=13.16 cfs 49,871 cf

Subcatchment EXDA-3: To ILSF

Runoff Area=36,928 sf 0.00% Impervious Runoff Depth>0.82"
Flow Length=85' Tc=14.4 min CN=43 Runoff=0.46 cfs 2,537 cf

Subcatchment EXDA-4: To Wetlands

Runoff Area=252,944 sf 0.00% Impervious Runoff Depth>1.26"

Flow Length=445' Tc=16.2 min CN=49 Runoff=5.81 cfs 26,607 cf

Reach 1R: Study Point: Wetlands North Inflow=11.70 cfs 54,931 cf

Outflow=11.70 cfs 54,931 cf

Reach 2R: Study Point: Wetlands South Inflow=13.16 cfs 49,871 cf

Outflow=13.16 cfs 49,871 cf

Reach 3R: Study Point: ILSF Inflow=0.46 cfs 2,537 cf

Outflow=0.46 cfs 2,537 cf

Reach 4R: Study Point: Wetlands Southeast Inflow=5.81 cfs 26,607 cf

Outflow=5.81 cfs 26,607 cf

Total Runoff Area = 992,682 sf Runoff Volume = 133,946 cf Average Runoff Depth = 1.62" 95.97% Pervious = 952,680 sf 4.03% Impervious = 40,002 sf

Page 29

Summary for Subcatchment EXDA-1: To Wetlands North

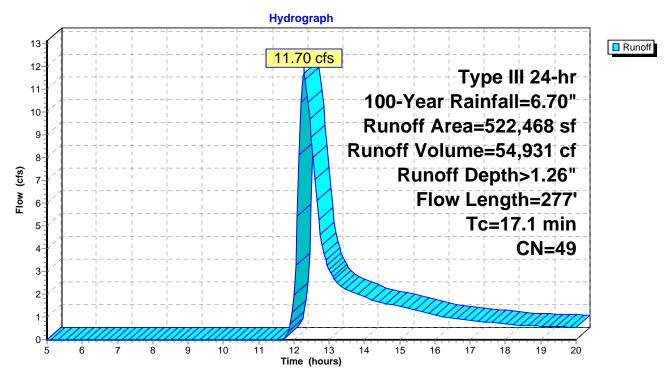
Runoff = 11.70 cfs @ 12.28 hrs, Volume= 54,931 cf, Depth> 1.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

A	rea (sf)	CN [Description		
2	240,432	30 \	Noods, Go	od, HSG A	
1	35,597	55 \	Noods, Go	od, HSG B	
	21,426	70 \	Noods, Go	od, HSG C	
	34,114	77 \	Noods, Go	od, HSG D	
	60,214	80 ′	1/2 acre lots	s, 25% imp	, HSG C
	10,171	54 <i>′</i>	1/2 acre lots	s, 25% imp	, HSG A
	7,805	72 [Dirt roads, I	HSG A	
	10,398	82 [Dirt roads, I	HSG B	
	2,311	89 [Dirt roads, I	HSG D	
5	522,468	49 \	Neighted A	verage	
5	04,872	Ç	96.63% Pei	vious Area	
	17,596	3	3.37% Impe	ervious Area	a
Tc	Length	Slope	•	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
16.3	50	0.0100	0.05		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.20"
0.8	227	0.0802	4.56		Shallow Concentrated Flow,
-					Unpaved Kv= 16.1 fps
17.1	277	Total			

Page 30

Subcatchment EXDA-1: To Wetlands North



Page 31

Summary for Subcatchment EXDA-2: To Wetlands South

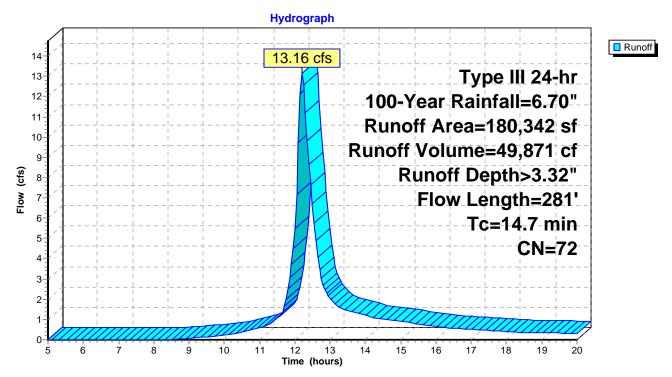
Runoff = 13.16 cfs @ 12.21 hrs, Volume= 49,871 cf, Depth> 3.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Α	rea (sf)	CN	Description			
21,719 30 Woods, Good, HSG A							
		22,048	70	Woods, Good, HSG C			
		87,774	77	Woods, Go	od, HSG D		
* 14,448 98 Imprevi				mprevious	nprevious		
6,196 54 1/2 acre lots, 25% imp, HSG A				, HSG A			
	25,634 80		80	1/2 acre lots, 25% imp, HSG C			
	908 72 Dirt roads, HSG A						
		1,615 89 Dirt roads, HSG D					
	180,342 72 Weighted Average						
	157,937		87.58% Pervious Area				
	22,406		12.42% Impervious Area			ea	
	Тс	Length	Slope	•	Capacity	Description	
((min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	13.5	50	0.0160	0.06		Sheet Flow,	
						Woods: Light underbrush n= 0.400 P2= 3.20"	
	1.2	231	0.0397	3.21		Shallow Concentrated Flow,	
						Unpaved Kv= 16.1 fps	
	14.7	281	Total				

Page 32

Subcatchment EXDA-2: To Wetlands South



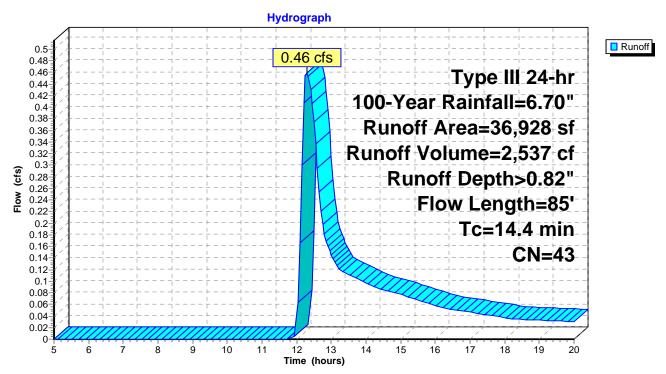
Summary for Subcatchment EXDA-3: To ILSF

Runoff = 0.46 cfs @ 12.29 hrs, Volume= 2,537 cf, Depth> 0.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

_	Α	rea (sf)	CN	Description		
26,428 30 Woods, Good, HSG A						
10,500 77 Woods, Good, HSG D						
36,928 43 Weighted Average						
		36,928		100.00% Pe	ervious Are	a
	Tc	Length	Slope	,	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	14.2	50	0.0140	0.06		Sheet Flow, AB
						Woods: Light underbrush n= 0.400 P2= 3.20"
	0.2	35	0.0350	3.01		Shallow Concentrated Flow, BC
						Unpaved Kv= 16.1 fps
_	14.4	85	Total	•	•	

Subcatchment EXDA-3: To ILSF



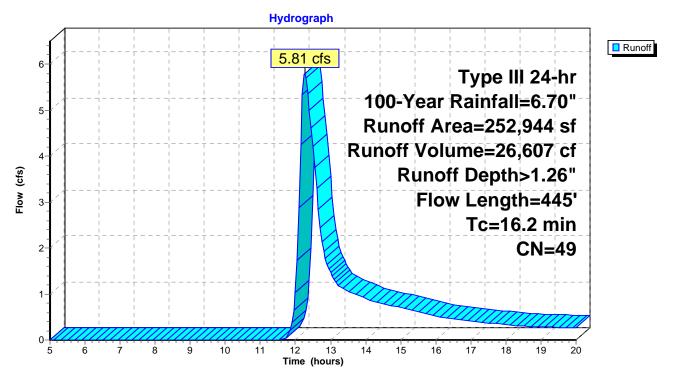
Summary for Subcatchment EXDA-4: To Wetlands Southeast

Runoff = 5.81 cfs @ 12.27 hrs, Volume= 26,607 cf, Depth> 1.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

_	Α	rea (sf)	CN [Description		
138,815 30 Woods, Good, HSG A			Voods, Go	od, HSG A		
		25,877	55 \	Voods, Go	od, HSG B	
_		88,252	77 \	<u>Voods, Go</u>	od, HSG D	
252,944 49 Weighted Average			Veighted A	verage		
	2	52,944	1	00.00% Pe	ervious Are	a
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	12.9	50	0.0180	0.06		Sheet Flow, AB
						Woods: Light underbrush n= 0.400 P2= 3.20"
	3.3	395	0.0150	1.97		Shallow Concentrated Flow, BC
_						Unpaved Kv= 16.1 fps
·	16.2	445	Total			

Subcatchment EXDA-4: To Wetlands Southeast



Page 35

Summary for Reach 1R: Study Point: Wetlands North

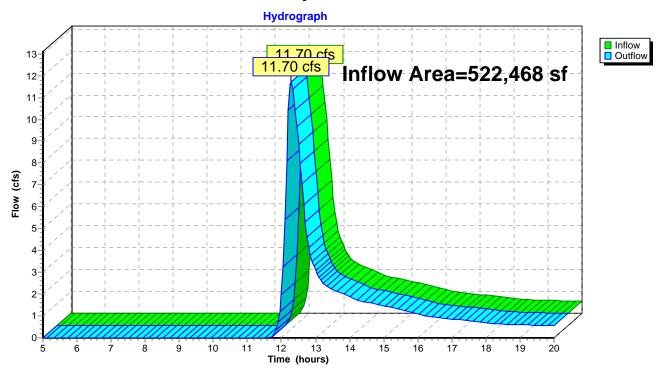
Inflow Area = 522,468 sf, 3.37% Impervious, Inflow Depth > 1.26" for 100-Year event

Inflow = 11.70 cfs @ 12.28 hrs, Volume= 54,931 cf

Outflow = 11.70 cfs @ 12.28 hrs, Volume= 54,931 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 1R: Study Point: Wetlands North



Page 36

Summary for Reach 2R: Study Point: Wetlands South

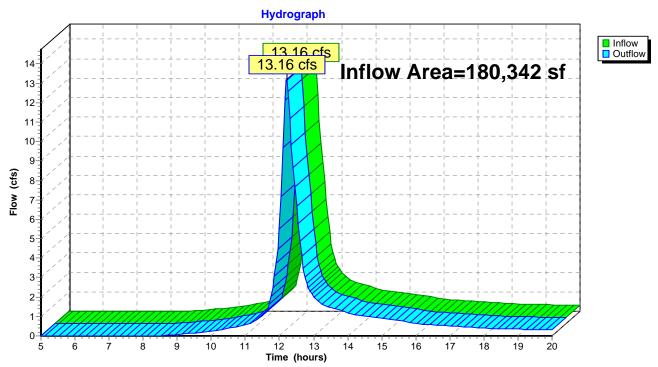
Inflow Area = 180,342 sf, 12.42% Impervious, Inflow Depth > 3.32" for 100-Year event

Inflow = 13.16 cfs @ 12.21 hrs, Volume= 49,871 cf

Outflow = 13.16 cfs @ 12.21 hrs, Volume= 49,871 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 2R: Study Point: Wetlands South



Page 37

Summary for Reach 3R: Study Point: ILSF

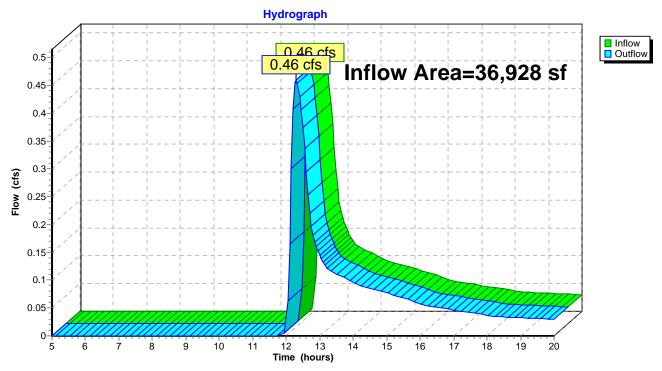
Inflow Area = 36,928 sf, 0.00% Impervious, Inflow Depth > 0.82" for 100-Year event

Inflow = 0.46 cfs @ 12.29 hrs, Volume= 2,537 cf

Outflow = 0.46 cfs @ 12.29 hrs, Volume= 2,537 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 3R: Study Point: ILSF



Page 38

Summary for Reach 4R: Study Point: Wetlands Southeast

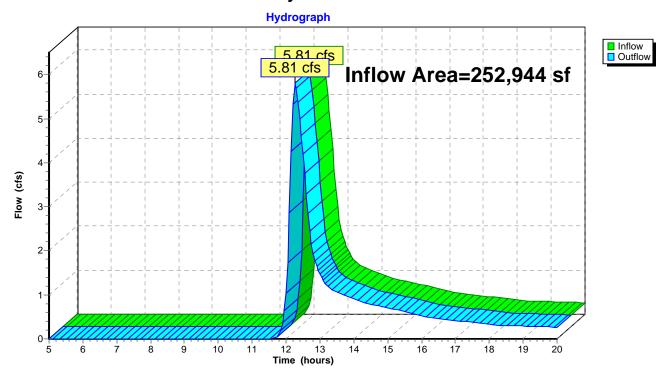
Inflow Area = 252,944 sf, 0.00% Impervious, Inflow Depth > 1.26" for 100-Year event

Inflow = 5.81 cfs @ 12.27 hrs, Volume= 26,607 cf

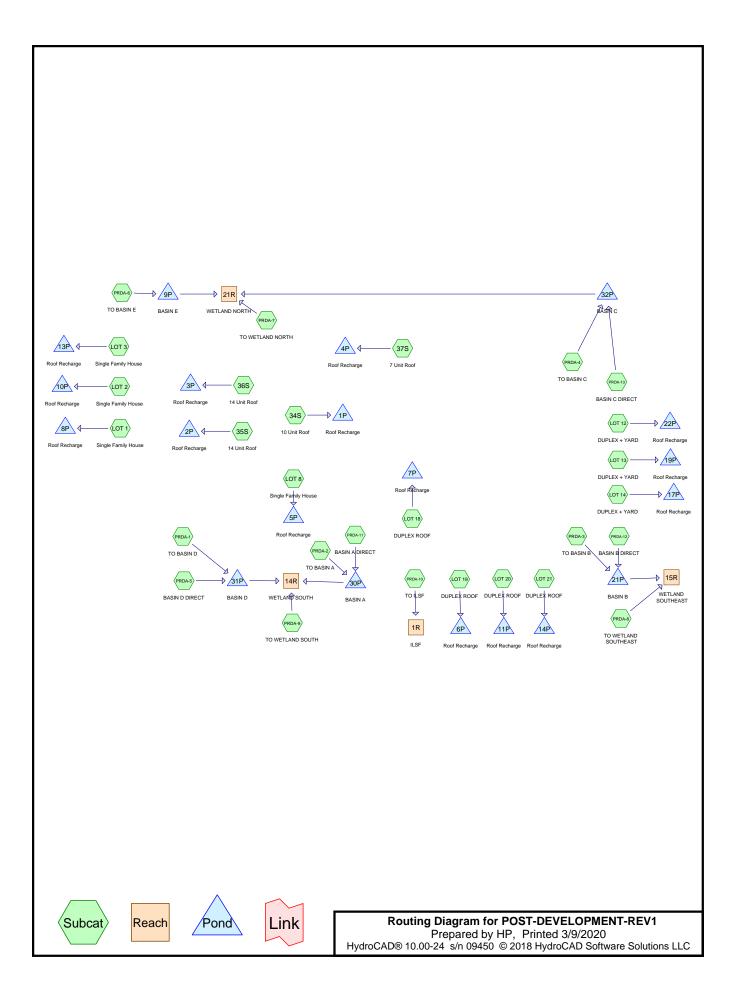
Outflow = 5.81 cfs @ 12.27 hrs, Volume= 26,607 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 4R: Study Point: Wetlands Southeast



Post-Development HydroCAD Analysis



Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC Printed 3/9/2020 Page 2

Project Notes

Rainfall events imported from "TP-40-Rain.txt" for 446 MA Norfolk Defined 10 rainfall events from MA_AVON_JOANNA-HILL IDF Defined 10 rainfall events from MA_AVON_JOANNA-HILL IDF Rainfall events imported from "TP-40-Rain.txt" for 446 MA Norfolk

Printed 3/9/2020 Page 3

Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
16,595	54	1/2 acre lots, 25% imp, HSG A (PRDA-1, PRDA-6, PRDA-7)
86,361	80	1/2 acre lots, 25% imp, HSG C (PRDA-1, PRDA-5, PRDA-6, PRDA-7, PRDA-9)
226,878	39	>75% Grass cover, Good, HSG A (PRDA-1, PRDA-10, PRDA-12, PRDA-3,
		PRDA-4, PRDA-5, PRDA-6, PRDA-7, PRDA-8, PRDA-9)
94,170	61	>75% Grass cover, Good, HSG B (LOT 12, PRDA-13, PRDA-3, PRDA-4,
		PRDA-7)
24,520	74	>75% Grass cover, Good, HSG C (PRDA-1, PRDA-5, PRDA-6, PRDA-7)
98,291	80	>75% Grass cover, Good, HSG D (LOT 12, LOT 13, LOT 14, PRDA-1, PRDA-10,
		PRDA-11, PRDA-12, PRDA-3, PRDA-4, PRDA-5, PRDA-6, PRDA-7, PRDA-8,
		PRDA-9)
432	96	Gravel surface, HSG C (PRDA-5)
76,926	98	IMPERVIOUS (PRDA-2, PRDA-3, PRDA-6, PRDA-9)
178,461	98	Impervious (34S, 35S, 36S, 37S, LOT 1, LOT 12, LOT 13, LOT 14, LOT 18, LOT
		19, LOT 2, LOT 20, LOT 21, LOT 3, LOT 8, PRDA-1, PRDA-4, PRDA-5)
18,387	98	Roofs, HSG A (PRDA-3, PRDA-6)
4,929	98	Roofs, HSG B (PRDA-4)
9,227	98	Water Surface, HSG A (PRDA-5)
19,611	98	Water Surface, HSG B (PRDA-11, PRDA-13)
13,399	98	Water Surface, HSG D (PRDA-12, PRDA-6)
36,216	30	Woods, Good, HSG A (PRDA-10, PRDA-7, PRDA-8)
23,645	70	Woods, Good, HSG C (PRDA-5, PRDA-7, PRDA-9)
63,256	77	Woods, Good, HSG D (PRDA-10, PRDA-5, PRDA-7, PRDA-8, PRDA-9)
1,378	98	impervious (PRDA-7)
992,682	72	TOTAL AREA

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 4

Soil Listing (all nodes)

Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
307,303	HSG A	PRDA-1, PRDA-10, PRDA-12, PRDA-3, PRDA-4, PRDA-5, PRDA-6, PRDA-7,
		PRDA-8, PRDA-9
118,710	HSG B	LOT 12, PRDA-11, PRDA-13, PRDA-3, PRDA-4, PRDA-7
134,958	HSG C	PRDA-1, PRDA-5, PRDA-6, PRDA-7, PRDA-9
174,946	HSG D	LOT 12, LOT 13, LOT 14, PRDA-1, PRDA-10, PRDA-11, PRDA-12, PRDA-3,
		PRDA-4, PRDA-5, PRDA-6, PRDA-7, PRDA-8, PRDA-9
256,765	Other	34S, 35S, 36S, 37S, LOT 1, LOT 12, LOT 13, LOT 14, LOT 18, LOT 19, LOT
		2, LOT 20, LOT 21, LOT 3, LOT 8, PRDA-1, PRDA-2, PRDA-3, PRDA-4,
		PRDA-5, PRDA-6, PRDA-7, PRDA-9
992,682		TOTAL AREA

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020

Page 5

Sub Nun

Ground Covers (all nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground
(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover
16,595	0	86,361	0	0	102,956	1/2 acre lots, 25% imp
226,878	94,170	24,520	98,291	0	443,859	>75% Grass cover, Good
0	0	432	0	0	432	Gravel surface
0	0	0	0	76,926	76,926	IMPERVIOUS
0	0	0	0	178,461	178,461	Impervious
18,387	4,929	0	0	0	23,316	Roofs
9,227	19,611	0	13,399	0	42,237	Water Surface
36,216	0	23,645	63,256	0	123,117	Woods, Good
0	0	0	0	1,378	1,378	impervious
307,303	118,710	134,958	174,946	256,765	992,682	TOTAL AREA

Type III 24-hr 2-Year Rainfall=3.20" Printed 3/9/2020

Tc=6.0 min CN=98 Runoff=0.08 cfs 277 cf

Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 6

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

5 ,	3 ,
Subcatchment34S: 10 Unit Roof	Runoff Area=10,448 sf 100.00% Impervious Runoff Depth>2.77" Tc=6.0 min CN=98 Runoff=0.73 cfs 2,415 cf
Subcatchment35S: 14 Unit Roof	Runoff Area=13,061 sf 100.00% Impervious Runoff Depth>2.77" Tc=6.0 min CN=98 Runoff=0.91 cfs 3,019 cf
Subcatchment 36S: 14 Unit Roof	Runoff Area=13,061 sf 100.00% Impervious Runoff Depth>2.77" Tc=6.0 min CN=98 Runoff=0.91 cfs 3,019 cf
Subcatchment 37S: 7 Unit Roof	Runoff Area=7,296 sf 100.00% Impervious Runoff Depth>2.77" Tc=6.0 min CN=98 Runoff=0.51 cfs 1,686 cf
Subcatchment LOT 1: Single Family House	Se Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>2.77" Tc=6.0 min CN=98 Runoff=0.08 cfs 277 cf
Subcatchment LOT 12: DUPLEX + YARD	Runoff Area=11,373 sf 43.34% Impervious Runoff Depth>1.17" Tc=6.0 min CN=78 Runoff=0.38 cfs 1,110 cf
Subcatchment LOT 13: DUPLEX + YARD	Runoff Area=11,720 sf 42.06% Impervious Runoff Depth>1.87" Tc=6.0 min CN=88 Runoff=0.61 cfs 1,828 cf
Subcatchment LOT 14: DUPLEX + YARD	Runoff Area=13,086 sf 37.67% Impervious Runoff Depth>1.79" Tc=6.0 min CN=87 Runoff=0.66 cfs 1,953 cf
Subcatchment LOT 18: DUPLEX ROOF	Runoff Area=4,929 sf 100.00% Impervious Runoff Depth>2.77" Tc=6.0 min CN=98 Runoff=0.34 cfs 1,139 cf
Subcatchment LOT 19: DUPLEX ROOF	Runoff Area=4,059 sf 100.00% Impervious Runoff Depth>2.77" Tc=6.0 min CN=98 Runoff=0.28 cfs 938 cf
Subcatchment LOT 2: Single Family House	Se Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>2.77" Tc=6.0 min CN=98 Runoff=0.08 cfs 277 cf
Subcatchment LOT 20: DUPLEX ROOF	Runoff Area=4,059 sf 100.00% Impervious Runoff Depth>2.77" Tc=6.0 min CN=98 Runoff=0.28 cfs 938 cf
Subcatchment LOT 21: DUPLEX ROOF	Runoff Area=4,059 sf 100.00% Impervious Runoff Depth>2.77" Tc=6.0 min CN=98 Runoff=0.28 cfs 938 cf
Subcatchment LOT 3: Single Family House	se Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>2.77" Tc=6.0 min CN=98 Runoff=0.08 cfs 277 cf

Subcatchment PRDA-1: TO BASIN D Runoff Area=51,731 sf 56.02% Impervious Runoff Depth>1.36" Tc=6.0 min CN=81 Runoff=1.99 cfs 5,860 cf

Subcatchment LOT 8: Single Family House Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>2.77"

POST-DEVELOPMENT-REV1 Prepared by HP	Type III 24-hr 2-Year Rainfall=3.20" Printed 3/9/2020
HydroCAD® 10.00-24 s/n 09450 © 2018 Hydr	roCAD Software Solutions LLC Page 7
Subcatchment PRDA-10: TO ILSF	Runoff Area=14,656 sf 0.00% Impervious Runoff Depth>0.05" Flow Length=85' Tc=15.0 min CN=47 Runoff=0.00 cfs 64 cf
Subcatchment PRDA-11: BASIN A DIREC	Runoff Area=8,000 sf 49.79% Impervious Runoff Depth>1.96" Tc=0.0 min CN=89 Runoff=0.51 cfs 1,305 cf
Subcatchment PRDA-12: BASIN B DIREC	Tc=6.0 min CN=78 Runoff=0.54 cfs 1,606 cf
Subcatchment PRDA-13: BASIN C DIREC	Runoff Area=31,453 sf 49.69% Impervious Runoff Depth>1.23" Tc=6.0 min CN=79 Runoff=1.09 cfs 3,230 cf
Subcatchment PRDA-2: TO BASIN A	Runoff Area=10,459 sf 100.00% Impervious Runoff Depth>2.77" Tc=0.0 min CN=98 Runoff=0.85 cfs 2,418 cf
Subcatchment PRDA-3: TO BASIN B	Runoff Area=151,241 sf 36.39% Impervious Runoff Depth>0.57" Flow Length=520' Tc=13.1 min CN=66 Runoff=1.66 cfs 7,199 cf
Subcatchment PRDA-4: TO BASIN C	Runoff Area=156,821 sf 48.34% Impervious Runoff Depth>0.94" Flow Length=370' Tc=10.4 min CN=74 Runoff=3.54 cfs 12,342 cf
Subcatchment PRDA-5: BASIN D DIRECT	Runoff Area=38,283 sf 32.09% Impervious Runoff Depth>1.49" Tc=6.0 min CN=83 Runoff=1.62 cfs 4,768 cf
Subcatchment PRDA-6: TO BASIN E	Runoff Area=135,239 sf 32.90% Impervious Runoff Depth>0.95" Tc=6.0 min CN=74 Runoff=3.52 cfs 10,664 cf
Subcatchment PRDA-7: TO WETLAND	Runoff Area=149,386 sf 2.80% Impervious Runoff Depth>0.14" Tc=6.0 min CN=52 Runoff=0.18 cfs 1,755 cf
Subcatchment PRDA-8: TO WETLAND	Runoff Area=46,646 sf 0.00% Impervious Runoff Depth>0.19" Tc=6.0 min CN=54 Runoff=0.09 cfs 724 cf
Subcatchment PRDA-9: TO WETLAND	Runoff Area=80,362 sf 11.49% Impervious Runoff Depth>1.23" Tc=6.0 min CN=79 Runoff=2.80 cfs 8,252 cf
Reach 1R: ILSF	Inflow=0.00 cfs 64 cf Outflow=0.00 cfs 64 cf
Reach 14R: WETLAND SOUTH	Inflow=2.80 cfs 8,252 cf Outflow=2.80 cfs 8,252 cf
Reach 15R: WETLAND SOUTHEAST	Inflow=0.09 cfs 724 cf

Reach 15R: WETLAND SOUTHEAST

Reach 21R: WETLAND NORTH

Inflow=0.18 cfs 1,755 cf Outflow=0.18 cfs 1,755 cf

Outflow=0.09 cfs 724 cf

Peak Elev=0.88' Storage=801 cf Inflow=0.73 cfs 2,415 cf Pond 1P: Roof Recharge

Outflow=0.09 cfs 2,412 cf

Pond 2P: Roof Recharge Peak Elev=0.98' Storage=1,366 cf Inflow=0.91 cfs 3,019 cf

Outflow=0.06 cfs 2,385 cf

POST-DEVELOPMENT-REV1 Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD	Type III 24-hr 2-Year Rainfall=3.20" Printed 3/9/2020 Software Solutions LLC Page 8
Pond 3P: Roof Recharge	Peak Elev=0.98' Storage=1,366 cf Inflow=0.91 cfs 3,019 cf Outflow=0.06 cfs 2,385 cf
Pond 4P: Roof Recharge	Peak Elev=1.00' Storage=762 cf Inflow=0.51 cfs 1,686 cf Outflow=0.03 cfs 1,341 cf
Pond 5P: Roof Recharge	Peak Elev=1.04' Storage=120 cf Inflow=0.08 cfs 277 cf Outflow=0.01 cfs 244 cf
Pond 6P: Roof Recharge	Peak Elev=0.77' Storage=378 cf Inflow=0.28 cfs 938 cf Outflow=0.02 cfs 924 cf
Pond 7P: Roof Recharge	Peak Elev=0.93' Storage=493 cf Inflow=0.34 cfs 1,139 cf Outflow=0.03 cfs 987 cf

Outflow=0.01 cfs 244 cf

Pond 8P: Roof Recharge

Pond 9P: BASIN E Peak Elev=238.98' Storage=5,062 cf Inflow=3.52 cfs 10,664 cf Discarded=0.28 cfs 8,057 cf Primary=0.00 cfs 0 cf Outflow=0.28 cfs 8,057 cf

Pond 10P: Roof Recharge Peak Elev=1.04' Storage=120 cf Inflow=0.08 cfs 277 cf

Outflow=0.01 cfs 244 cf

Peak Elev=1.04' Storage=120 cf Inflow=0.08 cfs 277 cf

Pond 11P: Roof Recharge Peak Elev=0.77' Storage=378 cf Inflow=0.28 cfs 938 cf

Outflow=0.02 cfs 924 cf

Pond 13P: Roof Recharge Peak Elev=1.04' Storage=120 cf Inflow=0.08 cfs 277 cf

Outflow=0.01 cfs 244 cf

Pond 14P: Roof Recharge Peak Elev=0.82' Storage=390 cf Inflow=0.28 cfs 938 cf

Outflow=0.02 cfs 870 cf

Pond 17P: Roof Recharge Peak Elev=0.93' Storage=945 cf Inflow=0.66 cfs 1,953 cf

Outflow=0.04 cfs 1,500 cf

Pond 19P: Roof Recharge Peak Elev=0.96' Storage=681 cf Inflow=0.61 cfs 1,828 cf

Outflow=0.08 cfs 1,825 cf

Pond 21P: BASIN B Peak Elev=226.10' Storage=3,195 cf Inflow=1.99 cfs 8,805 cf

 $Discarded = 0.31 \ cfs \ 8,740 \ cf \ Primary = 0.00 \ cfs \ 0 \ cf \ Outflow = 0.31 \ cfs \ 8,740 \ cf$

Pond 22P: Roof Recharge Peak Elev=0.80' Storage=504 cf Inflow=0.38 cfs 1,110 cf

Outflow=0.03 cfs 948 cf

Pond 30P: BASIN A Peak Elev=238.99' Storage=1,376 cf Inflow=1.36 cfs 3,723 cf

Discarded=0.17 cfs 3,715 cf Primary=0.00 cfs 0 cf Outflow=0.17 cfs 3,715 cf

Pond 31P: BASIN D Peak Elev=239.67' Storage=4,022 cf Inflow=3.62 cfs 10,629 cf

Discarded=0.53 cfs 10,593 cf Primary=0.00 cfs 0 cf Outflow=0.53 cfs 10,593 cf

Type III 24-hr 2-Year Rainfall=3.20"

Page 9

Printed 3/9/2020 Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Peak Elev=224.77' Storage=5,663 cf Inflow=4.44 cfs 15,572 cf Pond 32P: BASIN C Discarded=0.85 cfs 15,510 cf Primary=0.00 cfs 0 cf Outflow=0.85 cfs 15,510 cf

> Total Runoff Area = 992,682 sf Runoff Volume = 80,281 cf Average Runoff Depth = 0.97" 64.94% Pervious = 644,625 sf 35.06% Impervious = 348,057 sf

Page 10

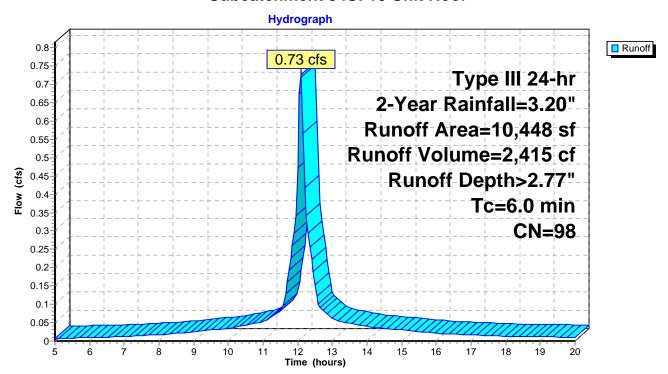
Summary for Subcatchment 34S: 10 Unit Roof

Runoff = 0.73 cfs @ 12.09 hrs, Volume= 2,415 cf, Depth> 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Α	rea (sf)	CN	Description		
*		10,448	98	Impervious		
		10,448		100.00% lm	npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment 34S: 10 Unit Roof



Page 11

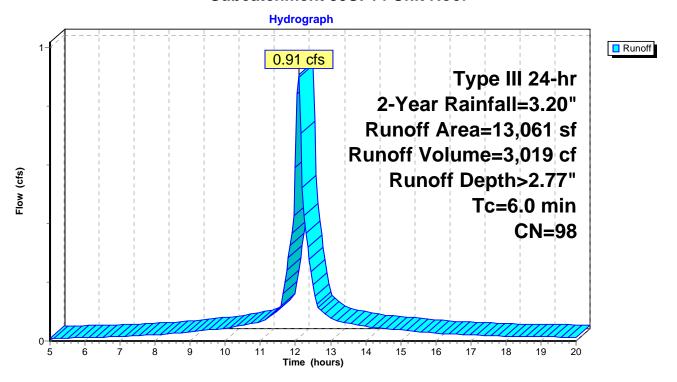
Summary for Subcatchment 35S: 14 Unit Roof

Runoff = 0.91 cfs @ 12.09 hrs, Volume= 3,019 cf, Depth> 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

_	Α	rea (sf)	CN	Description		
*		13,061	98	mpervious		
	13,061 100.00% Impervious Are				npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment 35S: 14 Unit Roof



Page 12

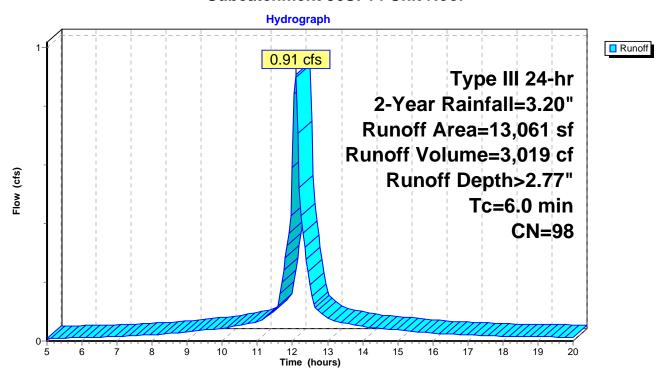
Summary for Subcatchment 36S: 14 Unit Roof

Runoff = 0.91 cfs @ 12.09 hrs, Volume= 3,019 cf, Depth> 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

_	Α	rea (sf)	CN	Description		
*		13,061	98	Impervious		
		13,061 100.00% Impervious Area				Area
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment 36S: 14 Unit Roof



Page 13

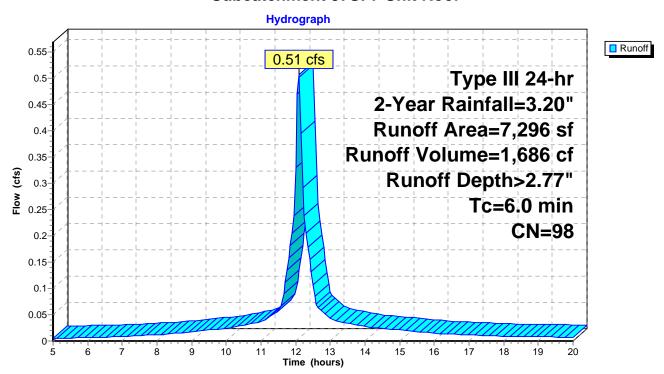
Summary for Subcatchment 37S: 7 Unit Roof

Runoff = 0.51 cfs @ 12.09 hrs, Volume= 1,686 cf, Depth> 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Α	rea (sf)	CN I	Description		
*		7,296	98	mpervious		
		7,296		100.00% Im	npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment 37S: 7 Unit Roof



Page 14

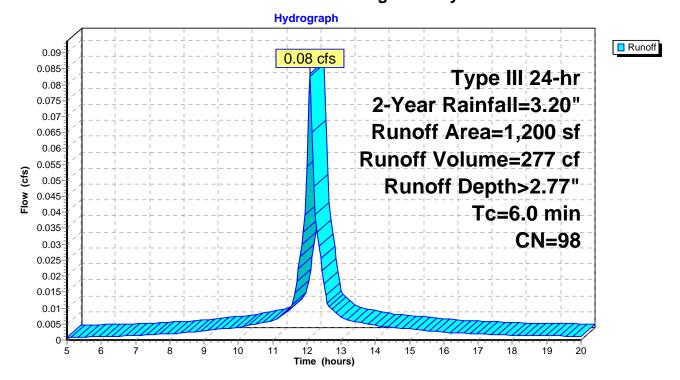
Summary for Subcatchment LOT 1: Single Family House

Runoff = 0.08 cfs @ 12.09 hrs, Volume= 277 cf, Depth> 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

_	Α	rea (sf)	CN	Description					
*		1,200	98	Impervious					
_		1,200	1,200 100.00% Impervious Area						
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 1: Single Family House



Page 15

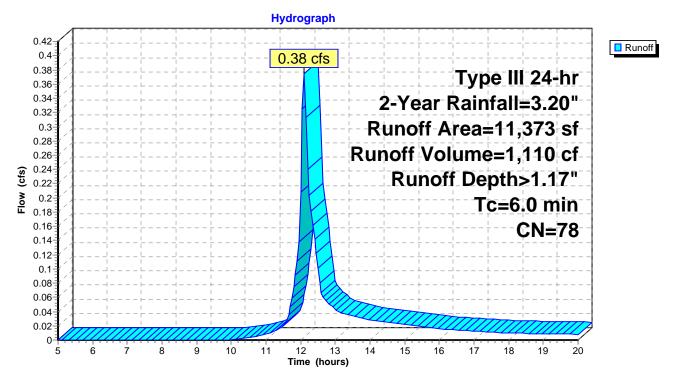
Summary for Subcatchment LOT 12: DUPLEX + YARD

Runoff = 0.38 cfs @ 12.10 hrs, Volume= 1,110 cf, Depth> 1.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Area (sf)	CN	Description							
*	4,929	98	Impervious							
	490	80	75% Grass cover, Good, HSG D							
	5,954	61	75% Grass cover, Good, HSG B							
	11,373	78	78 Weighted Average							
	6,444		56.66% Pervious Area							
	4,929		43.34% Impervious Area							
	T-	Ola	- Valacity Consoity Description							
	Tc Length	Slop								
<u>(r</u>	min) (feet)	(ft/	(ft) (ft/sec) (cfs)							
	6.0		Direct Entry.							

Subcatchment LOT 12: DUPLEX + YARD



Page 16

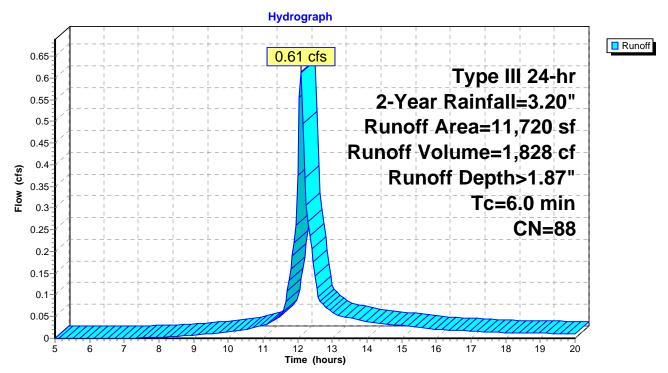
Summary for Subcatchment LOT 13: DUPLEX + YARD

Runoff = 0.61 cfs @ 12.09 hrs, Volume= 1,828 cf, Depth> 1.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

_	Α	rea (sf)	CN	Description								
*		4,929	98	Impervious	npervious							
_		6,791	80	>75% Gras	5% Grass cover, Good, HSG D							
		11,720	88	Weighted Average								
		6,791		57.94% Pei	57.94% Pervious Area							
		4,929		42.06% Imp	pervious Ar	ea						
	_		01	N/ 1 14	0 ''	5						
		-	Slope	,	Capacity	Description						
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)							
	6.0	•		•	•	Direct Entry						

Subcatchment LOT 13: DUPLEX + YARD



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 17

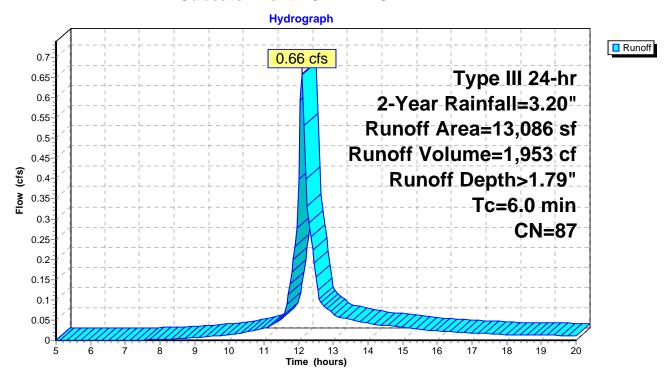
Summary for Subcatchment LOT 14: DUPLEX + YARD

Runoff = 0.66 cfs @ 12.09 hrs, Volume= 1,953 cf, Depth> 1.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Α	rea (sf)	CN	Description								
*		4,929	98	Impervious	npervious							
		8,157	80	>75% Gras	5% Grass cover, Good, HSG D							
		13,086	87	Weighted A	eighted Average							
		8,157		62.33% Per	62.33% Pervious Area							
		4,929		37.67% Imp	ervious Ar	rea						
	Тс	Length	Slop	e Velocity	Capacity	/ Description						
(n	nin)	(feet)	(ft/f	t) (ft/sec)	(cfs)							
	6.0					Direct Entry						

Subcatchment LOT 14: DUPLEX + YARD



Page 18

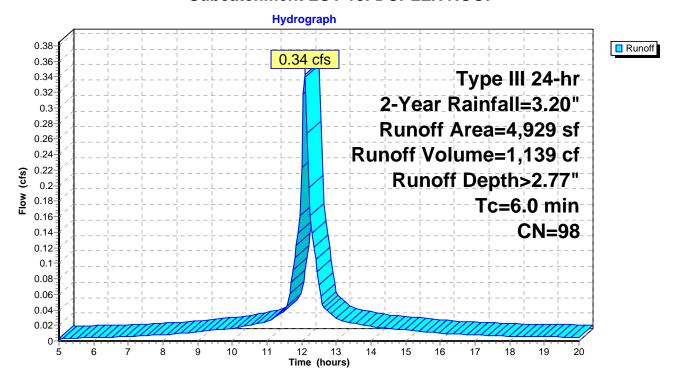
Summary for Subcatchment LOT 18: DUPLEX ROOF

Runoff = 0.34 cfs @ 12.09 hrs, Volume= 1,139 cf, Depth> 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Α	rea (sf)	CN	Description						
*		4,929	98	mpervious						
		4,929	,	100.00% Impervious Area						
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry,				

Subcatchment LOT 18: DUPLEX ROOF



Page 19

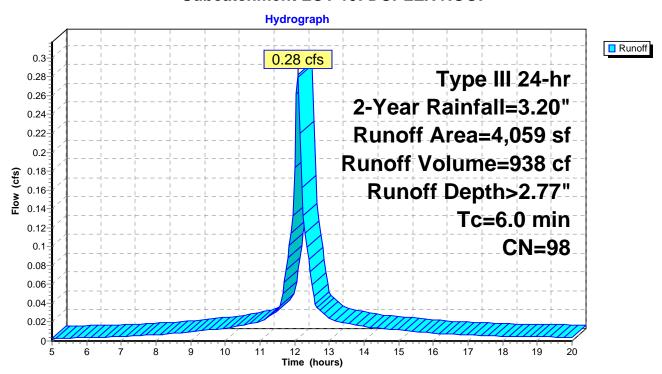
Summary for Subcatchment LOT 19: DUPLEX ROOF

Runoff = 0.28 cfs @ 12.09 hrs, Volume= 938 cf, Depth> 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Α	rea (sf)	CN I	Description		
*		4,059	98 I	mpervious		
		4,059	•	100.00% Im	npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment LOT 19: DUPLEX ROOF



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 20

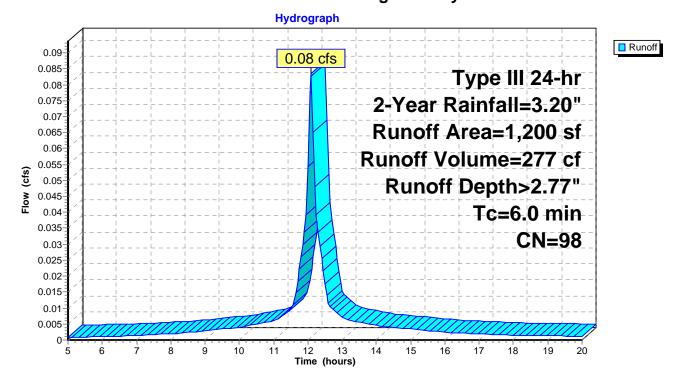
Summary for Subcatchment LOT 2: Single Family House

Runoff = 0.08 cfs @ 12.09 hrs, Volume= 277 cf, Depth> 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

_	Α	rea (sf)	CN	Description					
*		1,200	98	Impervious					
_		1,200	1,200 100.00% Impervious Area						
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 2: Single Family House



Page 21

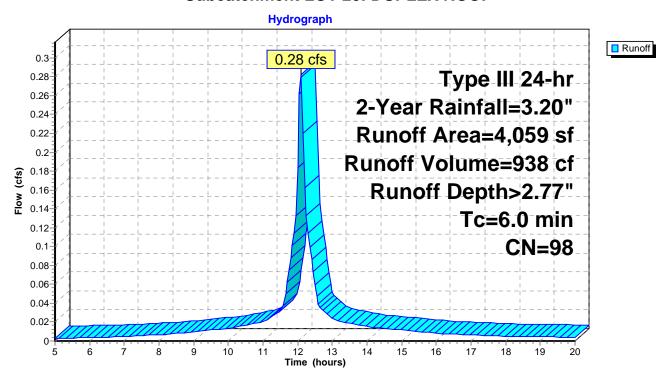
Summary for Subcatchment LOT 20: DUPLEX ROOF

Runoff = 0.28 cfs @ 12.09 hrs, Volume= 938 cf, Depth> 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Α	rea (sf)	CN I	Description					
*		4,059	98 I	mpervious					
		4,059	100.00% Impervious Area						
	Тс	Length	Slope	,	Capacity	•			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 20: DUPLEX ROOF



Page 22

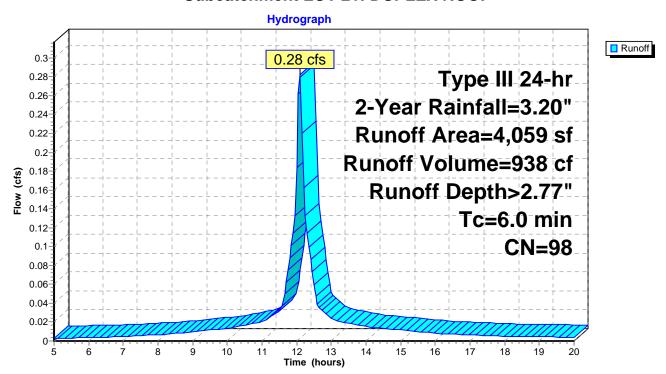
Summary for Subcatchment LOT 21: DUPLEX ROOF

Runoff = 0.28 cfs @ 12.09 hrs, Volume= 938 cf, Depth> 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Α	rea (sf)	CN I	Description		
*		4,059	98 I	mpervious		
		4,059	•	100.00% Im	npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment LOT 21: DUPLEX ROOF



Page 23

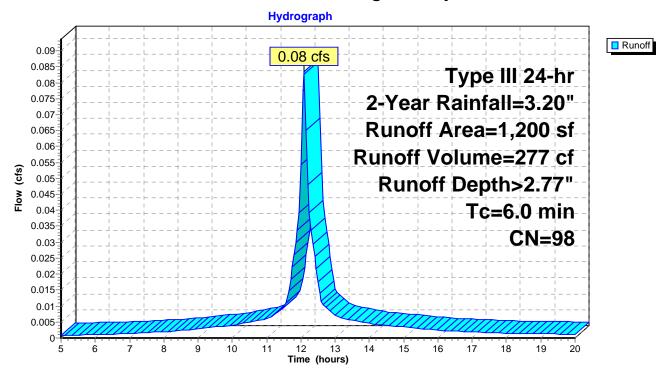
Summary for Subcatchment LOT 3: Single Family House

Runoff = 0.08 cfs @ 12.09 hrs, Volume= 277 cf, Depth> 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Α	rea (sf)	CN I	Description					
*		1,200	98	mpervious					
		1,200	1,200 100.00% Impervious Area						
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 3: Single Family House



Page 24

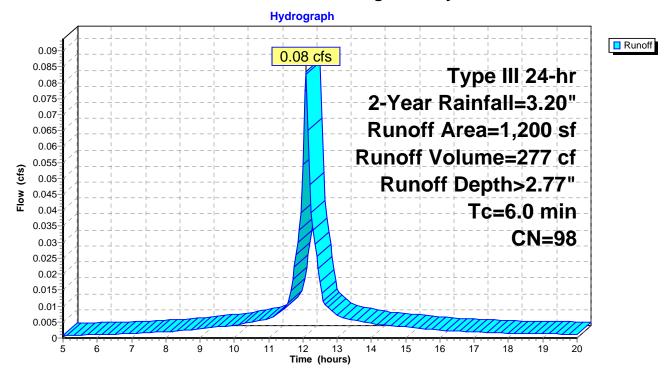
Summary for Subcatchment LOT 8: Single Family House

Runoff = 0.08 cfs @ 12.09 hrs, Volume= 277 cf, Depth> 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

_	Α	rea (sf)	CN	Description						
*		1,200	98	mpervious						
		1,200	,	100.00% Impervious Area						
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry,				

Subcatchment LOT 8: Single Family House



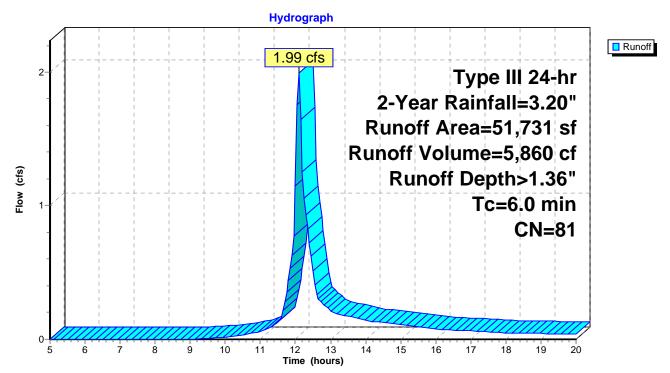
Summary for Subcatchment PRDA-1: TO BASIN D

Runoff = 1.99 cfs @ 12.10 hrs, Volume= 5,860 cf, Depth> 1.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Area (sf)	CN	Description									
*	25,896	98	Impervious	npervious								
	6,014	54	1/2 acre lots	/2 acre lots, 25% imp, HSG A								
	6,314	80	1/2 acre lots	/2 acre lots, 25% imp, HSG C								
	6,846	39	>75% Grass	75% Grass cover, Good, HSG A								
	462	74	>75% Grass	75% Grass cover, Good, HSG C								
	6,199	80	>75% Grass cover, Good, HSG D									
	51,731	81	81 Weighted Average									
	22,753		43.98% Per	vious Area	A							
	28,978		56.02% Imp	ervious Ar	ea							
	Tc Length	Slop	oe Velocity	Capacity	Description							
(m	in) (feet)	(ft/	ft) (ft/sec)	(cfs)								
6	6.0				Direct Entry,							

Subcatchment PRDA-1: TO BASIN D



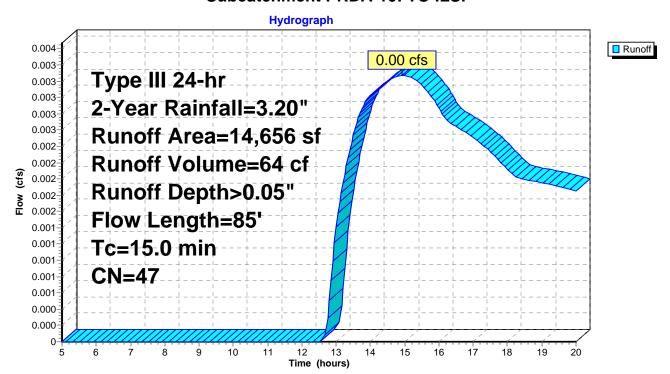
Summary for Subcatchment PRDA-10: TO ILSF

Runoff = 0.00 cfs @ 14.92 hrs, Volume= 64 cf, Depth> 0.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Α	rea (sf)	CN Description					
	1,166 39 >75% Grass cover, Good, HSG A							
2,000 80 >75% Grass cover, Good, HSG D								
8,622 30 Woods, Good, HSG A								
2,868 77 Woods, Good, HSG D								
14,656 47 Weighted Average								
	14,656 100.00% Pervious Area					a		
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	14.2	50	0.0140	0.06		Sheet Flow, AB		
						Woods: Light underbrush n= 0.400 P2= 3.20"		
	8.0	35	0.0220	0.74		Shallow Concentrated Flow, BC		
						Woodland Kv= 5.0 fps		
	15.0	85	Total			·		

Subcatchment PRDA-10: TO ILSF



Summary for Subcatchment PRDA-11: BASIN A DIRECT

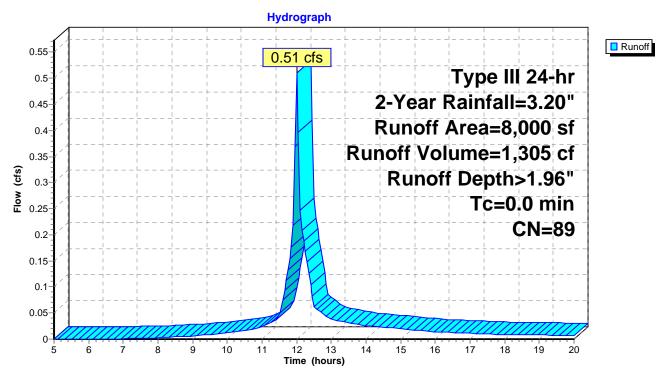
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 0.51 cfs @ 12.00 hrs, Volume= 1,305 cf, Depth> 1.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

Area (sf)	CN	Description	
3,983	98	Water Surface, HSG B	
4,017	80	>75% Grass cover, Good, HSG D	
8,000	89	Weighted Average	
4,017		50.21% Pervious Area	
3,983		49.79% Impervious Area	

Subcatchment PRDA-11: BASIN A DIRECT



Printed 3/9/2020 Page 28

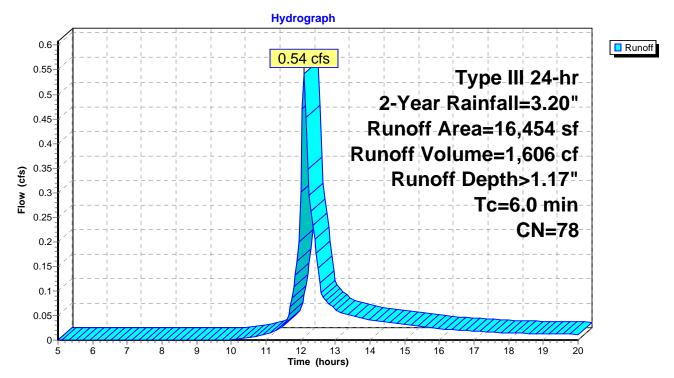
Summary for Subcatchment PRDA-12: BASIN B DIRECT

Runoff = 0.54 cfs @ 12.10 hrs, Volume= 1,606 cf, Depth> 1.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

Area (sf)	CN	Description						
7,423	98	Water Surface, HSG D						
5,050	80	>75% Grass cover, Good, HSG D						
3,981	39	>75% Grass cover, Good, HSG A						
16,454	78	Weighted Average						
9,031	· · · · · · · · · · · · · · · · · · ·							
7,423		45.11% Impervious Area						
Tc Length	Slop	pe Velocity Capacity Description						
(min) (feet)	(ft/	/ft) (ft/sec) (cfs)						
6.0		Direct Entry.						

Subcatchment PRDA-12: BASIN B DIRECT



Page 29

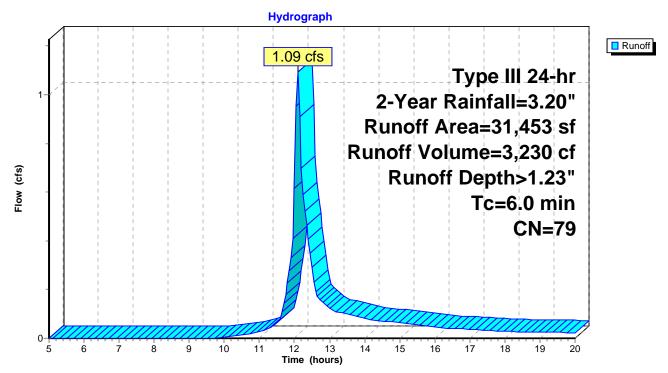
Summary for Subcatchment PRDA-13: BASIN C DIRECT

Runoff = 1.09 cfs @ 12.10 hrs, Volume= 3,230 cf, Depth> 1.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

A	rea (sf)	CN	Description						
	15,825	61	>75% Grass cover, Good, HSG B						
	15,628	98	Water Surface, HSG B						
	31,453	453 79 Weighted Average							
	15,825 50.31% Pervious Area								
	15,628		49.69% Impervious Area						
Тс	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft	,	(cfs)	•				
6.0		·			Direct Entry,				

Subcatchment PRDA-13: BASIN C DIRECT



Page 30

Summary for Subcatchment PRDA-2: TO BASIN A

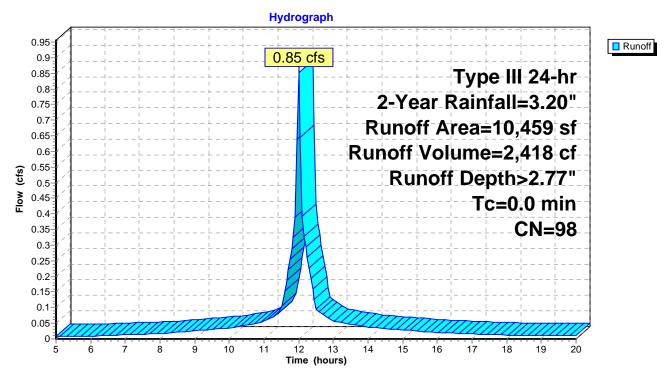
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 0.85 cfs @ 12.00 hrs, Volume= 2,418 cf, Depth> 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Area (sf)	CN	Description
*	10,459	98	IMPERVIOUS
	10 459		100 00% Impervious Area

Subcatchment PRDA-2: TO BASIN A



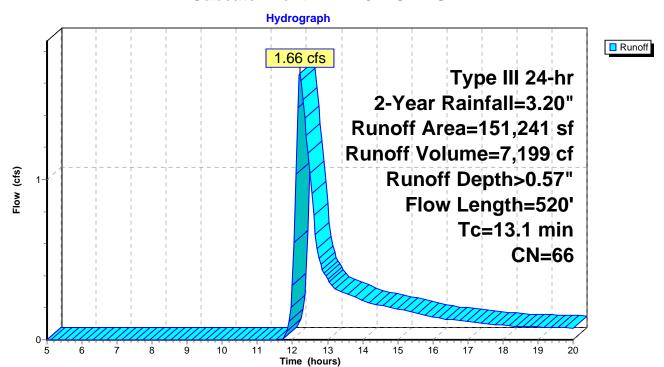
Summary for Subcatchment PRDA-3: TO BASIN B

Runoff = 1.66 cfs @ 12.22 hrs, Volume= 7,199 cf, Depth> 0.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Α	rea (sf)	CN E	escription				
*		40,248	98 II	MPERVIO	JS			
		14,787	98 F	Roofs, HSG	βA			
		77,313	39 >	75% Gras	s cover, Go	ood, HSG A		
		92	61 >	75% Gras	s cover, Go	ood, HSG B		
		18,801	80 >	75% Gras	s cover, Go	ood, HSG D		
	1	51,241		Veighted A				
		96,206	_		vious Area			
		55,035	3	36.39% Impervious Area				
	_		01			B 1.0		
	Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	6.3	50	0.0380	0.13		Sheet Flow,		
						Grass: Dense n= 0.240 P2= 3.20"		
	6.5	406	0.0220	1.04		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	0.3	64	0.0360	3.85		Shallow Concentrated Flow,		
						Paved Kv= 20.3 fps		
	13.1	520	Total					

Subcatchment PRDA-3: TO BASIN B



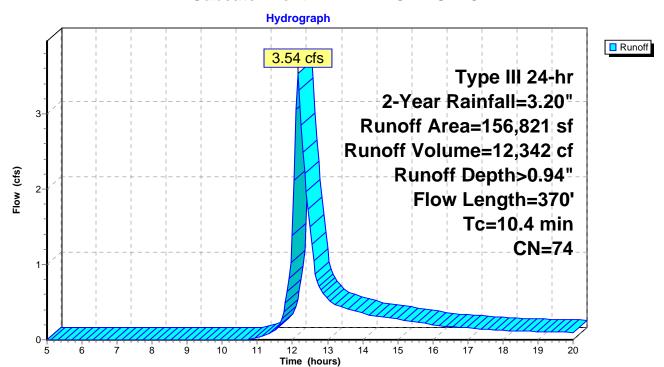
Summary for Subcatchment PRDA-4: TO BASIN C

Runoff = 3.54 cfs @ 12.16 hrs, Volume= 12,342 cf, Depth> 0.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Α	rea (sf)	CN E	escription				
*		70,874	98 lı	npervious				
		4,929	98 F	oofs, HSC	ВВ			
		35,430	39 >	75% Gras	s cover, Go	ood, HSG A		
		44,834	61 >	75% Gras	s cover, Go	ood, HSG B		
		754	80 >	75% Gras	s cover, Go	ood, HSG D		
	1	56,821	74 V	Veighted A	verage			
		81,018	5	51.66% Pervious Area				
		75,803	4	48.34% Impervious Area				
	_				_			
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	6.8	50	0.0320	0.12		Sheet Flow,		
						Grass: Dense n= 0.240 P2= 3.20"		
	3.1	217	0.0280	1.17		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	0.5	103	0.0270	3.34		Shallow Concentrated Flow,		
						Paved Kv= 20.3 fps		
	10.4	370	Total					

Subcatchment PRDA-4: TO BASIN C



Printed 3/9/2020 Page 33

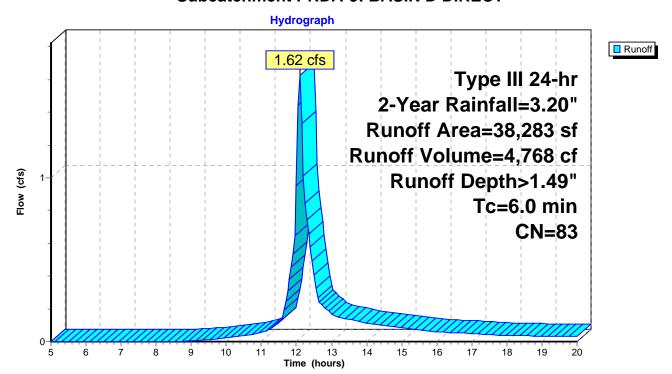
Summary for Subcatchment PRDA-5: BASIN D DIRECT

Runoff = 1.62 cfs @ 12.09 hrs, Volume= 4,768 cf, Depth> 1.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	rea (sf)	CN	CN Description				
*	1,132	98	Impervious				
	7,697	80	1/2 acre lots	s, 25% imp	o, HSG C		
	374	39	>75% Gras	s cover, Go	ood, HSG A		
	7,237	74	>75% Gras	s cover, Go	ood, HSG C		
	6,781	80	>75% Gras	s cover, Go	ood, HSG D		
	9,227	98	Water Surfa	ace, HSG A	A		
	4,080	77	Woods, Good, HSG D				
	1,323	70	Woods, Go	od, HSG C			
	432	96	Gravel surfa	ace, HSG C	C		
	38,283	83	Weighted A	verage			
	26,000		67.91% Per	vious Area	a		
	12,283		32.09% Impervious Area				
Tc	Length	Slop	e Velocity	Capacity	Description		
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)			
6.0					Direct Entry,		

Subcatchment PRDA-5: BASIN D DIRECT



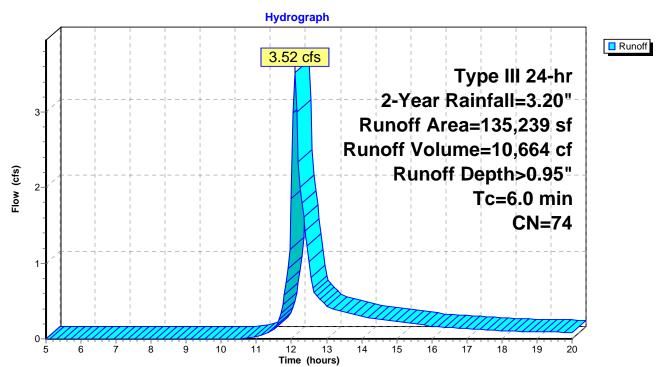
Summary for Subcatchment PRDA-6: TO BASIN E

Runoff = 3.52 cfs @ 12.10 hrs, Volume= 10,664 cf, Depth> 0.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	rea (sf)	CN	CN Description				
*	19,869	98	IMPERVIO	JS			
	57,855	80	1/2 acre lots	s, 25% imp	o, HSG C		
	2,327	54	1/2 acre lot	s, 25% imp	o, HSG A		
	3,600	98	Roofs, HSG	βA			
	27,519	39	>75% Gras	s cover, Go	ood, HSG A		
	16,283	74	74 >75% Grass cover, Good, HSG C				
	1,810	80	80 >75% Grass cover, Good, HSG D				
	5,976	98	Water Surfa				
•	135,239	74	Weighted A	verage			
	90,749		67.10% Pei	vious Area	a e e e e e e e e e e e e e e e e e e e		
44,491 32.90% Impervious Are				ervious Ar	rea		
_		01		•			
Tc	Length	Slop		Capacity	Description		
<u>(min)</u>	(feet)	(ft/f	t) (ft/sec)	(cfs)			
6.0					Direct Entry,		

Subcatchment PRDA-6: TO BASIN E



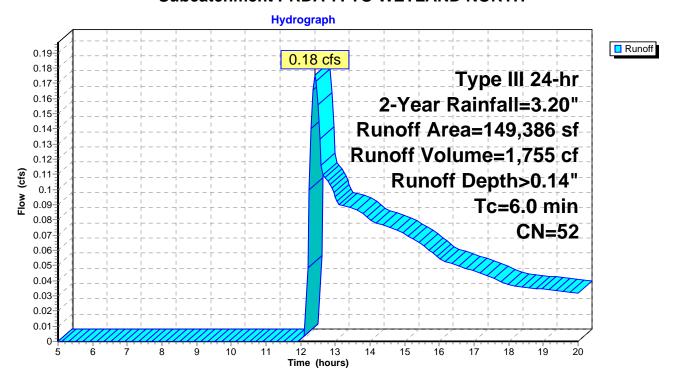
Summary for Subcatchment PRDA-7: TO WETLAND NORTH

Runoff = 0.18 cfs @ 12.41 hrs, Volume= 1,755 cf, Depth> 0.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Area (sf)	CN	N Description					
	2,951	80	1/2 acre lots, 25% imp, HSG C					
	8,254	54	1/2 acre lots, 25% imp, HSG A					
	52,416	39	>75% Grass cover, Good, HSG A					
	27,465	61	>75% Grass cover, Good, HSG B					
	538	74	>75% Grass cover, Good, HSG C					
	9,934	80	>75% Grass cover, Good, HSG D					
	22,059	30	Woods, Good, HSG A					
	14,418	70	Woods, Good, HSG C					
	9,973	77	Woods, Good, HSG D					
*	1,378	98	impervious					
•	149,386	52	Weighted Average					
•	145,207		97.20% Pervious Area					
	4,179 2.80% Impervious Area							
Tc	Length	Slop	pe Velocity Capacity Description					
(min)	(feet)	(ft/1						
6.0								

Subcatchment PRDA-7: TO WETLAND NORTH



Page 36

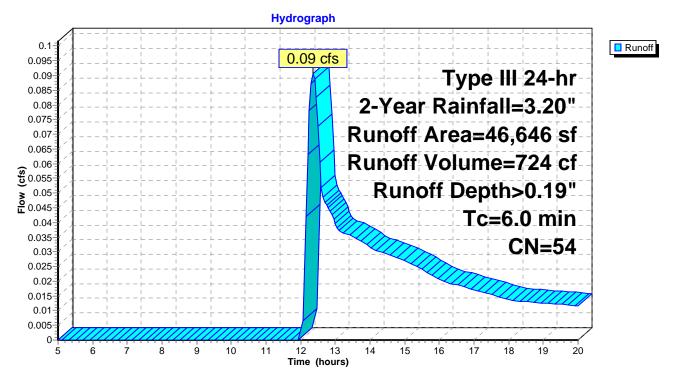
Summary for Subcatchment PRDA-8: TO WETLAND SOUTHEAST

Runoff = 0.09 cfs @ 12.36 hrs, Volume= 724 cf, Depth> 0.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

Area (sf)	CN	Description					
21,559	39	>75% Grass cover, Good, HSG A					
7,434	80	>75% Grass cover, Good, HSG D					
12,118	77	Woods, Good, HSG D					
5,535	30	Woods, Good, HSG A					
46,646	54	Weighted Average					
46,646		100.00% Pervious Area					
Tc Length	Slop	, , , ,					
(min) (feet)	(ft/	/ft) (ft/sec) (cfs)					
6.0		Direct Entry.					

Subcatchment PRDA-8: TO WETLAND SOUTHEAST



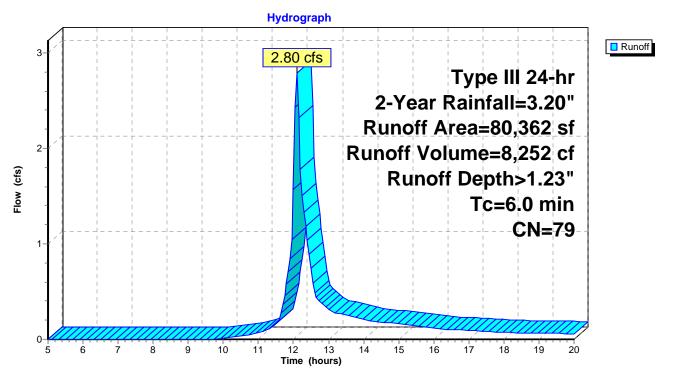
Summary for Subcatchment PRDA-9: TO WETLAND SOUTH

Runoff = 2.80 cfs @ 12.10 hrs, Volume= 8,252 cf, Depth> 1.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.20"

	Area (sf)	CN	Description					
	274	39	>75% Grass cover, Good, HSG A					
	20,073	80	>75% Grass	s cover, Go	Good, HSG D			
	7,904	70	Woods, Go	od, HSG C				
	34,217	77	Woods, Go	od, HSG D				
	11,544	80	1/2 acre lots, 25% imp, HSG C					
*	6,350	98	IMPERVIOUS					
	80,362	79	Weighted Average					
	71,126		88.51% Pervious Area					
	9,236		11.49% Impervious Area					
T	c Length	Slop	e Velocity	Capacity	Description			
(min) (feet)	(ft/f	t) (ft/sec)	(cfs)				
6.0	0				Direct Entry,			

Subcatchment PRDA-9: TO WETLAND SOUTH



Page 38

Summary for Reach 1R: ILSF

[40] Hint: Not Described (Outflow=Inflow)

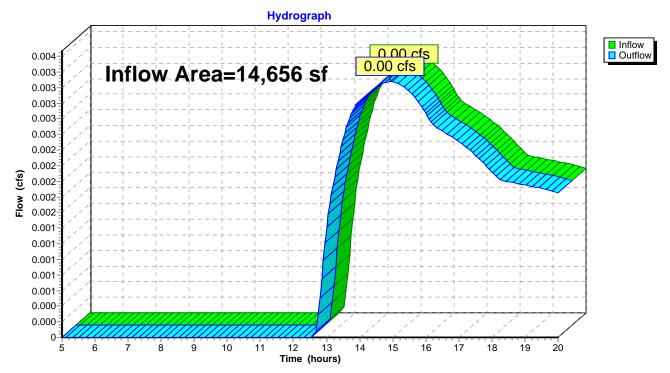
Inflow Area = 14,656 sf, 0.00% Impervious, Inflow Depth > 0.05" for 2-Year event

Inflow = 0.00 cfs @ 14.92 hrs, Volume= 64 cf

Outflow = 0.00 cfs @ 14.92 hrs, Volume= 64 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 1R: ILSF



Page 39

Summary for Reach 14R: WETLAND SOUTH

[40] Hint: Not Described (Outflow=Inflow)

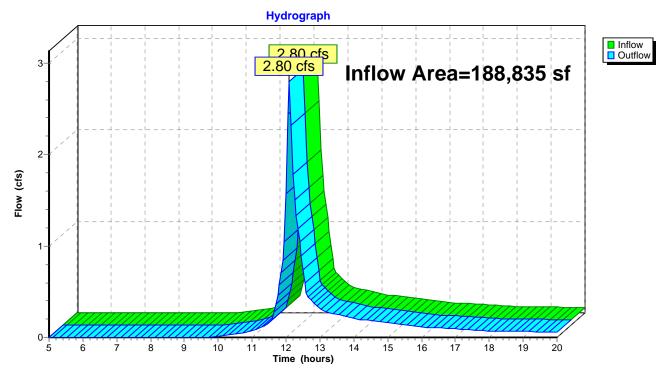
Inflow Area = 188,835 sf, 34.39% Impervious, Inflow Depth > 0.52" for 2-Year event

Inflow = 2.80 cfs @ 12.10 hrs, Volume= 8,252 cf

Outflow = 2.80 cfs @ 12.10 hrs, Volume= 8,252 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 14R: WETLAND SOUTH



Page 40

Summary for Reach 15R: WETLAND SOUTHEAST

[40] Hint: Not Described (Outflow=Inflow)

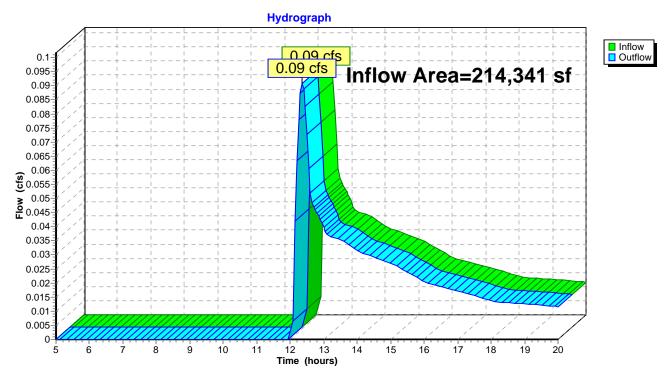
Inflow Area = 214,341 sf, 29.14% Impervious, Inflow Depth > 0.04" for 2-Year event

Inflow = 0.09 cfs @ 12.36 hrs, Volume= 724 cf

Outflow = 0.09 cfs @ 12.36 hrs, Volume= 724 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 15R: WETLAND SOUTHEAST



Page 41

Summary for Reach 21R: WETLAND NORTH

[40] Hint: Not Described (Outflow=Inflow)

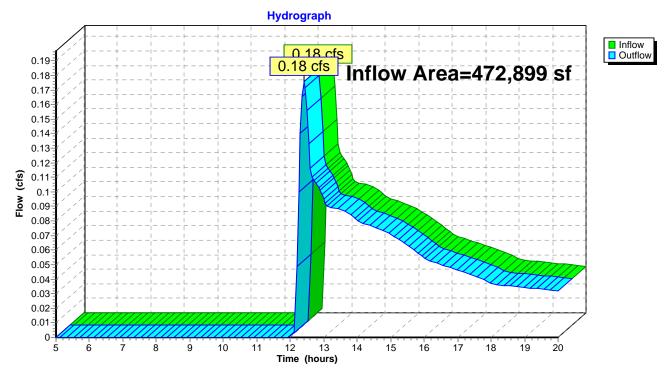
Inflow Area = 472,899 sf, 29.63% Impervious, Inflow Depth > 0.04" for 2-Year event

Inflow = 0.18 cfs @ 12.41 hrs, Volume= 1,755 cf

Outflow = 0.18 cfs @ 12.41 hrs, Volume= 1,755 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 21R: WETLAND NORTH



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC Printed 3/9/2020

Page 42

Summary for Pond 1P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 10,448 sf,100.00% Impervious, Inflow Depth > 2.77" for 2-Year event

Inflow = 0.73 cfs @ 12.09 hrs, Volume= 2,415 cf

Outflow = 0.09 cfs @ 12.63 hrs, Volume= 2,412 cf, Atten= 87%, Lag= 32.4 min

Discarded = 0.09 cfs @ 12.63 hrs, Volume= 2,412 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.88' @ 12.63 hrs Surf.Area= 1,531 sf Storage= 801 cf

Plug-Flow detention time= 58.6 min calculated for 2,412 cf (100% of inflow)

Center-of-Mass det. time= 58.0 min (796.5 - 738.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,317 cf	20.83'W x 73.50'L x 3.54'H Field A
			5,423 cf Overall - 2,131 cf Embedded = 3,292 cf x 40.0% Voids
#2A	0.50'	2,131 cf	Cultec R-330XLHD x 40 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 4 rows
•			

3,448 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.09 cfs @ 12.63 hrs HW=0.88' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.09 cfs)

Pond 1P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

10 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 71.50' Row Length +12.0" End Stone x 2 = 73.50' Base Length

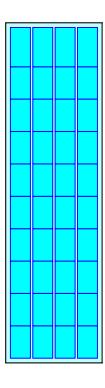
4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

40 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 2,131.0 cf Chamber Storage

5,423.2 cf Field - 2,131.0 cf Chambers = 3,292.2 cf Stone x 40.0% Voids = 1,316.9 cf Stone Storage

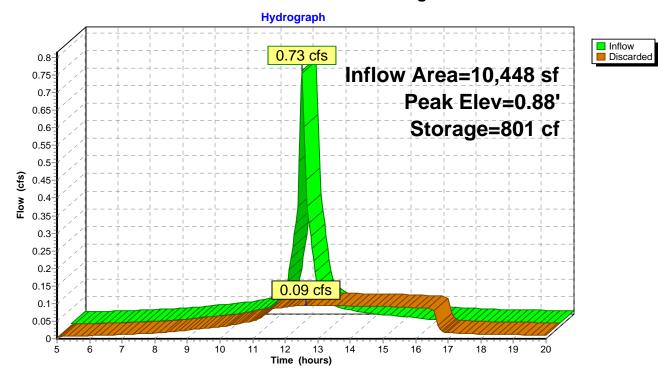
Chamber Storage + Stone Storage = 3,447.9 cf = 0.079 af Overall Storage Efficiency = 63.6% Overall System Size = 73.50' x 20.83' x 3.54'

40 Chambers 200.9 cy Field 121.9 cy Stone





Pond 1P: Roof Recharge



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC Printed 3/9/2020

Page 45

Summary for Pond 2P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 13,061 sf,100.00% Impervious, Inflow Depth > 2.77" for 2-Year event

Inflow = 0.91 cfs @ 12.09 hrs, Volume= 3,019 cf

Outflow = 0.06 cfs @ 13.61 hrs, Volume= 2,385 cf, Atten= 94%, Lag= 91.4 min

Discarded = 0.06 cfs @ 13.61 hrs, Volume= 2,385 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.98' @ 13.61 hrs Surf.Area= 2,246 sf Storage= 1,366 cf

Plug-Flow detention time= 161.1 min calculated for 2,376 cf (79% of inflow)

Center-of-Mass det. time= 105.7 min (844.3 - 738.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,907 cf	25.67'W x 87.50'L x 3.54'H Field A
			7,954 cf Overall - 3,185 cf Embedded = 4,769 cf \times 40.0% Voids
#2A	0.50'	3,185 cf	Cultec R-330XLHD x 60 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 5 rows
•		/	

5,093 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.06 cfs @ 13.61 hrs HW=0.98' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.06 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 46

Pond 2P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 5 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

12 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 85.50' Row Length +12.0" End Stone x 2 = 87.50' Base Length

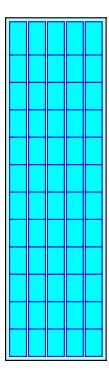
5 Rows x 52.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 25.67' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

60 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 5 Rows = 3,185.3 cf Chamber Storage

7,954.0 cf Field - 3,185.3 cf Chambers = 4,768.7 cf Stone x 40.0% Voids = 1,907.5 cf Stone Storage

Chamber Storage + Stone Storage = 5,092.8 cf = 0.117 af Overall Storage Efficiency = 64.0% Overall System Size = 87.50' x 25.67' x 3.54'

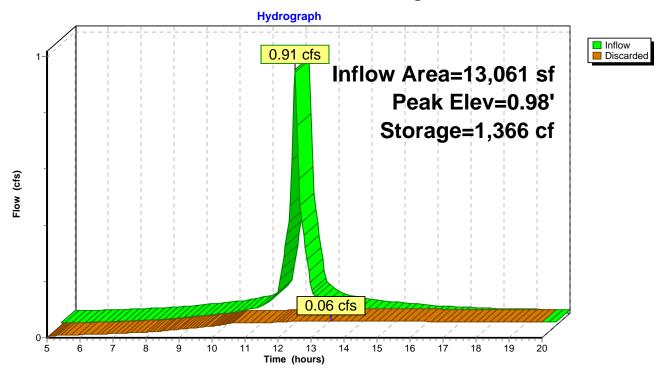
60 Chambers 294.6 cy Field 176.6 cy Stone





Page 47

Pond 2P: Roof Recharge



Type III 24-hr 2-Year Rainfall=3.20" Printed 3/9/2020

Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 48

Summary for Pond 3P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 13,061 sf,100.00% Impervious, Inflow Depth > 2.77" for 2-Year event

Inflow = 0.91 cfs @ 12.09 hrs, Volume= 3,019 cf

Outflow = 0.06 cfs @ 13.61 hrs, Volume= 2,385 cf, Atten= 94%, Lag= 91.4 min

Discarded = 0.06 cfs @ 13.61 hrs, Volume= 2,385 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.98' @ 13.61 hrs Surf.Area= 2,246 sf Storage= 1,366 cf

Plug-Flow detention time= 161.1 min calculated for 2,376 cf (79% of inflow)

Center-of-Mass det. time= 105.7 min (844.3 - 738.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,907 cf	25.67'W x 87.50'L x 3.54'H Field A
			7,954 cf Overall - 3,185 cf Embedded = 4,769 cf \times 40.0% Voids
#2A	0.50'	3,185 cf	Cultec R-330XLHD x 60 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 5 rows
•		/	

5,093 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.06 cfs @ 13.61 hrs HW=0.98' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.06 cfs)

Printed 3/9/2020

Page 49

Pond 3P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 5 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

12 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 85.50' Row Length +12.0" End Stone x 2 = 87.50' Base Length

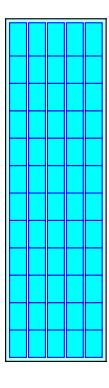
5 Rows x 52.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 25.67' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

60 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 5 Rows = 3,185.3 cf Chamber Storage

7,954.0 cf Field - 3,185.3 cf Chambers = 4,768.7 cf Stone x 40.0% Voids = 1,907.5 cf Stone Storage

Chamber Storage + Stone Storage = 5,092.8 cf = 0.117 af Overall Storage Efficiency = 64.0% Overall System Size = 87.50' x 25.67' x 3.54'

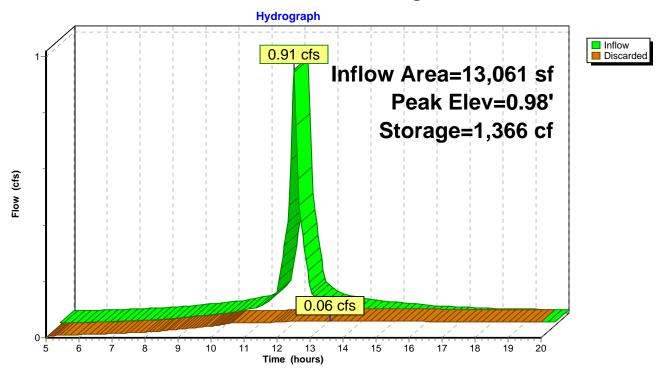
60 Chambers 294.6 cy Field 176.6 cy Stone





Page 50

Pond 3P: Roof Recharge



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Printed 3/9/2020
Page 51

Summary for Pond 4P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 7,296 sf,100.00% Impervious, Inflow Depth > 2.77" for 2-Year event

Inflow = 0.51 cfs @ 12.09 hrs, Volume= 1,686 cf

Outflow = 0.03 cfs @ 13.57 hrs, Volume= 1,341 cf, Atten= 93%, Lag= 89.1 min

Discarded = 0.03 cfs @ 13.57 hrs, Volume= 1,341 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.00' @ 13.57 hrs Surf.Area= 1,240 sf Storage= 762 cf

Plug-Flow detention time= 161.0 min calculated for 1,336 cf (79% of inflow)

Center-of-Mass det. time= 106.4 min (845.0 - 738.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,071 cf	20.83'W x 59.50'L x 3.54'H Field A
			4,390 cf Overall - 1,714 cf Embedded = 2,676 cf x 40.0% Voids
#2A	0.50'	1,714 cf	Cultec R-330XLHD x 32 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 4 rows
•			

2,784 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.03 cfs @ 13.57 hrs HW=1.00' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.03 cfs)

Pond 4P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

8 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 57.50' Row Length +12.0" End Stone x 2 = 59.50' Base Length

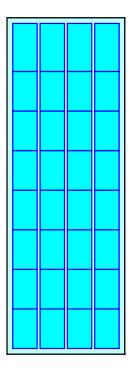
4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

32 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 1,713.7 cf Chamber Storage

4,390.2 cf Field - 1,713.7 cf Chambers = 2,676.5 cf Stone x 40.0% Voids = 1,070.6 cf Stone Storage

Chamber Storage + Stone Storage = 2,784.3 cf = 0.064 af Overall Storage Efficiency = 63.4% Overall System Size = 59.50' x 20.83' x 3.54'

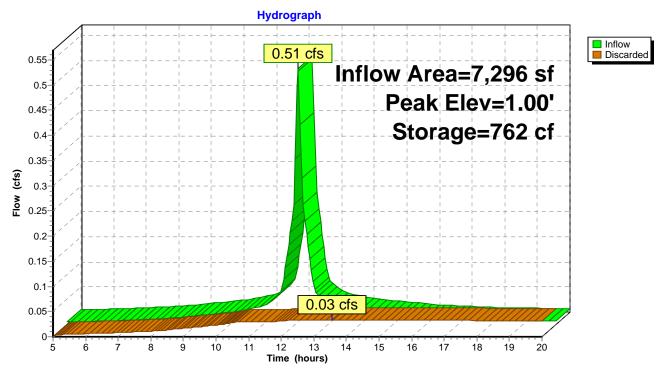
32 Chambers 162.6 cy Field 99.1 cy Stone





Page 53

Pond 4P: Roof Recharge



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC Printed 3/9/2020

Page 54

Summary for Pond 5P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 2.77" for 2-Year event

Inflow = 0.08 cfs @ 12.09 hrs, Volume= 277 cf

Outflow = 0.01 cfs @ 13.10 hrs, Volume= 244 cf, Atten= 92%, Lag= 60.6 min

Discarded = 0.01 cfs @ 13.10 hrs, Volume= 244 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.04' @ 13.10 hrs Surf.Area= 200 sf Storage= 120 cf

Plug-Flow detention time= 152.0 min calculated for 244 cf (88% of inflow)

Center-of-Mass det. time= 113.7 min (852.3 - 738.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 13.10 hrs HW=1.04' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.01 cfs)

Page 55

Pond 5P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

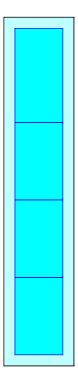
Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

- 4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length
- 1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height
- 4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

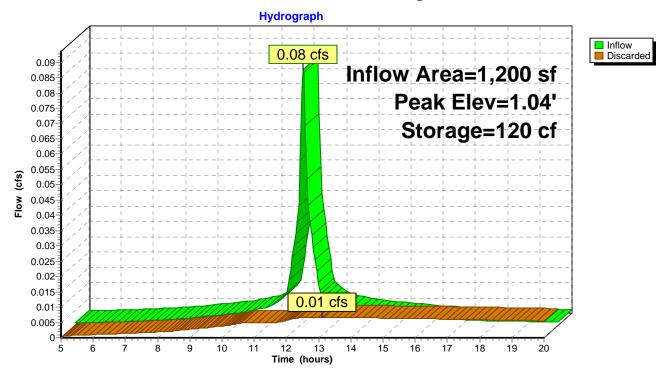
4 Chambers 26.2 cy Field 18.0 cy Stone





Page 56

Pond 5P: Roof Recharge



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Printed 3/9/2020
Page 57

Summary for Pond 6P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 4,059 sf,100.00% Impervious, Inflow Depth > 2.77" for 2-Year event

Inflow = 0.28 cfs @ 12.09 hrs, Volume= 938 cf

Outflow = 0.02 cfs @ 12.99 hrs, Volume= 924 cf, Atten= 91%, Lag= 54.4 min

Discarded = 0.02 cfs @ 12.99 hrs, Volume= 924 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.77' @ 12.99 hrs Surf.Area= 899 sf Storage= 378 cf

Plug-Flow detention time= 127.5 min calculated for 923 cf (98% of inflow)

Center-of-Mass det. time= 120.4 min (858.9 - 738.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	806 cf	11.17'W x 80.50'L x 3.54'H Field A
			$3,184 \text{ cf Overall - } 1,170 \text{ cf Embedded = } 2,014 \text{ cf } \times 40.0\% \text{ Voids}$
#2A	0.50'	1,170 cf	Cultec R-330XLHD x 22 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
	<u> </u>		

1,975 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.02 cfs @ 12.99 hrs HW=0.77' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.02 cfs)

Pond 6P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

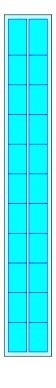
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

22 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,169.8 cf Chamber Storage

3,183.7 cf Field - 1,169.8 cf Chambers = 2,013.9 cf Stone x 40.0% Voids = 805.5 cf Stone Storage

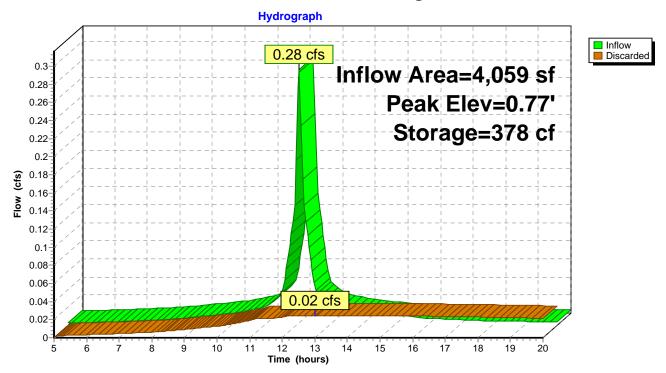
Chamber Storage + Stone Storage = 1,975.3 cf = 0.045 af Overall Storage Efficiency = 62.0% Overall System Size = 80.50' x 11.17' x 3.54'

22 Chambers 117.9 cy Field 74.6 cy Stone





Pond 6P: Roof Recharge



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Printed 3/9/2020
Page 60

Summary for Pond 7P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 4,929 sf,100.00% Impervious, Inflow Depth > 2.77" for 2-Year event

Inflow = 0.34 cfs @ 12.09 hrs, Volume= 1,139 cf

Outflow = 0.03 cfs @ 13.24 hrs, Volume= 987 cf, Atten= 93%, Lag= 69.0 min

Discarded = 0.03 cfs @ 13.24 hrs, Volume= 987 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.93' @ 13.24 hrs Surf.Area= 899 sf Storage= 493 cf

Plug-Flow detention time= 154.2 min calculated for 983 cf (86% of inflow)

Center-of-Mass det. time= 112.6 min (851.2 - 738.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	806 cf	11.17'W x 80.50'L x 3.54'H Field A
			$3,184 \text{ cf Overall - } 1,170 \text{ cf Embedded = } 2,014 \text{ cf } \times 40.0\% \text{ Voids}$
#2A	0.50'	1,170 cf	Cultec R-330XLHD x 22 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
	<u> </u>		

1,975 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.03 cfs @ 13.24 hrs HW=0.93' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.03 cfs)

Pond 7P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

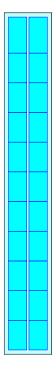
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

22 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,169.8 cf Chamber Storage

3,183.7 cf Field - 1,169.8 cf Chambers = 2,013.9 cf Stone x 40.0% Voids = 805.5 cf Stone Storage

Chamber Storage + Stone Storage = 1,975.3 cf = 0.045 af Overall Storage Efficiency = 62.0% Overall System Size = 80.50' x 11.17' x 3.54'

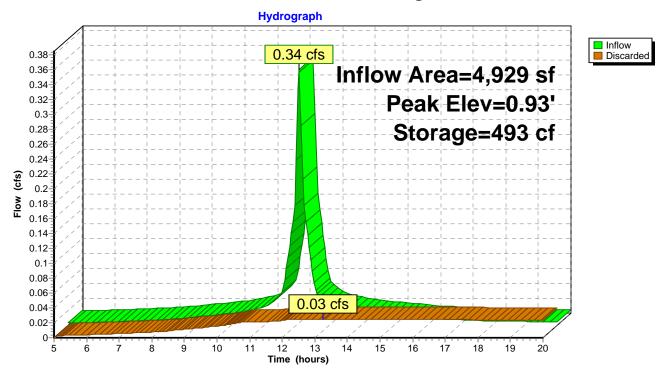
22 Chambers 117.9 cy Field 74.6 cy Stone





Page 62

Pond 7P: Roof Recharge



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP Printed 3/9/2020

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 63

Summary for Pond 8P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 2.77" for 2-Year event

Inflow = 0.08 cfs @ 12.09 hrs, Volume= 277 cf

Outflow = 0.01 cfs @ 13.10 hrs, Volume= 244 cf, Atten= 92%, Lag= 60.6 min

Discarded = 0.01 cfs @ 13.10 hrs, Volume= 244 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.04' @ 13.10 hrs Surf.Area= 200 sf Storage= 120 cf

Plug-Flow detention time= 152.0 min calculated for 244 cf (88% of inflow)

Center-of-Mass det. time= 113.7 min (852.3 - 738.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 13.10 hrs HW=1.04' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.01 cfs)

Page 64

Pond 8P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

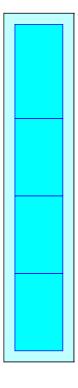
Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

- 4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length
- 1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height
- 4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

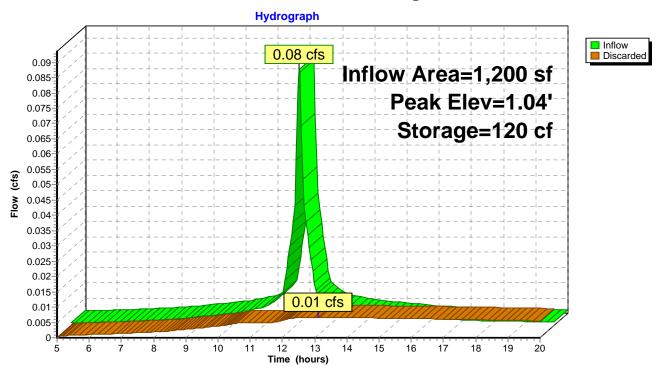
4 Chambers 26.2 cy Field 18.0 cy Stone





Page 65

Pond 8P: Roof Recharge



Type III 24-hr 2-Year Rainfall=3.20" Printed 3/9/2020

Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 66

Summary for Pond 9P: BASIN E

Inflow Area = 135,239 sf, 32.90% Impervious, Inflow Depth > 0.95" for 2-Year event
Inflow = 3.52 cfs @ 12.10 hrs, Volume= 10,664 cf
Outflow = 0.28 cfs @ 14.22 hrs, Volume= 8,057 cf, Atten= 92%, Lag= 127.1 min
Discarded = 0.28 cfs @ 14.22 hrs, Volume= 8,057 cf
Primary = 0.00 cfs @ 5.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 238.98' @ 14.22 hrs Surf.Area= 4,890 sf Storage= 5,062 cf

Plug-Flow detention time= 189.2 min calculated for 8,030 cf (75% of inflow)

Center-of-Mass det. time= 127.0 min (945.3 - 818.3)

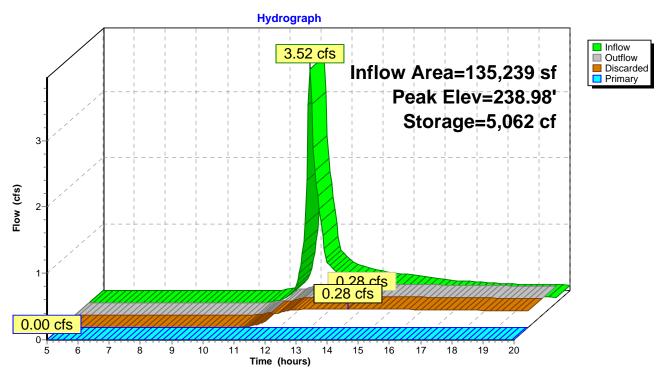
Volume	Inver	t Avail	.Storage	Storage Description	on		
#1	#1 237.80' 24,84		24,841 cf	Custom Stage Da	ata (Irregular) Liste	ed below (Recalc)	
Elevatio		Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>	
237.8	30	3,709	322.6	0	0	3,709	
238.0	00	3,904	326.4	761	761	3,916	
239.0	00	4,911	345.3	4,398	5,159	4,980	
240.0	00	5,976	364.3	5,435	10,594	6,110	
241.0	00	7,097	383.1	6,528	17,122	7,289	
241.9	90	10,147	430.2	7,719	24,841	10,360	
Device	Routing	Inv	ert Outle	et Devices			
#1	Discarded	scarded 237.80' 2.41		0 in/hr Exfiltration	over Wetted area	a	
#2	Primary			' long x 18.0' brea	dth Broad-Creste	ed Rectangular Weir	
	•		Head	d (feet) 0.20 0.40	0.60 0.80 1.00	1.20 1.40 1.60	
			Coef	f. (English) 2.68 2	.70 2.70 2.64 2.6	3 2.64 2.64 2.63	

Discarded OutFlow Max=0.28 cfs @ 14.22 hrs HW=238.98' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.28 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=237.80' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 67

Pond 9P: BASIN E



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Printed 3/9/2020
Page 68

Summary for Pond 10P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 2.77" for 2-Year event

Inflow = 0.08 cfs @ 12.09 hrs, Volume= 277 cf

Outflow = 0.01 cfs @ 13.10 hrs, Volume= 244 cf, Atten= 92%, Lag= 60.6 min

Discarded = 0.01 cfs @ 13.10 hrs, Volume= 244 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.04' @ 13.10 hrs Surf.Area= 200 sf Storage= 120 cf

Plug-Flow detention time= 152.0 min calculated for 244 cf (88% of inflow)

Center-of-Mass det. time= 113.7 min (852.3 - 738.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 13.10 hrs HW=1.04' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.01 cfs)

Page 69

Pond 10P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length

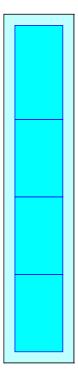
1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

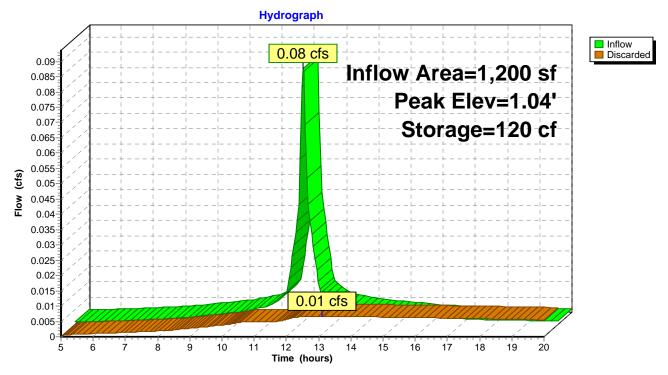
Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

4 Chambers 26.2 cy Field 18.0 cy Stone





Pond 10P: Roof Recharge



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Printed 3/9/2020
Page 71

Summary for Pond 11P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 4,059 sf,100.00% Impervious, Inflow Depth > 2.77" for 2-Year event

Inflow = 0.28 cfs @ 12.09 hrs, Volume= 938 cf

Outflow = 0.02 cfs @ 12.99 hrs, Volume= 924 cf, Atten= 91%, Lag= 54.4 min

Discarded = 0.02 cfs @ 12.99 hrs, Volume= 924 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.77' @ 12.99 hrs Surf.Area= 899 sf Storage= 378 cf

Plug-Flow detention time= 127.5 min calculated for 923 cf (98% of inflow)

Center-of-Mass det. time= 120.4 min (858.9 - 738.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	806 cf	11.17'W x 80.50'L x 3.54'H Field A
			3,184 cf Overall - 1,170 cf Embedded = 2,014 cf x 40.0% Voids
#2A	0.50'	1,170 cf	Cultec R-330XLHD x 22 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
	<u> </u>		

1,975 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.02 cfs @ 12.99 hrs HW=0.77' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.02 cfs)

Pond 11P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

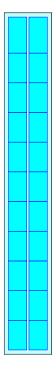
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

22 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,169.8 cf Chamber Storage

3,183.7 cf Field - 1,169.8 cf Chambers = 2,013.9 cf Stone x 40.0% Voids = 805.5 cf Stone Storage

Chamber Storage + Stone Storage = 1,975.3 cf = 0.045 af Overall Storage Efficiency = 62.0% Overall System Size = 80.50' x 11.17' x 3.54'

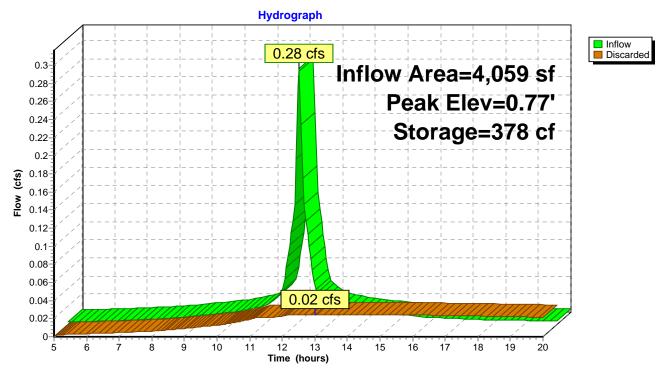
22 Chambers 117.9 cy Field 74.6 cy Stone





Page 73

Pond 11P: Roof Recharge



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Printed 3/9/2020
Page 74

Summary for Pond 13P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 2.77" for 2-Year event

Inflow = 0.08 cfs @ 12.09 hrs, Volume= 277 cf

Outflow = 0.01 cfs @ 13.10 hrs, Volume= 244 cf, Atten= 92%, Lag= 60.6 min

Discarded = 0.01 cfs @ 13.10 hrs, Volume= 244 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.04' @ 13.10 hrs Surf.Area= 200 sf Storage= 120 cf

Plug-Flow detention time= 152.0 min calculated for 244 cf (88% of inflow)

Center-of-Mass det. time= 113.7 min (852.3 - 738.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 13.10 hrs HW=1.04' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.01 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 75

Pond 13P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

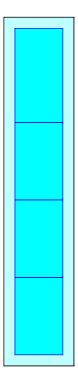
Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

- 4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length
- 1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height
- 4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

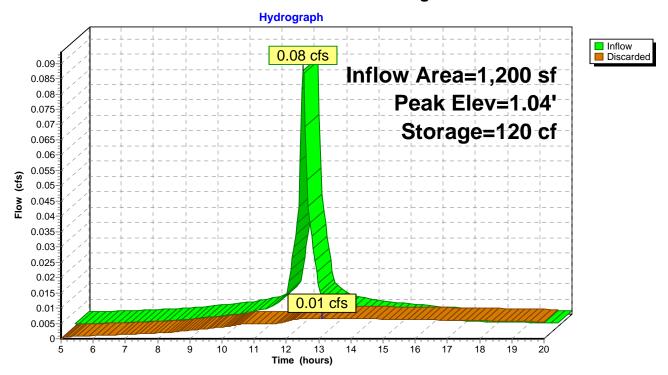
4 Chambers 26.2 cy Field 18.0 cy Stone





Page 76

Pond 13P: Roof Recharge



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Printed 3/9/2020
Page 77

Summary for Pond 14P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 4,059 sf,100.00% Impervious, Inflow Depth > 2.77" for 2-Year event

Inflow = 0.28 cfs @ 12.09 hrs, Volume= 938 cf

Outflow = 0.02 cfs @ 13.08 hrs, Volume= 870 cf, Atten= 92%, Lag= 59.5 min

Discarded = 0.02 cfs @ 13.08 hrs, Volume= 870 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.82' @ 13.08 hrs Surf.Area= 840 sf Storage= 390 cf

Plug-Flow detention time= 143.7 min calculated for 867 cf (92% of inflow)

Center-of-Mass det. time= 117.1 min (855.6 - 738.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	738 cf	16.00'W x 52.50'L x 3.54'H Field A
			2,975 cf Overall - 1,129 cf Embedded = 1,846 cf x 40.0% Voids
#2A	0.50'	1,129 cf	Cultec R-330XLHD x 21 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 3 rows

1,867 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.02 cfs @ 13.08 hrs HW=0.82' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.02 cfs)

Pond 14P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 3 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

7 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 50.50' Row Length +12.0" End Stone x 2 = 52.50' Base Length

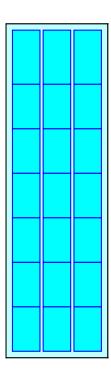
3 Rows x 52.0" Wide + 6.0" Spacing x 2 + 12.0" Side Stone x 2 = 16.00' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

21 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 3 Rows = 1,128.8 cf Chamber Storage

2,975.0 cf Field - 1,128.8 cf Chambers = 1,846.2 cf Stone x 40.0% Voids = 738.5 cf Stone Storage

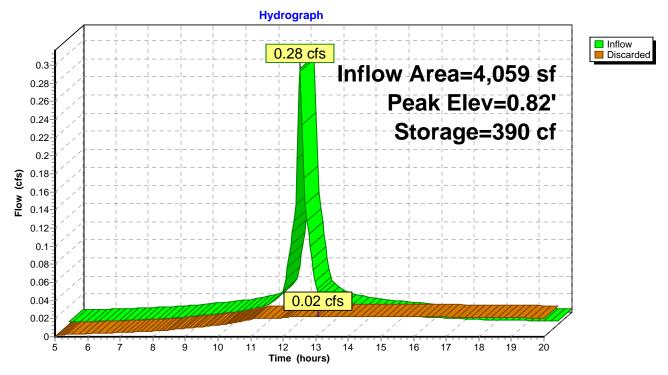
Chamber Storage + Stone Storage = 1,867.3 cf = 0.043 af Overall Storage Efficiency = 62.8% Overall System Size = 52.50' x 16.00' x 3.54'

21 Chambers 110.2 cy Field 68.4 cy Stone





Pond 14P: Roof Recharge



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Printed 3/9/2020
Page 80

Summary for Pond 17P: Roof Recharge

Inflow Area = 13,086 sf, 37.67% Impervious, Inflow Depth > 1.79" for 2-Year event

Inflow = 0.66 cfs @ 12.09 hrs, Volume= 1,953 cf

Outflow = 0.04 cfs @ 13.88 hrs, Volume= 1,500 cf, Atten= 93%, Lag= 107.5 min

Discarded = 0.04 cfs @ 13.88 hrs, Volume= 1,500 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.93' @ 13.88 hrs Surf.Area= 1,677 sf Storage= 945 cf

Plug-Flow detention time= 183.0 min calculated for 1,500 cf (77% of inflow)

Center-of-Mass det. time= 124.8 min (909.4 - 784.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,440 cf	20.83'W x 80.50'L x 3.54'H Field A
			5,940 cf Overall - 2,340 cf Embedded = 3,600 cf x 40.0% Voids
#2A	0.50'	2,340 cf	Cultec R-330XLHD x 44 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 4 rows
		0.700 (T

3,780 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.04 cfs @ 13.88 hrs HW=0.93' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.04 cfs)

Pond 17P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

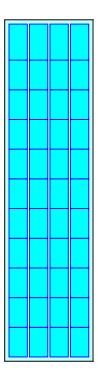
4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

44 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 2,339.6 cf Chamber Storage

5,939.7 cf Field - 2,339.6 cf Chambers = 3,600.1 cf Stone x 40.0% Voids = 1,440.0 cf Stone Storage

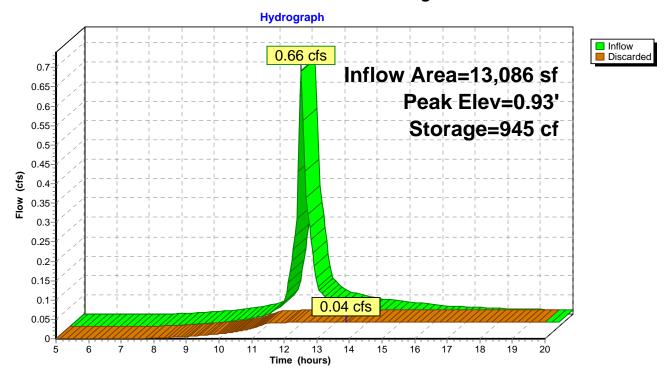
Chamber Storage + Stone Storage = 3,779.6 cf = 0.087 af Overall Storage Efficiency = 63.6% Overall System Size = 80.50' x 20.83' x 3.54'

44 Chambers 220.0 cy Field 133.3 cy Stone





Pond 17P: Roof Recharge



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Printed 3/9/2020
Page 83

Summary for Pond 19P: Roof Recharge

Inflow Area = 11,720 sf, 42.06% Impervious, Inflow Depth > 1.87" for 2-Year event

Inflow = 0.61 cfs @ 12.09 hrs, Volume= 1,828 cf

Outflow = 0.08 cfs @ 12.77 hrs, Volume= 1,825 cf, Atten= 88%, Lag= 40.5 min

Discarded = 0.08 cfs @ 12.77 hrs, Volume= 1,825 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.96' @ 12.77 hrs Surf.Area= 1,176 sf Storage= 681 cf

Plug-Flow detention time= 75.9 min calculated for 1,819 cf (100% of inflow)

Center-of-Mass det. time= 75.1 min (856.6 - 781.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,027 cf	16.00'W x 73.50'L x 3.54'H Field A
			$4,165 \text{ cf Overall - } 1,598 \text{ cf Embedded = } 2,567 \text{ cf } \times 40.0\% \text{ Voids}$
#2A	0.50'	1,598 cf	Cultec R-330XLHD x 30 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 3 rows
		0.005 (T + 1 A 3 1 1 1 O

2,625 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Wetted area	

Discarded OutFlow Max=0.08 cfs @ 12.77 hrs HW=0.96' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.08 cfs)

Pond 19P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 3 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

10 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 71.50' Row Length +12.0" End Stone x 2 = 73.50' Base Length

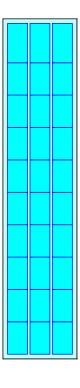
3 Rows x 52.0" Wide + 6.0" Spacing x 2 + 12.0" Side Stone x 2 = 16.00' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

30 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 3 Rows = 1,598.2 cf Chamber Storage

4,165.0 cf Field - 1,598.2 cf Chambers = 2,566.8 cf Stone x 40.0% Voids = 1,026.7 cf Stone Storage

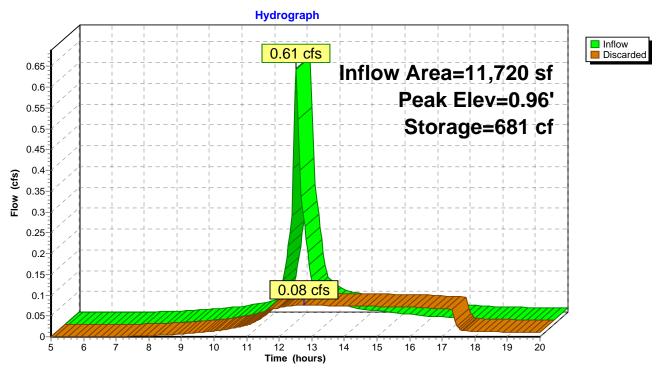
Chamber Storage + Stone Storage = 2,624.9 cf = 0.060 af Overall Storage Efficiency = 63.0% Overall System Size = 73.50' x 16.00' x 3.54'

30 Chambers 154.3 cy Field 95.1 cy Stone





Pond 19P: Roof Recharge



Page 86

Summary for Pond 21P: BASIN B

Inflow Area = 167,695 sf, 37.24% Impervious, Inflow Depth > 0.63" for 2-Year event Inflow 1.99 cfs @ 12.20 hrs. Volume= 8.805 cf 0.31 cfs @ 13.74 hrs, Volume= Outflow 8,740 cf, Atten= 84%, Lag= 92.4 min 0.31 cfs @ 13.74 hrs, Volume= 8.740 cf Discarded = Primary 0.00 cfs @ 5.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 226.10' @ 13.74 hrs Surf.Area= 5,580 sf Storage= 3,195 cf

Plug-Flow detention time= 112.7 min calculated for 8,711 cf (99% of inflow)

Center-of-Mass det. time= 109.8 min (947.9 - 838.2)

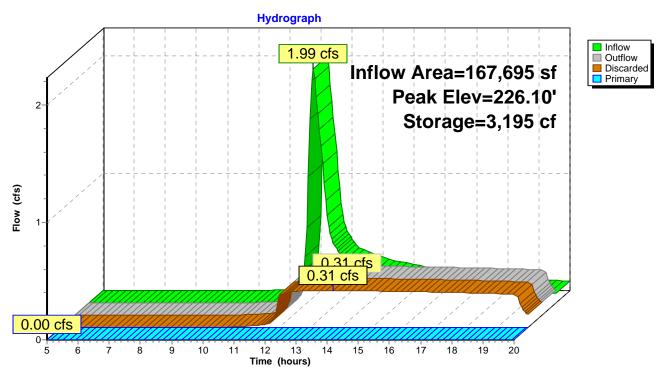
Volume	Inver	t Avail	l.Storage	e Storage Description				
#1	225.50	5.50' 23,381 cf		Custom Stage Data (Irregular)Listed below (Recalc)				
Elevation (feet		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
225.50)	5,039	294.3	0	0	5,039		
226.00	226.00 5,488 30		303.7	2,631	2,631	5,511		
227.00)	6,427	322.5	5,951	8,582	6,498		
228.00)	7,423	328.3	6,919	15,501	6,941		
228.90)	10,160	388.5	7,880	23,381	10,390		
#1	Routing Discarded Primary		.50' 2.41 .90' 25.0 Head	d (feet) 0.20 0.40	adth Broad-Crest 0.60 0.80 1.00	ed Rectangular Wei		

Discarded OutFlow Max=0.31 cfs @ 13.74 hrs HW=226.10' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.31 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=225.50' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 87

Pond 21P: BASIN B



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Printed 3/9/2020
Page 88

Summary for Pond 22P: Roof Recharge

Inflow Area = 11,373 sf, 43.34% Impervious, Inflow Depth > 1.17" for 2-Year event

Inflow = 0.38 cfs @ 12.10 hrs, Volume= 1,110 cf

Outflow = 0.03 cfs @ 13.79 hrs, Volume= 948 cf, Atten= 92%, Lag= 101.6 min

Discarded = 0.03 cfs @ 13.79 hrs, Volume= 948 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.80' @ 13.79 hrs Surf.Area= 1,133 sf Storage= 504 cf

Plug-Flow detention time= 172.5 min calculated for 945 cf (85% of inflow)

Center-of-Mass det. time= 129.4 min (938.0 - 808.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,013 cf	11.17'W x 101.50'L x 3.54'H Field A
			4,014 cf Overall - 1,483 cf Embedded = 2,531 cf x 40.0% Voids
#2A	0.50'	1,483 cf	Cultec R-330XLHD x 28 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
		0.405 (T + 1 A 3 1 1 1 O

2,495 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.03 cfs @ 13.79 hrs HW=0.80' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.03 cfs)

Pond 22P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

14 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 99.50' Row Length +12.0" End Stone x 2 = 101.50' Base Length

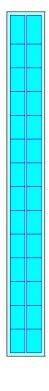
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

28 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,482.7 cf Chamber Storage

4,014.2 cf Field - 1,482.7 cf Chambers = 2,531.4 cf Stone x 40.0% Voids = 1,012.6 cf Stone Storage

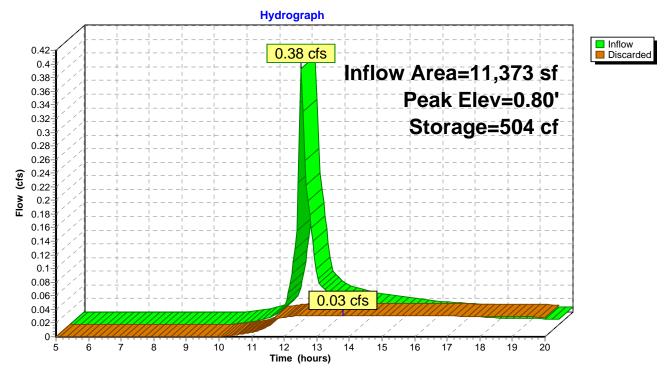
Chamber Storage + Stone Storage = 2,495.3 cf = 0.057 af Overall Storage Efficiency = 62.2% Overall System Size = 101.50' x 11.17' x 3.54'

28 Chambers 148.7 cy Field 93.8 cy Stone



Page 90

Pond 22P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020

Page 91

Summary for Pond 30P: BASIN A

[82] Warning: Early inflow requires earlier time span

Inflow Area = 18,459 sf, 78.24% Impervious, Inflow Depth > 2.42" for 2-Year event
Inflow = 1.36 cfs @ 12.00 hrs, Volume= 3,723 cf
Outflow = 0.17 cfs @ 12.50 hrs, Volume= 3,715 cf, Atten= 88%, Lag= 29.9 min
Discarded = 0.17 cfs @ 12.50 hrs, Volume= 3,715 cf
Primary = 0.00 cfs @ 5.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 238.99' @ 12.50 hrs Surf.Area= 2,077 sf Storage= 1,376 cf

Plug-Flow detention time= 74.1 min calculated for 3,702 cf (99% of inflow)

Center-of-Mass det. time= 72.7 min (820.5 - 747.8)

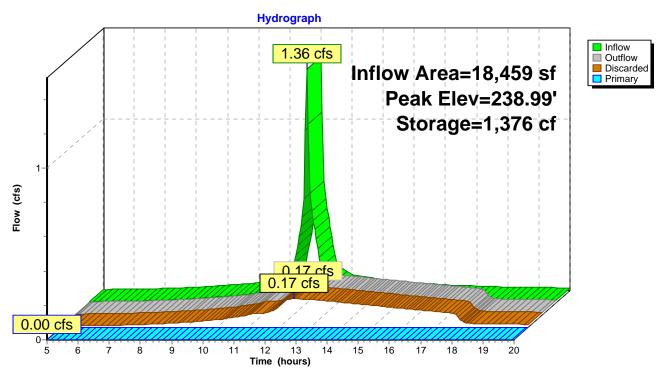
Volume	Invert	Avail.	Storage Storage Description				
#1 238.20' 7,401		7,401 cf	cf Custom Stage Data (Irregular)Listed below (Recalc)				
Elevation (fee		urf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
238.2	20	1,411	262.1	0	0	1,411	
239.0	00	2,083	297.8	1,389	1,389	3,017	
240.0	00	3,005	316.6	2,530	3,919	3,986	
241.0	00	3,983	335.5	3,483	7,401	5,020	
Device	Routing	Inv	ert Outle	et Devices			
#1	Discarded	238.2	20' 2.41	0 in/hr Exfiltration	over Wetted are	a	
#2	Primary	240.0	Head	d (feet) 0.20 0.40	0.60 0.80 1.00	ed Rectangular Weir 1.20 1.40 1.60 63 2.64 2.64 2.63	

Discarded OutFlow Max=0.17 cfs @ 12.50 hrs HW=238.99' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.17 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=238.20' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 92

Pond 30P: BASIN A



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP Printed 3/9/2020

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 93

Summary for Pond 31P: BASIN D

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 239.67' @ 12.67 hrs Surf.Area= 6,945 sf Storage= 4,022 cf

Plug-Flow detention time= 77.0 min calculated for 10,593 cf (100% of inflow) Center-of-Mass det. time= 75.7 min (874.5 - 798.8)

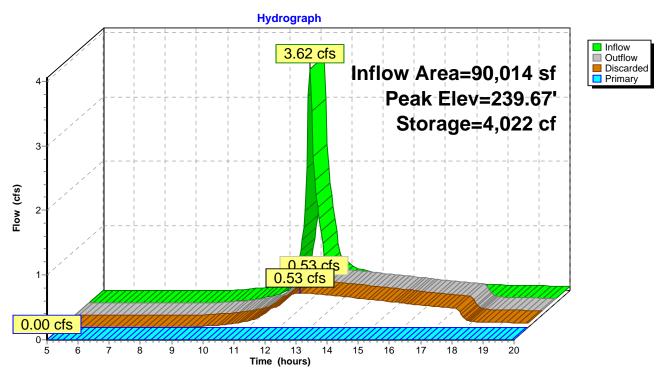
Volume	Inver	t Avail.S	Storage	age Storage Description				
#1 239.00' 24,82		1,822 cf	cf Custom Stage Data (Irregular)Listed below (Recalc)					
Elevatio	_	urf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
239.0	00	5,028	276.6	0	0	5,028		
240.0	00	7,979	406.4	6,447	6,447	12,091		
241.0	00	9,227	425.3	8,595	15,042	13,409		
241.9	90	12,593	472.4	9,780	24,822	16,798		
Device	Routing	Inve	ert Outle	et Devices				
#1	Discarded	239.0	0' 2.41	0 in/hr Exfiltration	over Wetted are	a		
#2 Primary 240.90' 25. 0		Head	O' long x 21.0' breadth Broad-Crested Rectangular Weir and (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 (f. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63					

Discarded OutFlow Max=0.53 cfs @ 12.67 hrs HW=239.67' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.53 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=239.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 94

Pond 31P: BASIN D



Type III 24-hr 2-Year Rainfall=3.20"

Prepared by HP

Printed 3/9/2020

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 95

Summary for Pond 32P: BASIN C

Inflow Area = 188,274 sf, 48.56% Impervious, Inflow Depth > 0.99" for 2-Year event Inflow = 4.44 cfs @ 12.15 hrs, Volume= 15,572 cf
Outflow = 0.85 cfs @ 12.77 hrs, Volume= 15,510 cf, Atten= 81%, Lag= 37.3 min Discarded = 0.85 cfs @ 12.77 hrs, Volume= 15,510 cf
Primary = 0.00 cfs @ 5.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 224.77' @ 12.77 hrs Surf.Area= 10,319 sf Storage= 5,663 cf

Plug-Flow detention time= 77.8 min calculated for 15,510 cf (100% of inflow) Center-of-Mass det. time= 76.3 min (894.8 - 818.5)

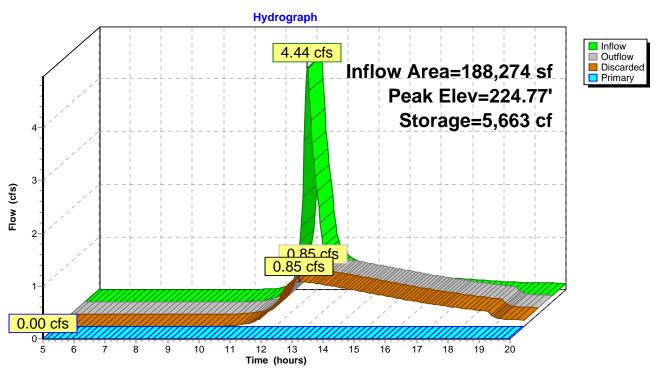
Volume	Invert	t Avail.S	Storage	age Storage Description				
#1 224.00'		' 38	3,476 cf	Custom Stage Data (Irregular)Listed below (Recalc)				
Elevatio	_	urf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
224.0	00	4,753	268.4	0	0	4,753		
225.0	00	12,400	506.3	8,277	8,277	19,424		
226.0	00	15,628	628.5	13,983	22,260	30,474		
226.9	90	20,519	675.7	16,216	38,476	35,408		
Device	Routing	Inve	ert Outle	et Devices				
#1	Discarded	224.0	0' 2.41	0 in/hr Exfiltration	over Wetted area			
#2 Primary 225.90' 25.0' long x 21.0' brea Head (feet) 0.20 0.40 Coef. (English) 2.68 2.		0.60 0.80 1.00 1	.20 1.40 1.60					

Discarded OutFlow Max=0.85 cfs @ 12.77 hrs HW=224.77' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.85 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=224.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 96

Pond 32P: BASIN C



Subcatchment PRDA-1: TO BASIN D

Type III 24-hr 10-Year Rainfall=4.70" Printed 3/9/2020

Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 97

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

3 7	3,7,5,6
Subcatchment 34S: 10 Unit Roof	Runoff Area=10,448 sf 100.00% Impervious Runoff Depth>4.15" Tc=6.0 min CN=98 Runoff=1.08 cfs 3,609 cf
Subcatchment 35S: 14 Unit Roof	Runoff Area=13,061 sf 100.00% Impervious Runoff Depth>4.15" Tc=6.0 min CN=98 Runoff=1.35 cfs 4,512 cf
Subcatchment 36S: 14 Unit Roof	Runoff Area=13,061 sf 100.00% Impervious Runoff Depth>4.15" Tc=6.0 min CN=98 Runoff=1.35 cfs 4,512 cf
Subcatchment 37S: 7 Unit Roof	Runoff Area=7,296 sf 100.00% Impervious Runoff Depth>4.15" Tc=6.0 min CN=98 Runoff=0.75 cfs 2,520 cf
Subcatchment LOT 1: Single Family House	Se Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>4.15" Tc=6.0 min CN=98 Runoff=0.12 cfs 415 cf
Subcatchment LOT 12: DUPLEX + YARD	Runoff Area=11,373 sf 43.34% Impervious Runoff Depth>2.29" Tc=6.0 min CN=78 Runoff=0.74 cfs 2,168 cf
Subcatchment LOT 13: DUPLEX + YARD	Runoff Area=11,720 sf 42.06% Impervious Runoff Depth>3.19" Tc=6.0 min CN=88 Runoff=1.02 cfs 3,116 cf
Subcatchment LOT 14: DUPLEX + YARD	Runoff Area=13,086 sf 37.67% Impervious Runoff Depth>3.09" Tc=6.0 min CN=87 Runoff=1.12 cfs 3,373 cf
Subcatchment LOT 18: DUPLEX ROOF	Runoff Area=4,929 sf 100.00% Impervious Runoff Depth>4.15" Tc=6.0 min CN=98 Runoff=0.51 cfs 1,703 cf
Subcatchment LOT 19: DUPLEX ROOF	Runoff Area=4,059 sf 100.00% Impervious Runoff Depth>4.15" Tc=6.0 min CN=98 Runoff=0.42 cfs 1,402 cf
Subcatchment LOT 2: Single Family House	Se Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>4.15" Tc=6.0 min CN=98 Runoff=0.12 cfs 415 cf
Subcatchment LOT 20: DUPLEX ROOF	Runoff Area=4,059 sf 100.00% Impervious Runoff Depth>4.15" Tc=6.0 min CN=98 Runoff=0.42 cfs 1,402 cf
Subcatchment LOT 21: DUPLEX ROOF	Runoff Area=4,059 sf 100.00% Impervious Runoff Depth>4.15" Tc=6.0 min CN=98 Runoff=0.42 cfs 1,402 cf
Subcatchment LOT 3: Single Family House	se Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>4.15" Tc=6.0 min CN=98 Runoff=0.12 cfs 415 cf
Subcatchment LOT 8: Single Family House	Se Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>4.15" Tc=6.0 min CN=98 Runoff=0.12 cfs 415 cf
0.1	D = ((A = = 54.704 - (50.000/ L = = = 1 + 1 + 1 + 1 + 1 + 1 + 1 + 1 + 1

Runoff Area=51,731 sf 56.02% Impervious Runoff Depth>2.54"

Tc=6.0 min CN=81 Runoff=3.71 cfs 10,961 cf

POST-DEVELOPMENT-REV1 Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD®	Type III 24-hr 10-Year Rainfall=4.70" Printed 3/9/2020 droCAD Software Solutions LLC Page 98
Subcatchment PRDA-10: TO ILSF	Runoff Area=14,656 sf 0.00% Impervious Runoff Depth>0.37" Flow Length=85' Tc=15.0 min CN=47 Runoff=0.06 cfs 450 cf
Subcatchment PRDA-11: BASIN A DIRE	CT Runoff Area=8,000 sf 49.79% Impervious Runoff Depth>3.29" Tc=0.0 min CN=89 Runoff=0.84 cfs 2,196 cf
Subcatchment PRDA-12: BASIN B DIRE	CT Runoff Area=16,454 sf 45.11% Impervious Runoff Depth>2.29" Tc=6.0 min CN=78 Runoff=1.07 cfs 3,137 cf
Subcatchment PRDA-13: BASIN C DIRE	CT Runoff Area=31,453 sf 49.69% Impervious Runoff Depth>2.37" Tc=6.0 min CN=79 Runoff=2.11 cfs 6,215 cf
Subcatchment PRDA-2: TO BASIN A	Runoff Area=10,459 sf 100.00% Impervious Runoff Depth>4.15" Tc=0.0 min CN=98 Runoff=1.26 cfs 3,614 cf
Subcatchment PRDA-3: TO BASIN B	Runoff Area=151,241 sf 36.39% Impervious Runoff Depth>1.39" Flow Length=520' Tc=13.1 min CN=66 Runoff=4.62 cfs 17,494 cf
Subcatchment PRDA-4: TO BASIN C	Runoff Area=156,821 sf 48.34% Impervious Runoff Depth>1.96" Flow Length=370' Tc=10.4 min CN=74 Runoff=7.61 cfs 25,670 cf
Subcatchment PRDA-5: BASIN D DIREC	Runoff Area=38,283 sf 32.09% Impervious Runoff Depth>2.72" Tc=6.0 min CN=83 Runoff=2.92 cfs 8,677 cf
Subcatchment PRDA-6: TO BASIN E	Runoff Area=135,239 sf 32.90% Impervious Runoff Depth>1.97" Tc=6.0 min CN=74 Runoff=7.54 cfs 22,174 cf
Subcatchment PRDA-7: TO WETLAND	Runoff Area=149,386 sf 2.80% Impervious Runoff Depth>0.59" Tc=6.0 min CN=52 Runoff=1.73 cfs 7,346 cf
Subcatchment PRDA-8: TO WETLAND	Runoff Area=46,646 sf 0.00% Impervious Runoff Depth>0.69" Tc=6.0 min CN=54 Runoff=0.70 cfs 2,676 cf
Subcatchment PRDA-9: TO WETLAND	Runoff Area=80,362 sf 11.49% Impervious Runoff Depth>2.37" Tc=6.0 min CN=79 Runoff=5.40 cfs 15,880 cf
Reach 1R: ILSF	Inflow=0.06 cfs 450 cf Outflow=0.06 cfs 450 cf
Reach 14R: WETLAND SOUTH	Inflow=5.40 cfs 15,880 cf Outflow=5.40 cfs 15,880 cf
Reach 15R: WETLAND SOUTHEAST	Inflow=0.70 cfs 2,676 cf Outflow=0.70 cfs 2,676 cf

Pond 1P: Roof Recharge

Peak Elev=1.35' Storage=1,383 cf Inflow=1.08 cfs 3,609 cf

Outflow=0.10 cfs 3,605 cf

Reach 21R: WETLAND NORTH

Inflow=1.73 cfs 7,346 cf

Outflow=1.73 cfs 7,346 cf

Pond 2P: Roof Recharge

Peak Elev=1.53' Storage=2,380 cf Inflow=1.35 cfs 4,512 cf
Outflow=0.06 cfs 2,693 cf

POST-DEVELOPMENT Prepared by HP HydroCAD® 10.00-24 s/n 094	-REV1 Type III 24-hr 10-Year Rainfall=4.70" Printed 3/9/2020 50 © 2018 HydroCAD Software Solutions LLC Page 99
Pond 3P: Roof Recharge	Peak Elev=1.53' Storage=2,380 cf Inflow=1.35 cfs 4,512 cf Outflow=0.06 cfs 2,693 cf
Pond 4P: Roof Recharge	Peak Elev=1.56' Storage=1,323 cf Inflow=0.75 cfs 2,520 cf Outflow=0.04 cfs 1,527 cf
Pond 5P: Roof Recharge	Peak Elev=1.62' Storage=204 cf Inflow=0.12 cfs 415 cf Outflow=0.01 cfs 296 cf
Pond 6P: Roof Recharge	Peak Elev=1.15' Storage=650 cf Inflow=0.42 cfs 1,402 cf Outflow=0.03 cfs 1,067 cf
Pond 7P: Roof Recharge	Peak Elev=1.43' Storage=851 cf Inflow=0.51 cfs 1,703 cf Outflow=0.03 cfs 1,143 cf
Pond 8P: Roof Recharge	Peak Elev=1.62' Storage=204 cf Inflow=0.12 cfs 415 cf Outflow=0.01 cfs 296 cf
Pond 9P: BASIN E	Peak Elev=240.40' Storage=13,055 cf Inflow=7.54 cfs 22,174 cf Discarded=0.37 cfs 11,434 cf Primary=0.00 cfs 0 cf Outflow=0.37 cfs 11,434 cf
Pond 10P: Roof Recharge	Peak Elev=1.62' Storage=204 cf Inflow=0.12 cfs 415 cf Outflow=0.01 cfs 296 cf
Pond 11P: Roof Recharge	Peak Elev=1.15' Storage=650 cf Inflow=0.42 cfs 1,402 cf Outflow=0.03 cfs 1,067 cf
Pond 13P: Roof Recharge	Peak Elev=1.62' Storage=204 cf Inflow=0.12 cfs 415 cf Outflow=0.01 cfs 296 cf
Pond 14P: Roof Recharge	Peak Elev=1.24' Storage=675 cf Inflow=0.42 cfs 1,402 cf Outflow=0.02 cfs 996 cf
Pond 17P: Roof Recharge	Peak Elev=1.66' Storage=1,935 cf Inflow=1.12 cfs 3,373 cf Outflow=0.05 cfs 1,791 cf

Pond 19P: Roof Recharge

Peak Elev=1.69' Storage=1,368 cf Inflow=1.02 cfs 3,116 cf Outflow=0.08 cfs 2,814 cf

Pond 21P: BASIN B

Peak Elev=227.44' Storage=11,502 cf Inflow=5.31 cfs 20,630 cf Discarded=0.37 cfs 11,385 cf Primary=0.00 cfs 0 cf Outflow=0.37 cfs 11,385 cf

Pond 22P: Roof Recharge

Peak Elev=1.62' Storage=1,238 cf Inflow=0.74 cfs 2,168 cf Outflow=0.04 cfs 1,180 cf

Pond 30P: BASIN A

Peak Elev=239.45' Storage=2,424 cf Inflow=2.10 cfs 5,810 cf Discarded=0.19 cfs 5,716 cf Primary=0.00 cfs 0 cf Outflow=0.19 cfs 5,716 cf

Pond 31P: BASIN D

Peak Elev=240.26' Storage=8,586 cf Inflow=6.63 cfs 19,639 cf Discarded=0.69 cfs 18,717 cf Primary=0.00 cfs 0 cf Outflow=0.69 cfs 18,717 cf

Type III 24-hr 10-Year Rainfall=4.70"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Page 100

Pond 32P: BASIN C Peak Elev=225.42' Storage=13,706 cf Inflow=9.35 cfs 31,885 cf Discarded=1.32 cfs 30,126 cf Primary=0.00 cfs 0 cf Outflow=1.32 cfs 30,126 cf

Total Runoff Area = 992,682 sf Runoff Volume = 157,867 cf Average Runoff Depth = 1.91" 64.94% Pervious = 644,625 sf 35.06% Impervious = 348,057 sf

Page 101

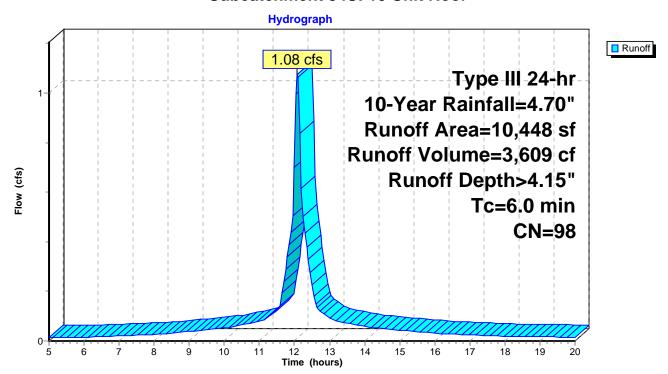
Summary for Subcatchment 34S: 10 Unit Roof

Runoff = 1.08 cfs @ 12.09 hrs, Volume= 3,609 cf, Depth> 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

_	Α	rea (sf)	CN	Description		
*		10,448	98	Impervious		
		10,448		100.00% lm	npervious A	Area
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment 34S: 10 Unit Roof



Page 102

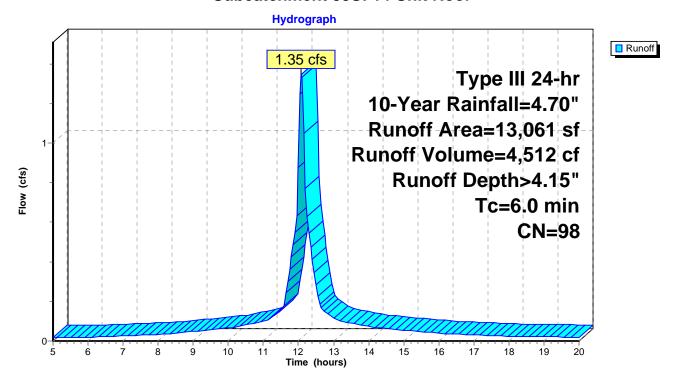
Summary for Subcatchment 35S: 14 Unit Roof

Runoff = 1.35 cfs @ 12.09 hrs, Volume= 4,512 cf, Depth> 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

_	Α	rea (sf)	CN I	Description		
*		13,061	98	mpervious		
		13,061		100.00% Im	npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment 35S: 14 Unit Roof



Page 103

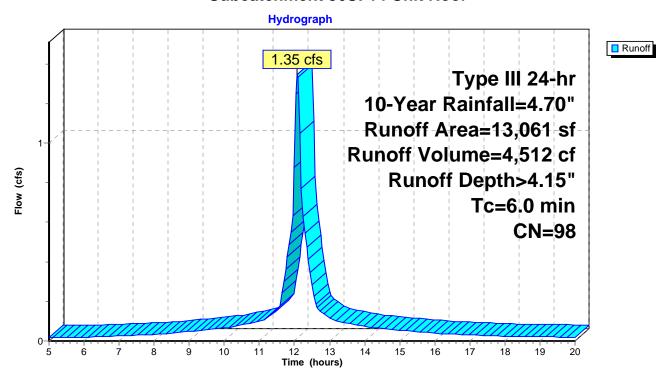
Summary for Subcatchment 36S: 14 Unit Roof

Runoff = 1.35 cfs @ 12.09 hrs, Volume= 4,512 cf, Depth> 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

_	Α	rea (sf)	CN I	Description		
*		13,061	98	mpervious		
		13,061		100.00% Im	npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment 36S: 14 Unit Roof



Page 104

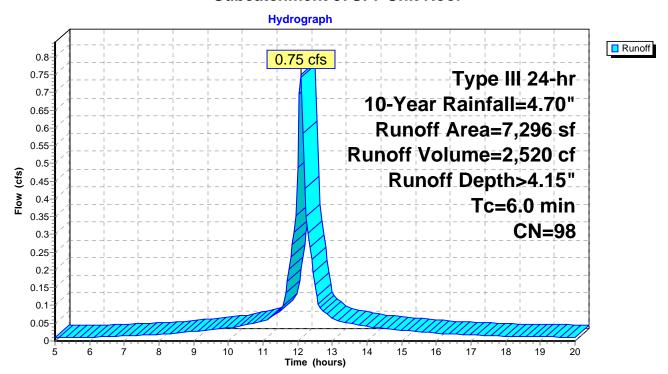
Summary for Subcatchment 37S: 7 Unit Roof

Runoff = 0.75 cfs @ 12.09 hrs, Volume= 2,520 cf, Depth> 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

_	Α	rea (sf)	CN	Description						
*		7,296	98	Impervious						
		7,296		100.00% Impervious Area						
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry,				

Subcatchment 37S: 7 Unit Roof



Page 105

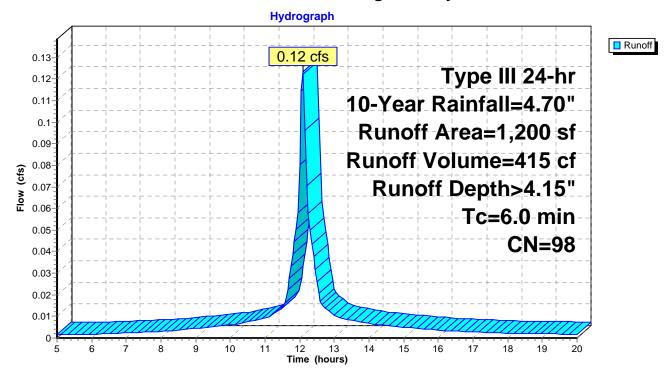
Summary for Subcatchment LOT 1: Single Family House

Runoff = 0.12 cfs @ 12.09 hrs, Volume= 415 cf, Depth> 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

_	Α	rea (sf)	CN	Description		
*		1,200	98	mpervious		
		1,200	,	100.00% Im	npervious A	Area
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment LOT 1: Single Family House



Page 106

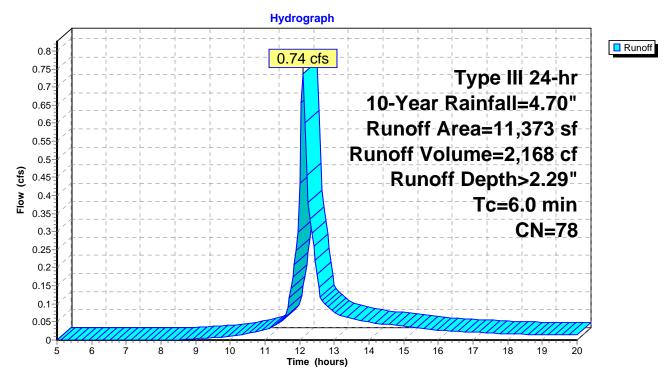
Summary for Subcatchment LOT 12: DUPLEX + YARD

Runoff = 0.74 cfs @ 12.09 hrs, Volume= 2,168 cf, Depth> 2.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Ar	ea (sf)	CN	Description							
*		4,929	98	Impervious							
		490	80	>75% Grass cover, Good, HSG D							
		5,954	61	75% Grass cover, Good, HSG B							
	1	11,373	78	Weighted Average							
		6,444		56.66% Pervious Area							
		4,929		43.34% Imp	pervious Ar	ea					
	To	Longth	Slope	Velocity	Capacity	Description					
	Tc (min)	Length (feet)	(ft/ft)	,	Capacity (cfs)	Description					
_		(leet)	(11/11)	(11/560)	(615)						
	6.0					Direct Entry					

Subcatchment LOT 12: DUPLEX + YARD



Page 107

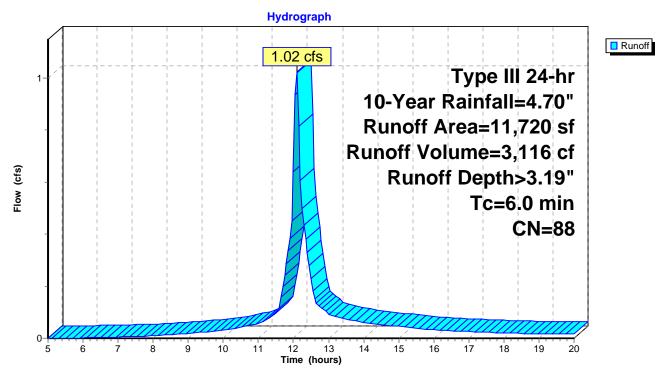
Summary for Subcatchment LOT 13: DUPLEX + YARD

Runoff = 1.02 cfs @ 12.09 hrs, Volume= 3,116 cf, Depth> 3.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Α	rea (sf)	CN	Description						
*		4,929	98	Impervious						
_		6,791	80	>75% Grass cover, Good, HSG D						
		11,720	88	Veighted Average						
		6,791		57.94% Pervious Area						
		4,929		42.06% Imp	ervious Ar	rea				
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry,				

Subcatchment LOT 13: DUPLEX + YARD



POST-DEVELOPMENT-REV1

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 108

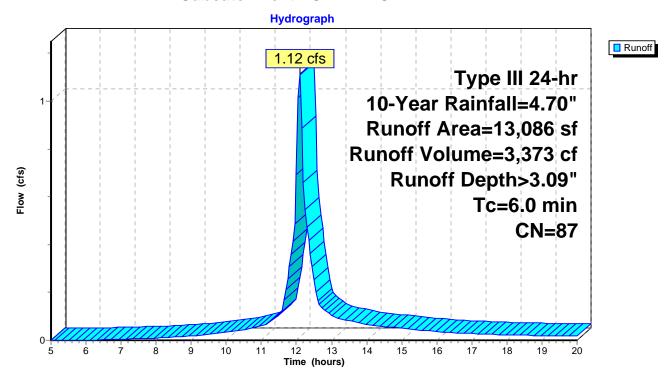
Summary for Subcatchment LOT 14: DUPLEX + YARD

Runoff = 1.12 cfs @ 12.09 hrs, Volume= 3,373 cf, Depth> 3.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Α	rea (sf)	CN	Description						
*		4,929	98	Impervious						
		8,157	80	>75% Grass cover, Good, HSG D						
		13,086	87	Weighted A	Veighted Average					
		8,157		62.33% Pervious Area						
		4,929		37.67% lmp	ervious Ar	rea				
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry,				

Subcatchment LOT 14: DUPLEX + YARD



Page 109

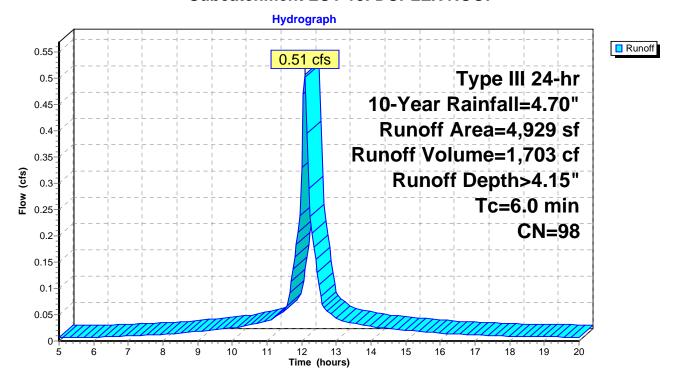
Summary for Subcatchment LOT 18: DUPLEX ROOF

Runoff = 0.51 cfs @ 12.09 hrs, Volume= 1,703 cf, Depth> 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Α	rea (sf)	CN I	Description					
*		4,929	98	Impervious					
		4,929		100.00% Im	npervious A	Area			
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 18: DUPLEX ROOF



Page 110

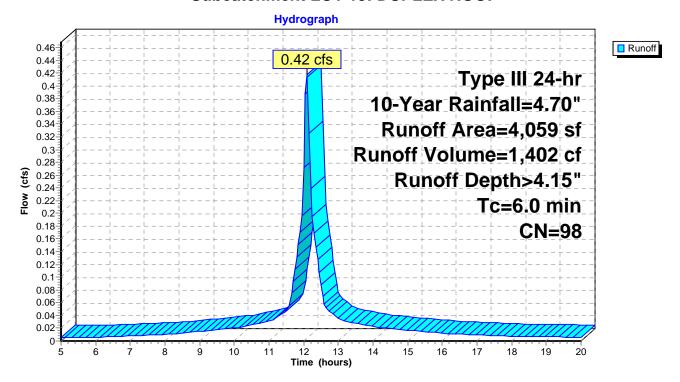
Summary for Subcatchment LOT 19: DUPLEX ROOF

Runoff = 0.42 cfs @ 12.09 hrs, Volume= 1,402 cf, Depth> 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Α	rea (sf)	CN I	Description					
*		4,059	98 I	mpervious					
		4,059	•	100.00% Impervious Area					
	Тс	Length	Slope	,	Capacity	•			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 19: DUPLEX ROOF



Page 111

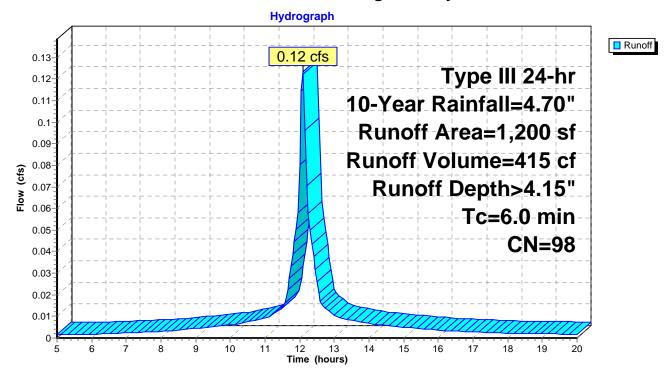
Summary for Subcatchment LOT 2: Single Family House

Runoff = 0.12 cfs @ 12.09 hrs, Volume= 415 cf, Depth> 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Α	rea (sf)	CN I	Description					
*		1,200	98	Impervious					
		1,200		100.00% Im	npervious A	Area			
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 2: Single Family House



Page 112

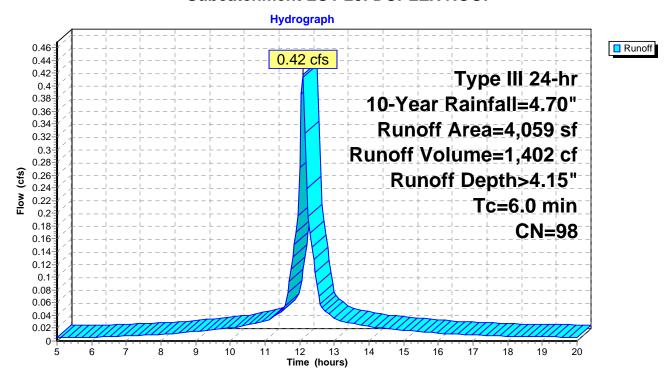
Summary for Subcatchment LOT 20: DUPLEX ROOF

Runoff = 0.42 cfs @ 12.09 hrs, Volume= 1,402 cf, Depth> 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Α	rea (sf)	CN I	Description				
*		4,059	98 I	Impervious				
		4,059	•	100.00% Im	npervious A	Area		
	Тс	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	6.0					Direct Entry,		

Subcatchment LOT 20: DUPLEX ROOF



Page 113

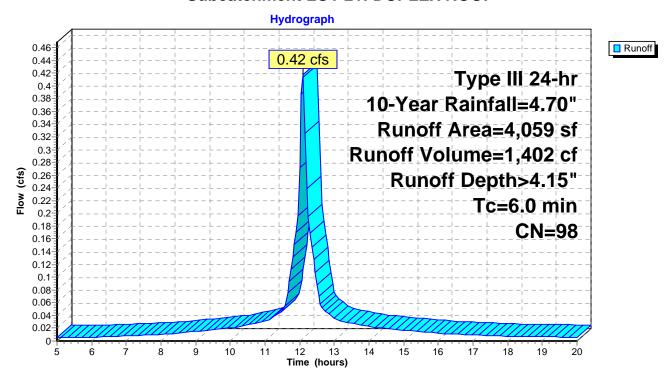
Summary for Subcatchment LOT 21: DUPLEX ROOF

Runoff = 0.42 cfs @ 12.09 hrs, Volume= 1,402 cf, Depth> 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Α	rea (sf)	CN I	Description				
*		4,059	98 I	Impervious				
		4,059	•	100.00% Im	npervious A	Area		
	Тс	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	6.0					Direct Entry,		

Subcatchment LOT 21: DUPLEX ROOF



Page 114

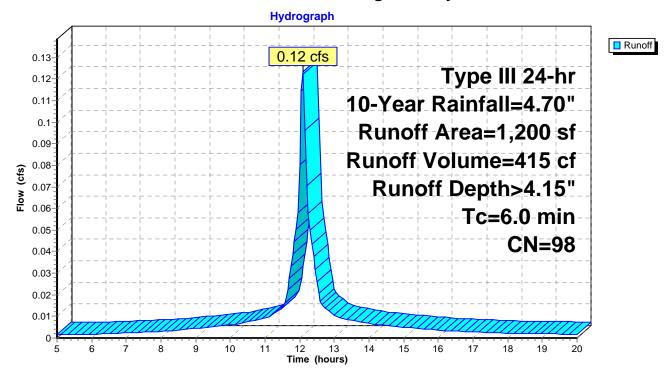
Summary for Subcatchment LOT 3: Single Family House

Runoff = 0.12 cfs @ 12.09 hrs, Volume= 415 cf, Depth> 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Α	rea (sf)	CN	Description		
*		1,200	98	Impervious		
		1,200		100.00% lm	npervious A	Area
		Length	Slope	,	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment LOT 3: Single Family House



Page 115

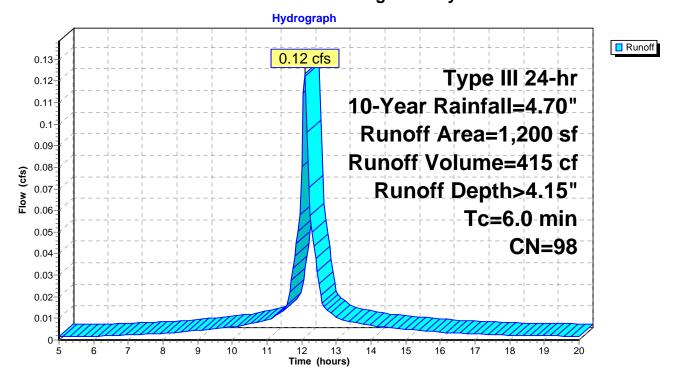
Summary for Subcatchment LOT 8: Single Family House

Runoff = 0.12 cfs @ 12.09 hrs, Volume= 415 cf, Depth> 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

_	Α	rea (sf)	CN	Description					
*		1,200	98	Impervious					
		1,200	,	100.00% Impervious Area					
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 8: Single Family House



POST-DEVELOPMENT-REV1

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 116

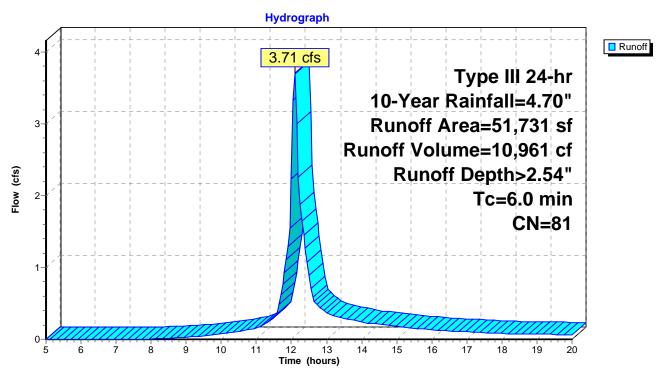
Summary for Subcatchment PRDA-1: TO BASIN D

Runoff = 3.71 cfs @ 12.09 hrs, Volume= 10,961 cf, Depth> 2.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Area (sf)	CN	Description	Description					
*	25,896	98	Impervious						
	6,014	54	1/2 acre lots	s, 25% imp	o, HSG A				
	6,314	80	1/2 acre lots	s, 25% imp	o, HSG C				
	6,846	39	>75% Gras	s cover, Go	lood, HSG A				
	462	74	>75% Gras	s cover, Go	lood, HSG C				
	6,199	80	>75% Gras	s cover, Go	ood, HSG D				
	51,731	81	Weighted A	verage					
	22,753		43.98% Per	vious Area	a				
	28,978		56.02% Imp	ervious Ar	rea				
Т	c Length	Slop		Capacity	·				
(mir	n) (feet)	(ft/f	t) (ft/sec)	(cfs)					
6.	0				Direct Entry,				

Subcatchment PRDA-1: TO BASIN D



Page 117

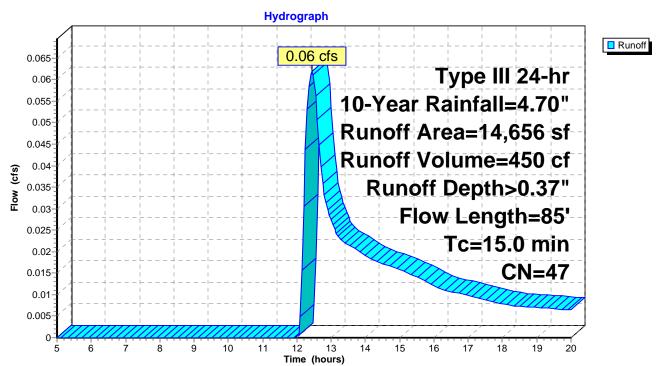
Summary for Subcatchment PRDA-10: TO ILSF

Runoff = 0.06 cfs @ 12.44 hrs, Volume= 450 cf, Depth> 0.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

_	Α	rea (sf)	CN I	CN Description							
		1,166	39 :	39 >75% Grass cover, Good, HSG A							
		2,000	80 :	>75% Gras	s cover, Go	ood, HSG D					
		8,622	30 \	Noods, Go	od, HSG A						
_		2,868	77 \	Noods, Go	od, HSG D						
		14,656	47 ١	Neighted A	verage						
		14,656	•	100.00% Pe	ervious Are	a					
	Tc	Length	Slope	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	14.2	50	0.0140	0.06		Sheet Flow, AB					
						Woods: Light underbrush n= 0.400 P2= 3.20"					
	8.0	35	0.0220	0.74		Shallow Concentrated Flow, BC					
_						Woodland Kv= 5.0 fps					
	15.0	85	Total		·						

Subcatchment PRDA-10: TO ILSF



Summary for Subcatchment PRDA-11: BASIN A DIRECT

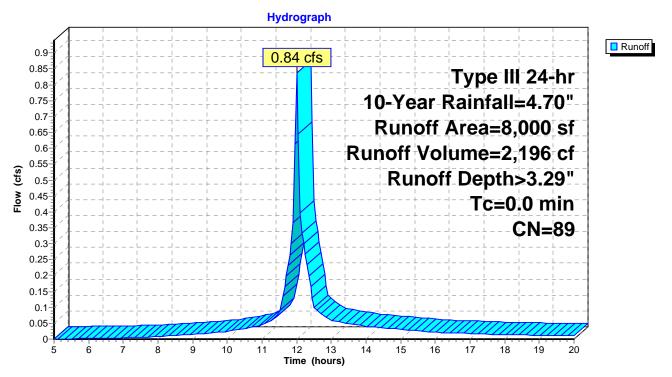
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 0.84 cfs @ 12.00 hrs, Volume= 2,196 cf, Depth> 3.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

Area (sf)	CN	Description			
3,983	98	Water Surface, HSG B			
4,017	80	>75% Grass cover, Good, HSG D			
8,000	89	Weighted Average			
4,017		50.21% Pervious Area			
3,983		49.79% Impervious Area			

Subcatchment PRDA-11: BASIN A DIRECT



Page 119

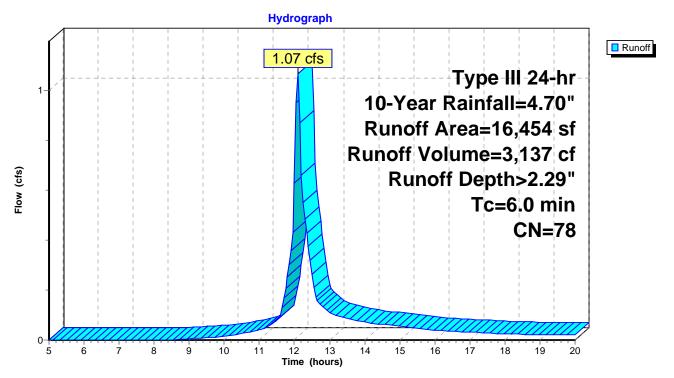
Summary for Subcatchment PRDA-12: BASIN B DIRECT

Runoff = 1.07 cfs @ 12.09 hrs, Volume= 3,137 cf, Depth> 2.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

A	rea (sf)	CN	Description							
	7,423	98	Water Surface, HSG D							
	5,050	80	>75% Gras	s cover, Go	od, HSG D					
	3,981	39	>75% Gras	s cover, Go	od, HSG A					
	16,454	78	Weighted A	verage						
	9,031		54.89% Per	vious Area						
	7,423		45.11% lmp	ervious Are	ea					
Тс	Length	Slope	Velocity	Capacity	Description					
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry					

Subcatchment PRDA-12: BASIN B DIRECT



Page 120

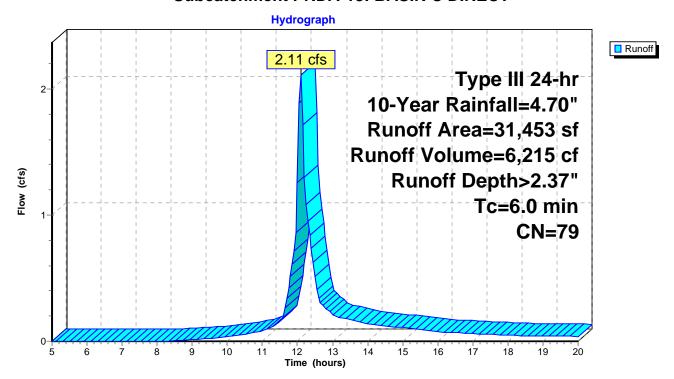
Summary for Subcatchment PRDA-13: BASIN C DIRECT

Runoff = 2.11 cfs @ 12.09 hrs, Volume= 6,215 cf, Depth> 2.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

Are	ea (sf)	CN	Description							
1	5,825	61	>75% Grass cover, Good, HSG B							
1	5,628	98	Water Surfa	ice, HSG B						
3	31,453	79	Weighted A	verage						
1	5,825		50.31% Per	vious Area						
1	5,628		49.69% Imp	ervious Are	ea					
Tc	Length	Slope	e Velocity	Capacity	Description					
(min)	(feet)	(ft/ft								
6.0					Direct Entry,					

Subcatchment PRDA-13: BASIN C DIRECT



Summary for Subcatchment PRDA-2: TO BASIN A

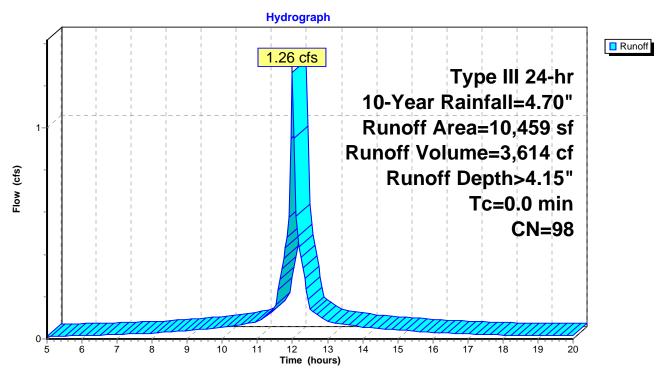
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 1.26 cfs @ 12.00 hrs, Volume= 3,614 cf, Depth> 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Area (sf)	CN	Description
*	10,459	98	IMPERVIOUS
	10,459		100.00% Impervious Area

Subcatchment PRDA-2: TO BASIN A



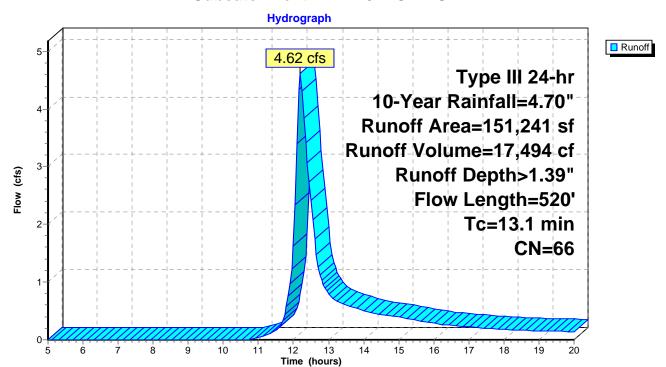
Summary for Subcatchment PRDA-3: TO BASIN B

Runoff = 4.62 cfs @ 12.20 hrs, Volume= 17,494 cf, Depth> 1.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Α	rea (sf)	CN E	escription							
*		40,248	98 II	IMPERVIOUS							
		14,787	98 F	Roofs, HSG							
		77,313	39 >	>75% Grass cover, Good, HSG A							
		92	61 >	75% Gras	s cover, Go	ood, HSG B					
		18,801	80 >	75% Gras	s cover, Go	ood, HSG D					
	1	51,241		Veighted A							
		96,206	_		vious Area						
		55,035	3	6.39% Imp	ervious Ar	ea					
	_		01			B 1.0					
	Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	6.3	50	0.0380	0.13		Sheet Flow,					
						Grass: Dense n= 0.240 P2= 3.20"					
	6.5	406	0.0220	1.04		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	0.3	64	0.0360	3.85		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	13.1	520	Total								

Subcatchment PRDA-3: TO BASIN B



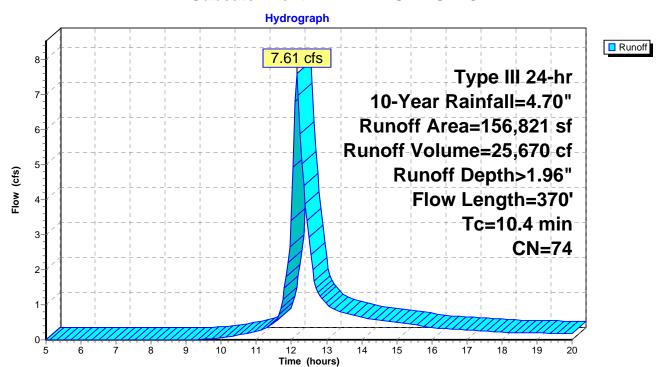
Summary for Subcatchment PRDA-4: TO BASIN C

Runoff = 7.61 cfs @ 12.15 hrs, Volume= 25,670 cf, Depth> 1.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Α	rea (sf)	CN D	escription							
*		70,874	98 Ir	Impervious							
		4,929	98 F	Roofs, HSC							
		35,430	39 >	75% Gras	s cover, Go	ood, HSG A					
		44,834	61 >	>75% Grass cover, Good, HSG B							
		754	80 >	75% Gras	s cover, Go	ood, HSG D					
	1	56,821	74 V	Veighted A	verage						
		81,018	5	1.66% Per	vious Area						
		75,803	4	8.34% Imp	ervious Ar	ea					
	_										
	Tc	Length	Slope	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	6.8	50	0.0320	0.12		Sheet Flow,					
						Grass: Dense n= 0.240 P2= 3.20"					
	3.1	217	0.0280	1.17		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	0.5	103	0.0270	3.34		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	10.4	370	Total								

Subcatchment PRDA-4: TO BASIN C



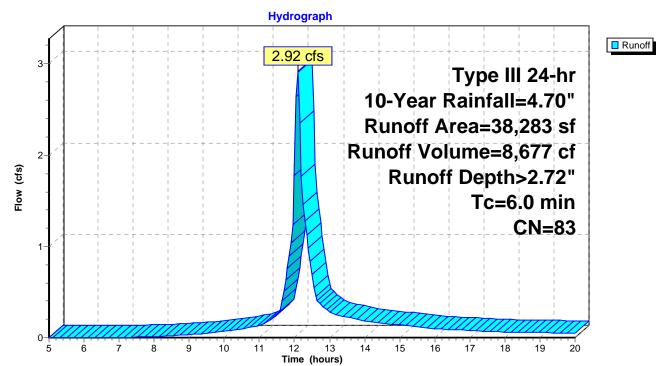
Summary for Subcatchment PRDA-5: BASIN D DIRECT

Runoff = 2.92 cfs @ 12.09 hrs, Volume= 8,677 cf, Depth> 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	rea (sf)	CN	Description						
*	1,132	98	Impervious	Impervious					
	7,697	80	1/2 acre lots	s, 25% imp	o, HSG C				
	374	39	>75% Gras	s cover, Go	ood, HSG A				
	7,237	74	>75% Gras	s cover, Go	ood, HSG C				
	6,781	80	>75% Gras	s cover, Go	ood, HSG D				
	9,227	98	Water Surfa	ace, HSG A	A				
	4,080	77	Woods, Go	od, HSG D)				
	1,323	70	Woods, Go	od, HSG C					
	432	96	Gravel surfa	ace, HSG C	C				
	38,283	83	Weighted A	verage					
	26,000		67.91% Per	vious Area	a				
	12,283		32.09% Imp	ervious Ar	rea				
Tc	Length	Slop	e Velocity	Capacity	Description				
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)					
6.0					Direct Entry,				

Subcatchment PRDA-5: BASIN D DIRECT



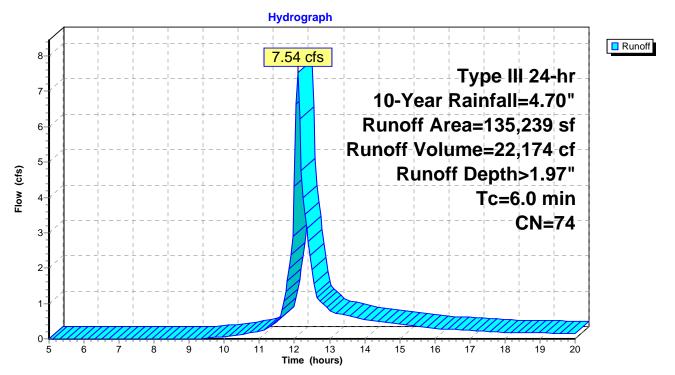
Summary for Subcatchment PRDA-6: TO BASIN E

Runoff = 7.54 cfs @ 12.10 hrs, Volume= 22,174 cf, Depth> 1.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Area (sf)	CN	Description					
*	19,869	98	IMPERVIO	JS				
	57,855	80	1/2 acre lot	s, 25% imp	o, HSG C			
	2,327	54	1/2 acre lot	s, 25% imp	o, HSG A			
	3,600	98	Roofs, HSG	A .				
	27,519	39	>75% Gras	s cover, Go	ood, HSG A			
	16,283	74	>75% Gras	s cover, Go	ood, HSG C			
	1,810	80	>75% Gras	s cover, Go	ood, HSG D			
	5,976	98	Water Surfa	ace, HSG D				
	135,239	74	Weighted A	verage				
	90,749		67.10% Per	vious Area	a			
	44,491		32.90% Imp	ervious Ar	rea			
Т	c Length	Slop	e Velocity	Capacity	Description			
(min) (feet)	(ft/f	t) (ft/sec)	(cfs)				
6.	0				Direct Entry,			

Subcatchment PRDA-6: TO BASIN E



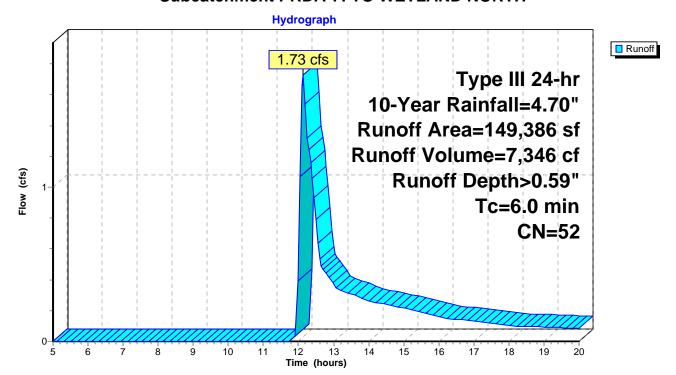
Summary for Subcatchment PRDA-7: TO WETLAND NORTH

Runoff = 1.73 cfs @ 12.12 hrs, Volume= 7,346 cf, Depth> 0.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

A	rea (sf)	CN	Description			
	2,951	80	1/2 acre lots, 25% imp, HSG C			
	8,254	54	1/2 acre lots, 25% imp, HSG A			
	52,416	39	>75% Grass cover, Good, HSG A			
	27,465	61	>75% Grass cover, Good, HSG B			
	538	74	>75% Grass cover, Good, HSG C			
	9,934	80	>75% Grass cover, Good, HSG D			
	22,059	30	Woods, Good, HSG A			
	14,418	70	Woods, Good, HSG C			
	9,973	77	Woods, Good, HSG D			
*	1,378	98	impervious			
1	149,386	52	Weighted Average			
1	145,207		97.20% Pervious Area			
	4,179		2.80% Impervious Area			
Tc (min)	Length (feet)	Slop (ft/f				
6.0			Direct Entry,			

Subcatchment PRDA-7: TO WETLAND NORTH



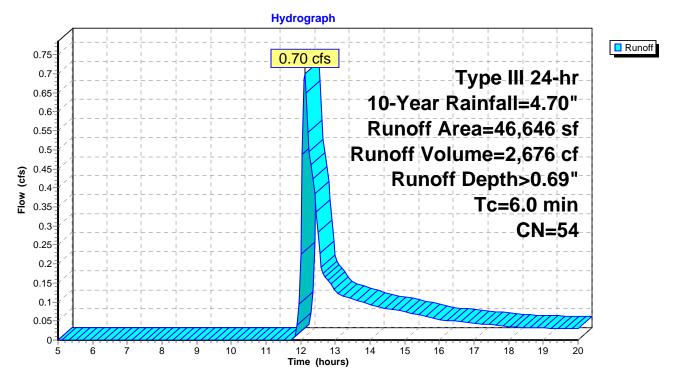
Summary for Subcatchment PRDA-8: TO WETLAND SOUTHEAST

Runoff = 0.70 cfs @ 12.12 hrs, Volume= 2,676 cf, Depth> 0.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

Area (s	sf) CN	Description				
21,55	59 39	>75% Grass cover, Good, HSG A				
7,43	34 80	>75% Grass cover, Good, HSG D				
12,1	18 77	Woods, Good, HSG D				
5,53	35 30	Woods, Good, HSG A				
46,64	46 54	54 Weighted Average				
46,64	46	100.00% Pervious Area				
Tc Len	gth Slo _l	pe Velocity Capacity Description				
(min) (fe	eet) (ft/	'ft) (ft/sec) (cfs)				
6.0		Direct Entry.				

Subcatchment PRDA-8: TO WETLAND SOUTHEAST



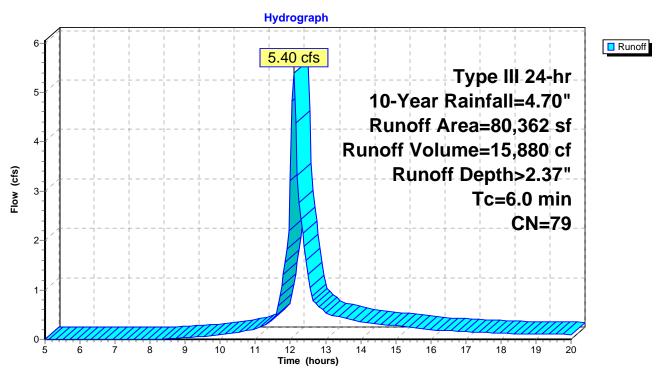
Summary for Subcatchment PRDA-9: TO WETLAND SOUTH

Runoff = 5.40 cfs @ 12.09 hrs, Volume= 15,880 cf, Depth> 2.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Area (sf)	CN	Description				
	274	39	>75% Grass cover, Good, HSG A				
	20,073	80	>75% Grass cover, Good, HSG D				
	7,904	70	Woods, Go	od, HSG C			
	34,217	77	Woods, Go	od, HSG D			
	11,544	80	1/2 acre lots	s, 25% imp	p, HSG C		
*	6,350	98	IMPERVIOUS				
	80,362	79	79 Weighted Average				
	71,126		88.51% Pervious Area				
	9,236		11.49% Impervious Area				
Tc	Length	Slop	e Velocity	Capacity	Description		
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Subcatchment PRDA-9: TO WETLAND SOUTH



Page 129

Summary for Reach 1R: ILSF

[40] Hint: Not Described (Outflow=Inflow)

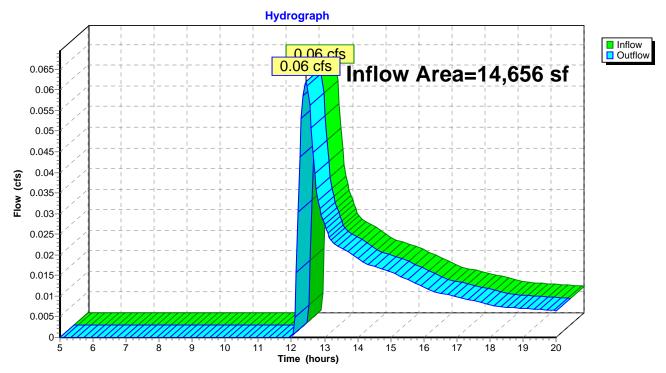
Inflow Area = 14,656 sf, 0.00% Impervious, Inflow Depth > 0.37" for 10-Year event

Inflow = 0.06 cfs @ 12.44 hrs, Volume= 450 cf

Outflow = 0.06 cfs @ 12.44 hrs, Volume= 450 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 1R: ILSF



Page 130

Summary for Reach 14R: WETLAND SOUTH

[40] Hint: Not Described (Outflow=Inflow)

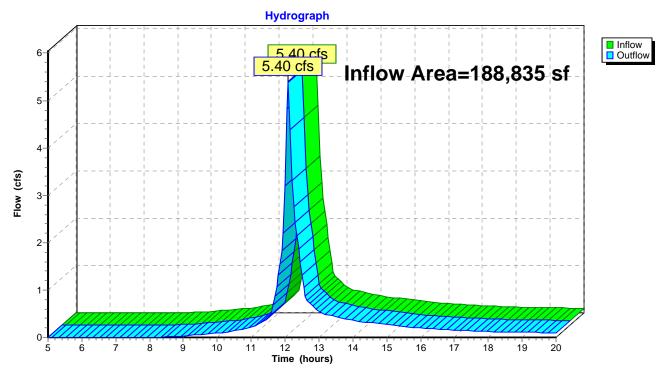
Inflow Area = 188,835 sf, 34.39% Impervious, Inflow Depth > 1.01" for 10-Year event

Inflow = 5.40 cfs @ 12.09 hrs, Volume= 15,880 cf

Outflow = 5.40 cfs @ 12.09 hrs, Volume= 15,880 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 14R: WETLAND SOUTH



Page 131

Summary for Reach 15R: WETLAND SOUTHEAST

[40] Hint: Not Described (Outflow=Inflow)

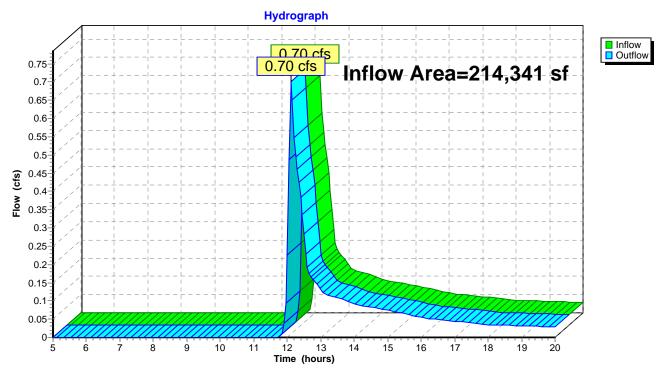
Inflow Area = 214,341 sf, 29.14% Impervious, Inflow Depth > 0.15" for 10-Year event

Inflow = 0.70 cfs @ 12.12 hrs, Volume= 2,676 cf

Outflow = 0.70 cfs @ 12.12 hrs, Volume= 2,676 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 15R: WETLAND SOUTHEAST



Summary for Reach 21R: WETLAND NORTH

[40] Hint: Not Described (Outflow=Inflow)

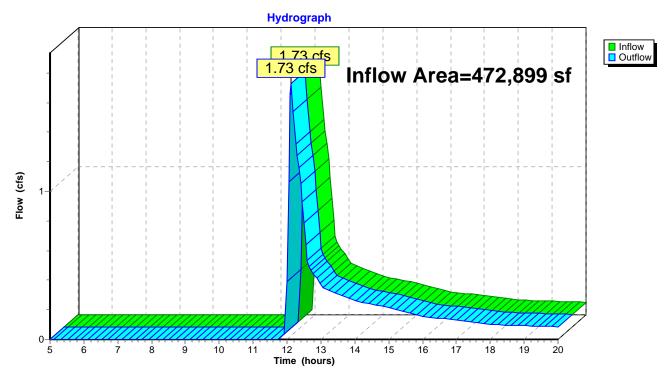
Inflow Area = 472,899 sf, 29.63% Impervious, Inflow Depth > 0.19" for 10-Year event

Inflow = 1.73 cfs @ 12.12 hrs, Volume= 7,346 cf

Outflow = 1.73 cfs @ 12.12 hrs, Volume= 7,346 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 21R: WETLAND NORTH



POST-DEVELOPMENT-REV1

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 133

Summary for Pond 1P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 10,448 sf,100.00% Impervious, Inflow Depth > 4.15" for 10-Year event

Inflow = 1.08 cfs @ 12.09 hrs, Volume= 3,609 cf

Outflow = 0.10 cfs @ 12.93 hrs, Volume= 3,605 cf, Atten= 91%, Lag= 50.7 min

Discarded = 0.10 cfs @ 12.93 hrs, Volume= 3,605 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.35' @ 12.93 hrs Surf.Area= 1,531 sf Storage= 1,383 cf

Plug-Flow detention time= 108.7 min calculated for 3,604 cf (100% of inflow)

Center-of-Mass det. time= 108.0 min (843.5 - 735.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,317 cf	20.83'W x 73.50'L x 3.54'H Field A
			5,423 cf Overall - 2,131 cf Embedded = 3,292 cf x 40.0% Voids
#2A	0.50'	2,131 cf	Cultec R-330XLHD x 40 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

3,448 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.10 cfs @ 12.93 hrs HW=1.35' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.10 cfs)

Pond 1P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

10 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 71.50' Row Length +12.0" End Stone x 2 = 73.50' Base Length

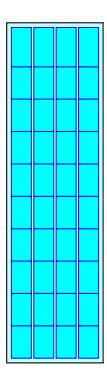
4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

40 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 2,131.0 cf Chamber Storage

5,423.2 cf Field - 2,131.0 cf Chambers = 3,292.2 cf Stone x 40.0% Voids = 1,316.9 cf Stone Storage

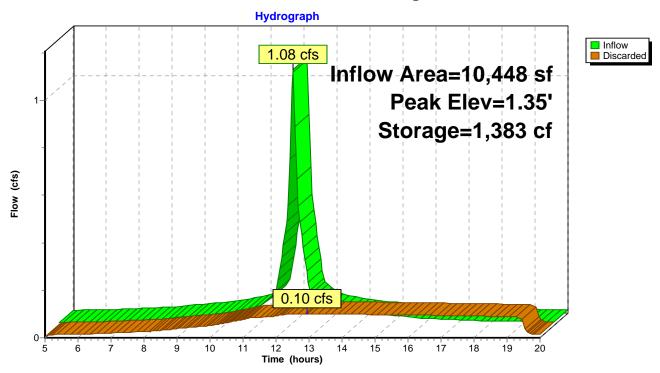
Chamber Storage + Stone Storage = 3,447.9 cf = 0.079 af Overall Storage Efficiency = 63.6% Overall System Size = 73.50' x 20.83' x 3.54'

40 Chambers 200.9 cy Field 121.9 cy Stone





Pond 1P: Roof Recharge



POST-DEVELOPMENT-REV1

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 136

Summary for Pond 2P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 13,061 sf,100.00% Impervious, Inflow Depth > 4.15" for 10-Year event

Inflow = 1.35 cfs @ 12.09 hrs, Volume= 4,512 cf

Outflow = 0.06 cfs @ 14.60 hrs, Volume= 2,693 cf, Atten= 95%, Lag= 150.7 min

Discarded = 0.06 cfs @ 14.60 hrs, Volume= 2,693 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.53' @ 14.60 hrs Surf.Area= 2,246 sf Storage= 2,380 cf

Plug-Flow detention time= 165.9 min calculated for 2,692 cf (60% of inflow)

Center-of-Mass det. time= 84.4 min (819.9 - 735.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,907 cf	25.67'W x 87.50'L x 3.54'H Field A
			7,954 cf Overall - 3,185 cf Embedded = 4,769 cf \times 40.0% Voids
#2A	0.50'	3,185 cf	Cultec R-330XLHD x 60 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 5 rows
•			

5,093 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.06 cfs @ 14.60 hrs HW=1.53' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.06 cfs)

Pond 2P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 5 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

12 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 85.50' Row Length +12.0" End Stone x 2 = 87.50' Base Length

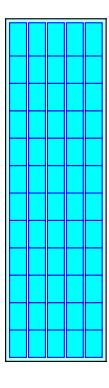
5 Rows x 52.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 25.67' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

60 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 5 Rows = 3,185.3 cf Chamber Storage

7,954.0 cf Field - 3,185.3 cf Chambers = 4,768.7 cf Stone x 40.0% Voids = 1,907.5 cf Stone Storage

Chamber Storage + Stone Storage = 5,092.8 cf = 0.117 af Overall Storage Efficiency = 64.0% Overall System Size = 87.50' x 25.67' x 3.54'

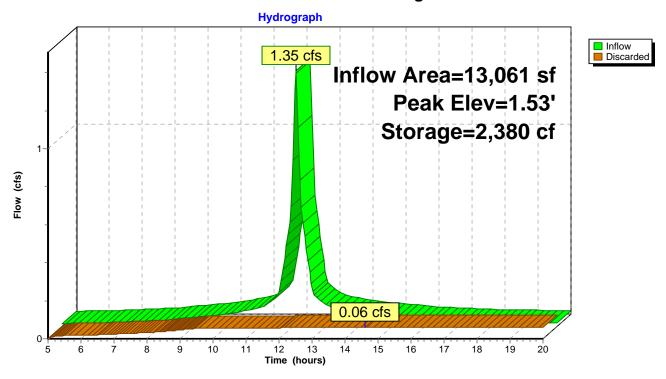
60 Chambers 294.6 cy Field 176.6 cy Stone





Page 138

Pond 2P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 139

Summary for Pond 3P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 13,061 sf,100.00% Impervious, Inflow Depth > 4.15" for 10-Year event

Inflow = 1.35 cfs @ 12.09 hrs, Volume= 4,512 cf

Outflow = 0.06 cfs @ 14.60 hrs, Volume= 2,693 cf, Atten= 95%, Lag= 150.7 min

Discarded = 0.06 cfs @ 14.60 hrs, Volume= 2,693 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.53' @ 14.60 hrs Surf.Area= 2,246 sf Storage= 2,380 cf

Plug-Flow detention time= 165.9 min calculated for 2,692 cf (60% of inflow)

Center-of-Mass det. time= 84.4 min (819.9 - 735.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,907 cf	25.67'W x 87.50'L x 3.54'H Field A
			7,954 cf Overall - 3,185 cf Embedded = 4,769 cf \times 40.0% Voids
#2A	0.50'	3,185 cf	Cultec R-330XLHD x 60 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 5 rows
•			

5,093 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.06 cfs @ 14.60 hrs HW=1.53' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.06 cfs)

Pond 3P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 5 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

12 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 85.50' Row Length +12.0" End Stone x 2 = 87.50' Base Length

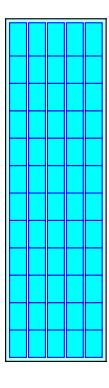
5 Rows x 52.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 25.67' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

60 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 5 Rows = 3,185.3 cf Chamber Storage

7,954.0 cf Field - 3,185.3 cf Chambers = 4,768.7 cf Stone x 40.0% Voids = 1,907.5 cf Stone Storage

Chamber Storage + Stone Storage = 5,092.8 cf = 0.117 af Overall Storage Efficiency = 64.0% Overall System Size = 87.50' x 25.67' x 3.54'

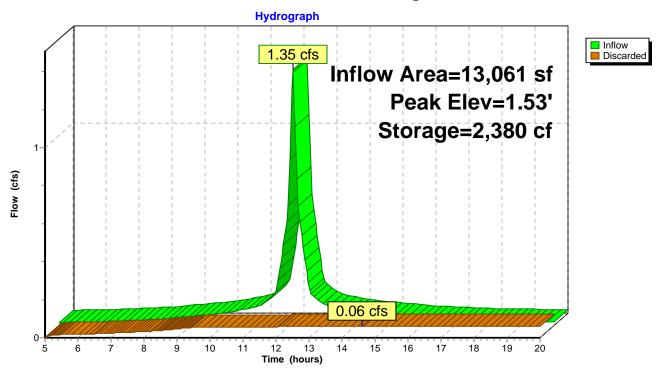
60 Chambers 294.6 cy Field 176.6 cy Stone





Page 141

Pond 3P: Roof Recharge



POST-DEVELOPMENT-REV1 Type III 24-hr 10-Year Rainfall=4.70"

Prepared by HP

Printed 3/9/2020

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 142

Summary for Pond 4P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 7,296 sf,100.00% Impervious, Inflow Depth > 4.15" for 10-Year event

Inflow = 0.75 cfs @ 12.09 hrs, Volume= 2,520 cf

Outflow = 0.04 cfs @ 14.49 hrs, Volume= 1,527 cf, Atten= 95%, Lag= 144.3 min

Discarded = 0.04 cfs @ 14.49 hrs, Volume= 1,527 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.56' @ 14.49 hrs Surf.Area= 1,240 sf Storage= 1,323 cf

Plug-Flow detention time= 167.0 min calculated for 1,526 cf (61% of inflow)

Center-of-Mass det. time= 86.8 min (822.3 - 735.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,071 cf	20.83'W x 59.50'L x 3.54'H Field A
			4,390 cf Overall - 1,714 cf Embedded = 2,676 cf \times 40.0% Voids
#2A	0.50'	1,714 cf	Cultec R-330XLHD x 32 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 4 rows
•			

2,784 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area	

Discarded OutFlow Max=0.04 cfs @ 14.49 hrs HW=1.56' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.04 cfs)

Pond 4P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

8 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 57.50' Row Length +12.0" End Stone x 2 = 59.50' Base Length

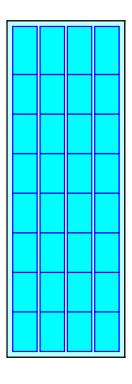
4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

32 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 1,713.7 cf Chamber Storage

4,390.2 cf Field - 1,713.7 cf Chambers = 2,676.5 cf Stone x 40.0% Voids = 1,070.6 cf Stone Storage

Chamber Storage + Stone Storage = 2,784.3 cf = 0.064 af Overall Storage Efficiency = 63.4% Overall System Size = 59.50' x 20.83' x 3.54'

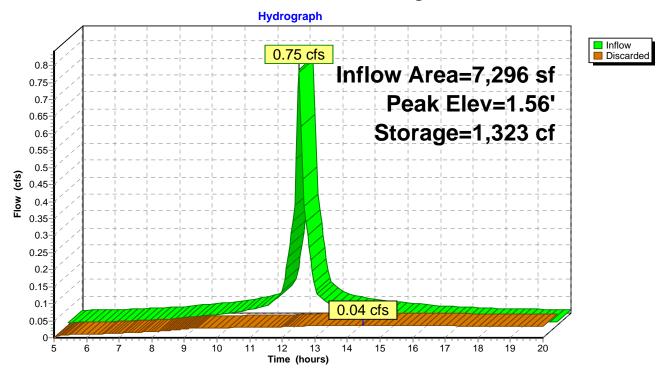
32 Chambers 162.6 cy Field 99.1 cy Stone





Page 144

Pond 4P: Roof Recharge



Type III 24-hr 10-Year Rainfall=4.70" Printed 3/9/2020

Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 145

Summary for Pond 5P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 4.15" for 10-Year event

Inflow = 0.12 cfs @ 12.09 hrs, Volume= 415 cf

Outflow = 0.01 cfs @ 13.70 hrs, Volume= 296 cf, Atten= 94%, Lag= 96.9 min

Discarded = 0.01 cfs @ 13.70 hrs, Volume= 296 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.62' @ 13.70 hrs Surf.Area= 200 sf Storage= 204 cf

Plug-Flow detention time= 169.7 min calculated for 295 cf (71% of inflow)

Center-of-Mass det. time= 103.7 min (839.2 - 735.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 13.70 hrs HW=1.62' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.01 cfs)

Page 146

Pond 5P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length

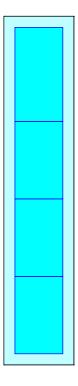
1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

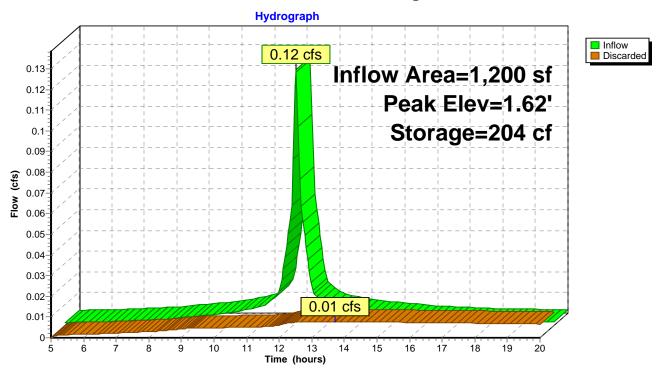
4 Chambers 26.2 cy Field 18.0 cy Stone





Page 147

Pond 5P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 148

Summary for Pond 6P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 4,059 sf,100.00% Impervious, Inflow Depth > 4.15" for 10-Year event

Inflow = 0.42 cfs @ 12.09 hrs, Volume= 1,402 cf

Outflow = 0.03 cfs @ 13.66 hrs, Volume= 1,067 cf, Atten= 94%, Lag= 94.2 min

Discarded = 0.03 cfs @ 13.66 hrs, Volume= 1,067 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.15' @ 13.66 hrs Surf.Area= 899 sf Storage= 650 cf

Plug-Flow detention time= 162.6 min calculated for 1,063 cf (76% of inflow)

Center-of-Mass det. time= 102.7 min (838.2 - 735.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	806 cf	11.17'W x 80.50'L x 3.54'H Field A
			$3,184 \text{ cf Overall - } 1,170 \text{ cf Embedded = } 2,014 \text{ cf } \times 40.0\% \text{ Voids}$
#2A	0.50'	1,170 cf	Cultec R-330XLHD x 22 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
	<u> </u>		

1,975 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.03 cfs @ 13.66 hrs HW=1.15' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.03 cfs)

Pond 6P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

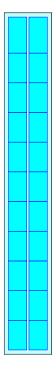
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

22 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,169.8 cf Chamber Storage

3,183.7 cf Field - 1,169.8 cf Chambers = 2,013.9 cf Stone x 40.0% Voids = 805.5 cf Stone Storage

Chamber Storage + Stone Storage = 1,975.3 cf = 0.045 af Overall Storage Efficiency = 62.0% Overall System Size = 80.50' x 11.17' x 3.54'

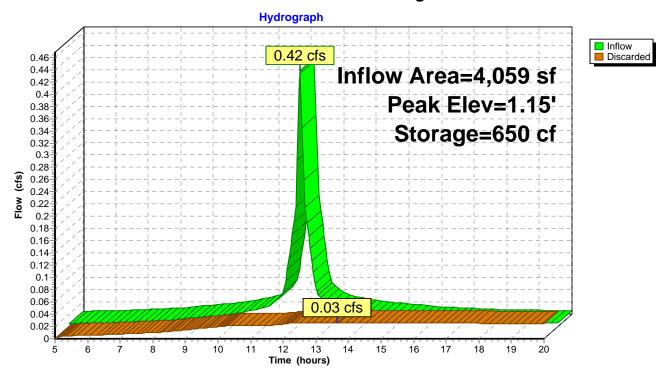
22 Chambers 117.9 cy Field 74.6 cy Stone





HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Pond 6P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 151

Summary for Pond 7P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 4,929 sf,100.00% Impervious, Inflow Depth > 4.15" for 10-Year event

Inflow = 0.51 cfs @ 12.09 hrs, Volume= 1,703 cf

Outflow = 0.03 cfs @ 14.00 hrs, Volume= 1,143 cf, Atten= 95%, Lag= 114.7 min

Discarded = 0.03 cfs @ 14.00 hrs, Volume= 1,143 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.43' @ 14.00 hrs Surf.Area= 899 sf Storage= 851 cf

Plug-Flow detention time= 166.5 min calculated for 1,139 cf (67% of inflow)

Center-of-Mass det. time= 95.4 min (830.9 - 735.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	806 cf	11.17'W x 80.50'L x 3.54'H Field A
			$3,184 \text{ cf Overall - } 1,170 \text{ cf Embedded = } 2,014 \text{ cf } \times 40.0\% \text{ Voids}$
#2A	0.50'	1,170 cf	Cultec R-330XLHD x 22 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
	•		—

1,975 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.03 cfs @ 14.00 hrs HW=1.43' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.03 cfs)

Pond 7P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

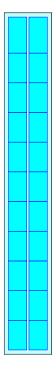
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

22 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,169.8 cf Chamber Storage

3,183.7 cf Field - 1,169.8 cf Chambers = 2,013.9 cf Stone x 40.0% Voids = 805.5 cf Stone Storage

Chamber Storage + Stone Storage = 1,975.3 cf = 0.045 af Overall Storage Efficiency = 62.0% Overall System Size = 80.50' x 11.17' x 3.54'

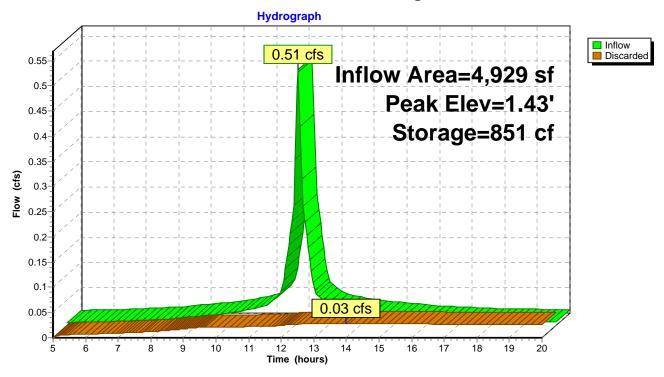
22 Chambers 117.9 cy Field 74.6 cy Stone





Page 153

Pond 7P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 154

Summary for Pond 8P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 4.15" for 10-Year event

Inflow = 0.12 cfs @ 12.09 hrs, Volume= 415 cf

Outflow = 0.01 cfs @ 13.70 hrs, Volume= 296 cf, Atten= 94%, Lag= 96.9 min

Discarded = 0.01 cfs @ 13.70 hrs, Volume= 296 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.62' @ 13.70 hrs Surf.Area= 200 sf Storage= 204 cf

Plug-Flow detention time= 169.7 min calculated for 295 cf (71% of inflow)

Center-of-Mass det. time= 103.7 min (839.2 - 735.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 13.70 hrs HW=1.62' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.01 cfs)

Pond 8P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

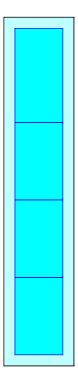
Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

- 4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length
- 1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height
- 4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

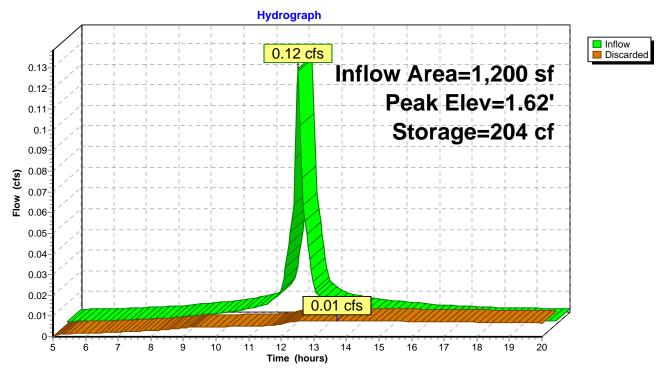
Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

4 Chambers 26.2 cy Field 18.0 cy Stone





Pond 8P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 157

Summary for Pond 9P: BASIN E

Inflow Area = 135,239 sf, 32.90% Impervious, Inflow Depth > 1.97" for 10-Year event
Inflow = 7.54 cfs @ 12.10 hrs, Volume= 22,174 cf
Outflow = 0.37 cfs @ 15.42 hrs, Volume= 11,434 cf, Atten= 95%, Lag= 199.6 min
Discarded = 0.37 cfs @ 15.42 hrs, Volume= 11,434 cf
Primary = 0.00 cfs @ 5.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 240.40' @ 15.42 hrs Surf.Area= 6,410 sf Storage= 13,055 cf

Plug-Flow detention time= 213.4 min calculated for 11,434 cf (52% of inflow) Center-of-Mass det. time= 129.6 min (931.6 - 801.9)

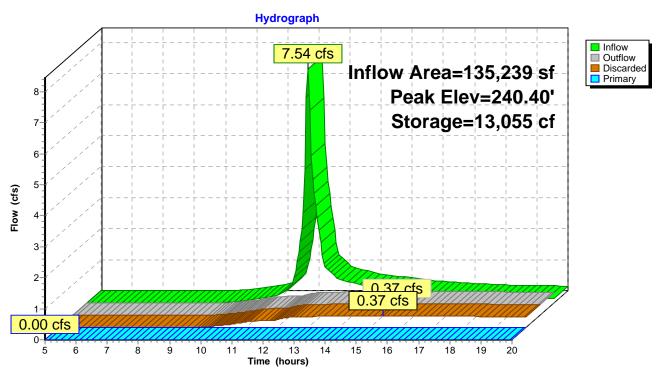
l below (Recalc	a (Irregular)Listed	Custom Stage Data	4,841 cf	237.80' 2	#1 237	
Wet.Area (sq-ft)	Cum.Store (cubic-feet)	Inc.Store (cubic-feet)	Perim. (feet)	Surf.Area (sq-ft)	Elevation (feet)	
3,709	0	0	322.6	3,709	237.80	
3,916	761	761	326.4	3,904	238.00	
4,980	5,159	4,398	345.3	4,911	239.00	
6,110	10,594	5,435	364.3	5,976	240.00	
7,289	17,122	6,528	383.1	7,097	241.00	
10,360	24,841	7,719	430.2	10,147	241.90	

Device	Routing	invert	Outlet Devices
#1	Discarded	237.80'	2.410 in/hr Exfiltration over Wetted area
#2	Primary	240.90'	25.0' long x 18.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Discarded OutFlow Max=0.37 cfs @ 15.42 hrs HW=240.40' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.37 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=237.80' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 9P: BASIN E



Type III 24-hr 10-Year Rainfall=4.70" Printed 3/9/2020

Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 159

Summary for Pond 10P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 4.15" for 10-Year event

Inflow = 0.12 cfs @ 12.09 hrs, Volume= 415 cf

Outflow = 0.01 cfs @ 13.70 hrs, Volume= 296 cf, Atten= 94%, Lag= 96.9 min

Discarded = 0.01 cfs @ 13.70 hrs, Volume= 296 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.62' @ 13.70 hrs Surf.Area= 200 sf Storage= 204 cf

Plug-Flow detention time= 169.7 min calculated for 295 cf (71% of inflow)

Center-of-Mass det. time= 103.7 min (839.2 - 735.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 13.70 hrs HW=1.62' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.01 cfs)

Pond 10P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length

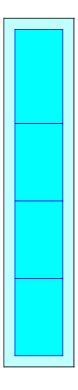
1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

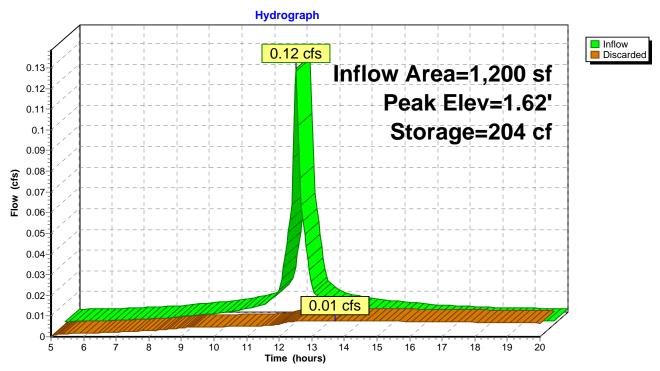
4 Chambers 26.2 cy Field 18.0 cy Stone





Page 161

Pond 10P: Roof Recharge



Type III 24-hr 10-Year Rainfall=4.70"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Page 162

Summary for Pond 11P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 4,059 sf,100.00% Impervious, Inflow Depth > 4.15" for 10-Year event

Inflow = 0.42 cfs @ 12.09 hrs, Volume= 1,402 cf

Outflow = 0.03 cfs @ 13.66 hrs, Volume= 1,067 cf, Atten= 94%, Lag= 94.2 min

Discarded = 0.03 cfs @ 13.66 hrs, Volume= 1,067 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.15' @ 13.66 hrs Surf.Area= 899 sf Storage= 650 cf

Plug-Flow detention time= 162.6 min calculated for 1,063 cf (76% of inflow)

Center-of-Mass det. time= 102.7 min (838.2 - 735.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	806 cf	11.17'W x 80.50'L x 3.54'H Field A
			3,184 cf Overall - 1,170 cf Embedded = 2,014 cf x 40.0% Voids
#2A	0.50'	1,170 cf	Cultec R-330XLHD x 22 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
	•		

1,975 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.03 cfs @ 13.66 hrs HW=1.15' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.03 cfs)

Pond 11P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

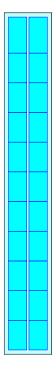
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

22 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,169.8 cf Chamber Storage

3,183.7 cf Field - 1,169.8 cf Chambers = 2,013.9 cf Stone x 40.0% Voids = 805.5 cf Stone Storage

Chamber Storage + Stone Storage = 1,975.3 cf = 0.045 af Overall Storage Efficiency = 62.0% Overall System Size = 80.50' x 11.17' x 3.54'

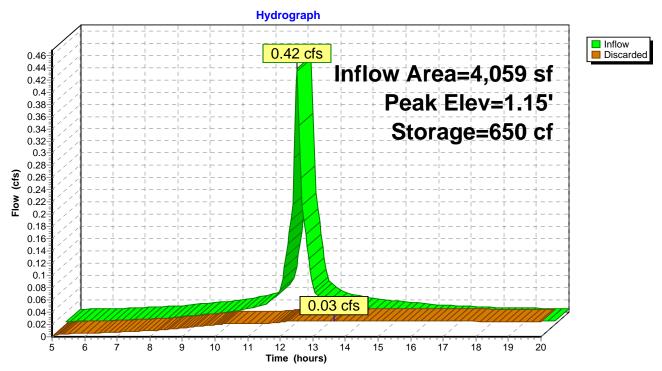
22 Chambers 117.9 cy Field 74.6 cy Stone





Page 164

Pond 11P: Roof Recharge



Type III 24-hr 10-Year Rainfall=4.70"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Page 165

Summary for Pond 13P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 4.15" for 10-Year event

Inflow = 0.12 cfs @ 12.09 hrs, Volume= 415 cf

Outflow = 0.01 cfs @ 13.70 hrs, Volume= 296 cf, Atten= 94%, Lag= 96.9 min

Discarded = 0.01 cfs @ 13.70 hrs, Volume= 296 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.62' @ 13.70 hrs Surf.Area= 200 sf Storage= 204 cf

Plug-Flow detention time= 169.7 min calculated for 295 cf (71% of inflow)

Center-of-Mass det. time= 103.7 min (839.2 - 735.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 13.70 hrs HW=1.62' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.01 cfs)

Page 166

Pond 13P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length

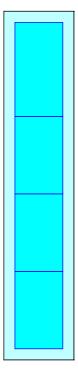
1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

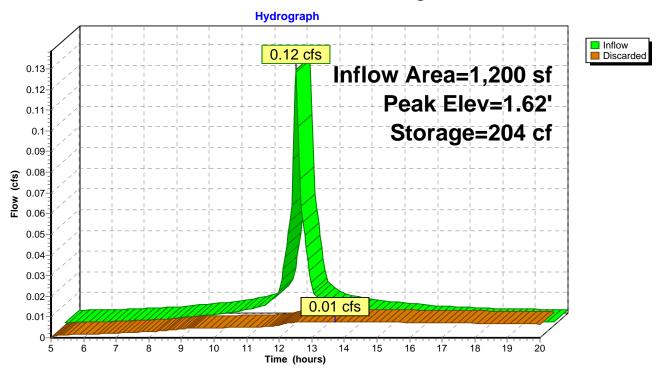
Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

4 Chambers 26.2 cy Field 18.0 cy Stone





Pond 13P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 168

Summary for Pond 14P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 4,059 sf,100.00% Impervious, Inflow Depth > 4.15" for 10-Year event

Inflow = 0.42 cfs @ 12.09 hrs, Volume= 1,402 cf

Outflow = 0.02 cfs @ 13.88 hrs, Volume= 996 cf, Atten= 94%, Lag= 107.6 min

Discarded = 0.02 cfs @ 13.88 hrs, Volume= 996 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.24' @ 13.88 hrs Surf.Area= 840 sf Storage= 675 cf

Plug-Flow detention time= 163.9 min calculated for 992 cf (71% of inflow)

Center-of-Mass det. time= 97.5 min (833.0 - 735.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	738 cf	16.00'W x 52.50'L x 3.54'H Field A
			2,975 cf Overall - 1,129 cf Embedded = 1,846 cf x 40.0% Voids
#2A	0.50'	1,129 cf	Cultec R-330XLHD x 21 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 3 rows
•		1 007 (T . I A . II I I O.

1,867 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.02 cfs @ 13.88 hrs HW=1.24' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.02 cfs)

Pond 14P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 3 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

7 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 50.50' Row Length +12.0" End Stone x 2 = 52.50' Base Length

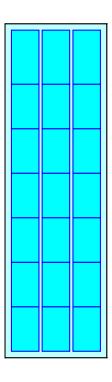
3 Rows x 52.0" Wide + 6.0" Spacing x 2 + 12.0" Side Stone x 2 = 16.00' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

21 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 3 Rows = 1,128.8 cf Chamber Storage

2,975.0 cf Field - 1,128.8 cf Chambers = 1,846.2 cf Stone x 40.0% Voids = 738.5 cf Stone Storage

Chamber Storage + Stone Storage = 1,867.3 cf = 0.043 af Overall Storage Efficiency = 62.8% Overall System Size = 52.50' x 16.00' x 3.54'

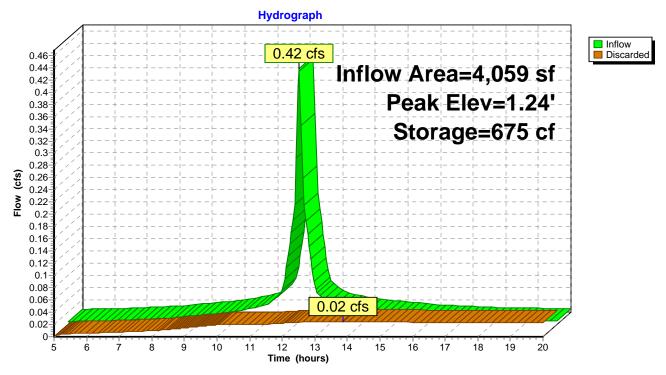
21 Chambers 110.2 cy Field 68.4 cy Stone





Page 170

Pond 14P: Roof Recharge



Type III 24-hr 10-Year Rainfall=4.70"

POST-DEVELOPMENT-REV1

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 171

Printed 3/9/2020

Summary for Pond 17P: Roof Recharge

Inflow Area = 13,086 sf, 37.67% Impervious, Inflow Depth > 3.09" for 10-Year event

Inflow = 1.12 cfs @ 12.09 hrs, Volume= 3,373 cf

Outflow = 0.05 cfs @ 15.19 hrs, Volume= 1,791 cf, Atten= 96%, Lag= 186.1 min

Discarded = 0.05 cfs @ 15.19 hrs, Volume= 1,791 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.66' @ 15.19 hrs Surf.Area= 1,677 sf Storage= 1,935 cf

Plug-Flow detention time= 186.6 min calculated for 1,791 cf (53% of inflow)

Center-of-Mass det. time= 105.8 min (877.5 - 771.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,440 cf	20.83'W x 80.50'L x 3.54'H Field A
			$5,940 \text{ cf Overall - } 2,340 \text{ cf Embedded = } 3,600 \text{ cf } \times 40.0\% \text{ Voids}$
#2A	0.50'	2,340 cf	Cultec R-330XLHD x 44 Inside #1
			Effective Size= 47.8 "W x 30.0 "H => 7.45 sf x 7.00 'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 4 rows
		0.700 - (Tatal A silable Otanana

3,780 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.05 cfs @ 15.19 hrs HW=1.66' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.05 cfs)

Page 172

Pond 17P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

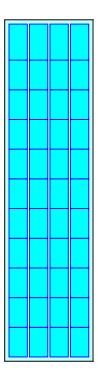
4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

44 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 2,339.6 cf Chamber Storage

5,939.7 cf Field - 2,339.6 cf Chambers = 3,600.1 cf Stone x 40.0% Voids = 1,440.0 cf Stone Storage

Chamber Storage + Stone Storage = 3,779.6 cf = 0.087 af Overall Storage Efficiency = 63.6% Overall System Size = 80.50' x 20.83' x 3.54'

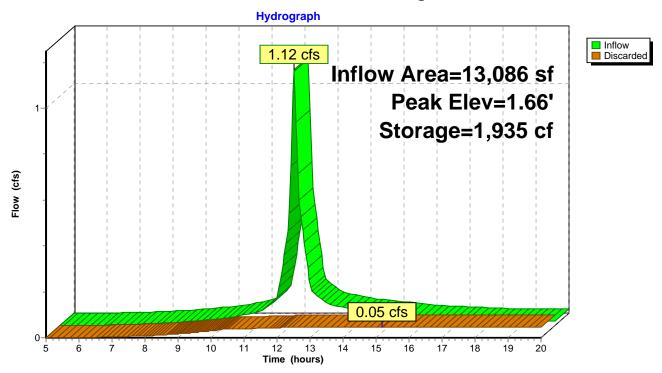
44 Chambers 220.0 cy Field 133.3 cy Stone





Page 173

Pond 17P: Roof Recharge



Type III 24-hr 10-Year Rainfall=4.70"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Page 174

Summary for Pond 19P: Roof Recharge

Inflow Area = 11,720 sf, 42.06% Impervious, Inflow Depth > 3.19" for 10-Year event

Inflow = 1.02 cfs @ 12.09 hrs, Volume= 3,116 cf

Outflow = 0.08 cfs @ 13.19 hrs, Volume= 2,814 cf, Atten= 92%, Lag= 66.2 min

Discarded = 0.08 cfs @ 13.19 hrs, Volume= 2,814 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.69' @ 13.19 hrs Surf.Area= 1,176 sf Storage= 1,368 cf

Plug-Flow detention time= 153.5 min calculated for 2,805 cf (90% of inflow)

Center-of-Mass det. time= 121.6 min (890.3 - 768.8)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,027 cf	16.00'W x 73.50'L x 3.54'H Field A
			$4,165 \text{ cf Overall - } 1,598 \text{ cf Embedded = } 2,567 \text{ cf } \times 40.0\% \text{ Voids}$
#2A	0.50'	1,598 cf	Cultec R-330XLHD x 30 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 3 rows
		0.005 (T

2,625 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.08 cfs @ 13.19 hrs HW=1.69' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.08 cfs)

Pond 19P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 3 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

10 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 71.50' Row Length +12.0" End Stone x 2 = 73.50' Base Length

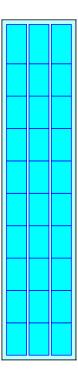
3 Rows x 52.0" Wide + 6.0" Spacing x 2 + 12.0" Side Stone x 2 = 16.00' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

30 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 3 Rows = 1,598.2 cf Chamber Storage

4,165.0 cf Field - 1,598.2 cf Chambers = 2,566.8 cf Stone x 40.0% Voids = 1,026.7 cf Stone Storage

Chamber Storage + Stone Storage = 2,624.9 cf = 0.060 af Overall Storage Efficiency = 63.0% Overall System Size = 73.50' x 16.00' x 3.54'

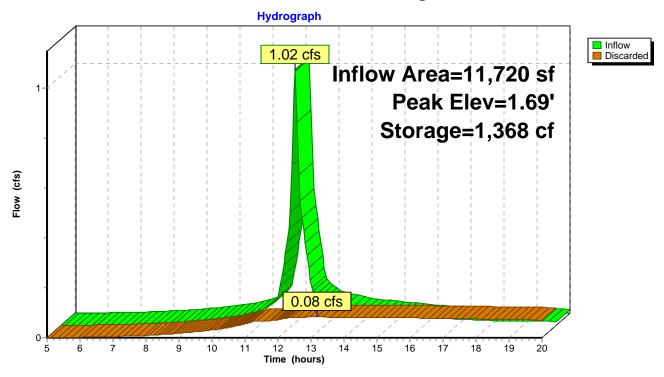
30 Chambers 154.3 cy Field 95.1 cy Stone





Page 176

Pond 19P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 177

Summary for Pond 21P: BASIN B

Inflow Area = 167,695 sf, 37.24% Impervious, Inflow Depth > 1.48" for 10-Year event
Inflow = 5.31 cfs @ 12.18 hrs, Volume= 20,630 cf
Outflow = 0.37 cfs @ 15.60 hrs, Volume= 11,385 cf, Atten= 93%, Lag= 205.2 min
Discarded = 0.00 cfs @ 5.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 227.44' @ 15.60 hrs Surf.Area= 6,856 sf Storage= 11,502 cf

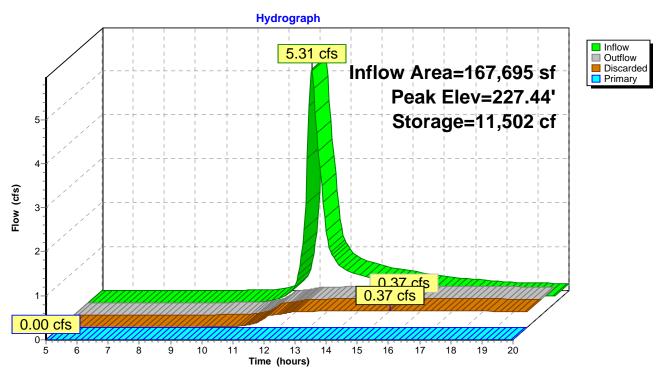
Plug-Flow detention time= 209.3 min calculated for 11,347 cf (55% of inflow) Center-of-Mass det. time= 124.6 min (943.6 - 819.1)

<u>Volume</u>	Inve	<u>rt Avail</u>	.Storage	e Storage Description			
#1	225.50	0' 2	23,381 cf	Custom Stage Da	ata (Irregular)Liste	ed below (Recalc)	
Elevatio		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
225.5 226.0 227.0 228.0 228.9	50 00 00	5,039 5,488 6,427 7,423 10,160	294.3 303.7 322.5 328.3 388.5	0 2,631 5,951 6,919 7,880	2,631 8,582 15,501 23,381	5,039 5,511 6,498 6,941 10,390	
<u>Device</u> #1 #2	Routing Discarded Primary		50' 2.41	et Devices 0 in/hr Exfiltration ' long x 21.0' brea		a ed Rectangular Weir	
			Head	d (feet) 0.20 0.40	0.60 0.80 1.00	•	

Discarded OutFlow Max=0.37 cfs @ 15.60 hrs HW=227.44' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.37 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=225.50' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 21P: BASIN B



Type III 24-hr 10-Year Rainfall=4.70"

POST-DEVELOPMENT-REV1

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020

Page 179

Summary for Pond 22P: Roof Recharge

Inflow Area = 11,373 sf, 43.34% Impervious, Inflow Depth > 2.29" for 10-Year event

Inflow = 0.74 cfs @ 12.09 hrs, Volume= 2,168 cf

Outflow = 0.04 cfs @ 15.24 hrs, Volume= 1,180 cf, Atten= 95%, Lag= 189.0 min

Discarded = 0.04 cfs @ 15.24 hrs, Volume= 1.180 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.62' @ 15.24 hrs Surf.Area= 1,133 sf Storage= 1,238 cf

Plug-Flow detention time= 201.3 min calculated for 1,176 cf (54% of inflow)

Center-of-Mass det. time= 121.7 min (915.3 - 793.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,013 cf	11.17'W x 101.50'L x 3.54'H Field A
			4,014 cf Overall - 1,483 cf Embedded = 2,531 cf x 40.0% Voids
#2A	0.50'	1,483 cf	Cultec R-330XLHD x 28 Inside #1
			Effective Size= 47.8 "W x 30.0 "H => 7.45 sf x 7.00 'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
		0.405 -(Tatal A silable Otanana

2,495 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.04 cfs @ 15.24 hrs HW=1.62' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.04 cfs)

Pond 22P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

14 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 99.50' Row Length +12.0" End Stone x 2 = 101.50' Base Length

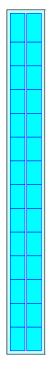
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

28 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,482.7 cf Chamber Storage

4,014.2 cf Field - 1,482.7 cf Chambers = 2,531.4 cf Stone x 40.0% Voids = 1,012.6 cf Stone Storage

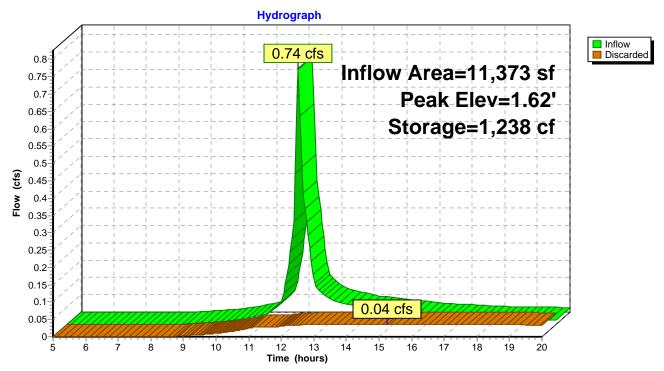
Chamber Storage + Stone Storage = 2,495.3 cf = 0.057 af Overall Storage Efficiency = 62.2% Overall System Size = 101.50' x 11.17' x 3.54'

28 Chambers 148.7 cy Field 93.8 cy Stone



Page 181

Pond 22P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 182

Summary for Pond 30P: BASIN A

[82] Warning: Early inflow requires earlier time span

Inflow Area = 18,459 sf, 78.24% Impervious, Inflow Depth > 3.78" for 10-Year event
Inflow = 2.10 cfs @ 12.00 hrs, Volume= 5,810 cf
Outflow = 0.19 cfs @ 12.71 hrs, Volume= 5,716 cf, Atten= 91%, Lag= 42.7 min
Discarded = 0.00 cfs @ 5.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 239.45' @ 12.71 hrs Surf.Area= 2,481 sf Storage= 2,424 cf

Plug-Flow detention time= 118.6 min calculated for 5,715 cf (98% of inflow)

Center-of-Mass det. time= 111.3 min (853.7 - 742.3)

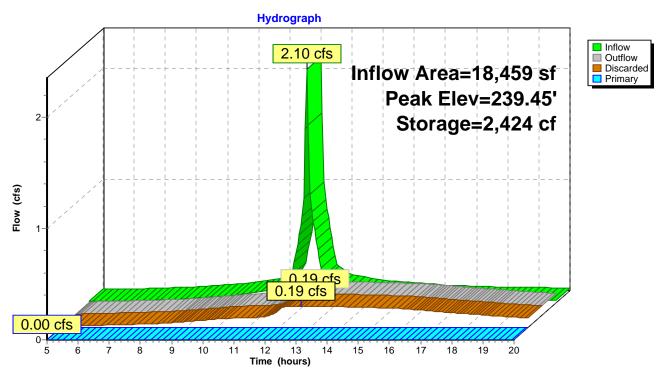
Volume	Invert	Avail	.Storage	Storage Description	on		
#1	238.20'		7,401 cf	Custom Stage Da	ata (Irregular)Liste	ed below (Recalc)	
Elevatio	_	urf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
238.20 1,411		262.1	0	0	1,411		
239.0	00	2,083	297.8	1,389	1,389	3,017	
240.0	00	3,005	316.6	2,530	3,919	3,986	
241.0	00	3,983	335.5	3,483	7,401	5,020	
Device	Routing	lnv	ert Outle	et Devices			_
#1	t1 Discarded 238.20' 2.4		20' 2.41	410 in/hr Exfiltration over Wetted area			
#2	#2 Primary 240.00'		00' 25.0 '	25.0' long x 21.0' breadth Broad-Crested Rectangular Weir			
	_		Head	d (feet) 0.20 0.40	0.60 0.80 1.00 1	.20 1.40 1.60	
			Coef	f. (English) 2.68 2.	70 2.70 2.64 2.6	3 2.64 2.64 2.63	

Discarded OutFlow Max=0.19 cfs @ 12.71 hrs HW=239.45' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.19 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=238.20' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 183

Pond 30P: BASIN A



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 184

Summary for Pond 31P: BASIN D

Inflow Area = 90,014 sf, 45.84% Impervious, Inflow Depth > 2.62" for 10-Year event
Inflow = 6.63 cfs @ 12.09 hrs, Volume= 19,639 cf
Outflow = 0.69 cfs @ 12.95 hrs, Volume= 18,717 cf, Atten= 90%, Lag= 51.4 min
Discarded = 0.00 cfs @ 5.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 240.26' @ 12.95 hrs Surf.Area= 8,298 sf Storage= 8,586 cf

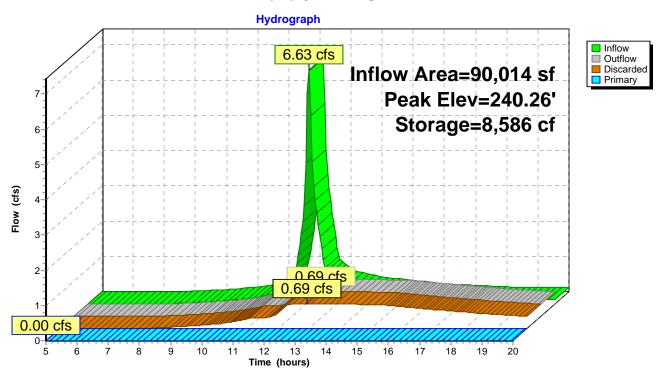
Plug-Flow detention time= 131.6 min calculated for 18,655 cf (95% of inflow) Center-of-Mass det. time= 114.2 min (899.1 - 784.9)

Volume	Inver	t Avail.	Storage	rage Storage Description				
#1 239.00' 24,822		4,822 cf	cf Custom Stage Data (Irregular)Listed below (Recalc)					
Elevatio	_	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
239.0	00	5,028	276.6	0	0	5,028		
240.0	00	7,979	406.4	6,447	6,447	12,091		
241.0	00	9,227	425.3	8,595	15,042	13,409		
241.9	90	12,593	472.4	9,780	24,822	16,798		
Device	Routing	Inve	ert Outle	et Devices				
#1	Discarded	239.0	00' 2.41	0 in/hr Exfiltration	over Wetted are	a		
#2 Primary 240.90' 25.0' long x 21.0' breadth Broad-Crested Rectangula Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64			1.20 1.40 1.60	r				

Discarded OutFlow Max=0.69 cfs @ 12.95 hrs HW=240.26' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.69 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=239.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 31P: BASIN D



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 186

Summary for Pond 32P: BASIN C

Inflow Area = 188,274 sf, 48.56% Impervious, Inflow Depth > 2.03" for 10-Year event Inflow = 9.35 cfs @ 12.14 hrs, Volume= 31,885 cf Outflow = 1.32 cfs @ 12.94 hrs, Volume= 30,126 cf, Atten= 86%, Lag= 48.0 min Discarded = 1.32 cfs @ 12.94 hrs, Volume= 30,126 cf Primary = 0.00 cfs @ 5.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 225.42' @ 12.94 hrs Surf.Area= 13,698 sf Storage= 13,706 cf

Plug-Flow detention time= 124.2 min calculated for 30,026 cf (94% of inflow) Center-of-Mass det. time= 104.7 min (907.3 - 802.6)

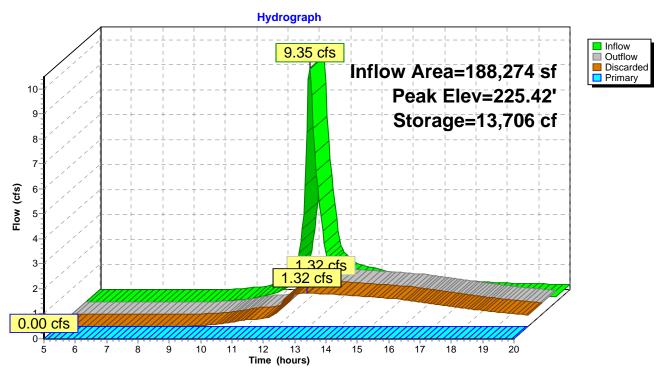
Volume	Inver	t Avail	.Storage	Storage Description	n		
#1	224.00	' 3	38,476 cf	Custom Stage Da	ita (Irregular) List	ed below (Recalc)	
Elevatio		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
224.00)	4,753	268.4	0	0	4,753	
225.00)	12,400	506.3	8,277	8,277	19,424	
226.00)	15,628	628.5	13,983	22,260	30,474	
226.90)	20,519	675.7	16,216	38,476	35,408	
Device	Routing	Inv	ert Outle	et Devices			
#1	Discarded	224.	00' 2.41	0 in/hr Exfiltration	over Wetted are	a	
#2	Primary	225.	90' 25.0 '	' long x 21.0' brea	dth Broad-Crest	ed Rectangular Weir	

Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.63

Discarded OutFlow Max=1.32 cfs @ 12.94 hrs HW=225.42' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 1.32 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=224.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 32P: BASIN C



Subcatchment PRDA-1: TO BASIN D

Prepared by HP

Type III 24-hr 25-Year Rainfall=5.50" Printed 3/9/2020

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 188

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Reach routing by Stor-Ind	method - Pond routing by Stor-Ind method
Subcatchment34S: 10 Unit Roof	Runoff Area=10,448 sf 100.00% Impervious Runoff Depth>4.87" Tc=6.0 min CN=98 Runoff=1.26 cfs 4,244 cf
Subcatchment35S: 14 Unit Roof	Runoff Area=13,061 sf 100.00% Impervious Runoff Depth>4.87" Tc=6.0 min CN=98 Runoff=1.58 cfs 5,305 cf
Subcatchment 36S: 14 Unit Roof	Runoff Area=13,061 sf 100.00% Impervious Runoff Depth>4.87" Tc=6.0 min CN=98 Runoff=1.58 cfs 5,305 cf
Subcatchment 37S: 7 Unit Roof	Runoff Area=7,296 sf 100.00% Impervious Runoff Depth>4.87" Tc=6.0 min CN=98 Runoff=0.88 cfs 2,963 cf
Subcatchment LOT 1: Single Family House	Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>4.87" Tc=6.0 min CN=98 Runoff=0.14 cfs 487 cf
Subcatchment LOT 12: DUPLEX + YARD	Runoff Area=11,373 sf 43.34% Impervious Runoff Depth>2.93" Tc=6.0 min CN=78 Runoff=0.94 cfs 2,779 cf
Subcatchment LOT 13: DUPLEX + YARD	Runoff Area=11,720 sf 42.06% Impervious Runoff Depth>3.91" Tc=6.0 min CN=88 Runoff=1.24 cfs 3,823 cf
Subcatchment LOT 14: DUPLEX + YARD	Runoff Area=13,086 sf 37.67% Impervious Runoff Depth>3.81" Tc=6.0 min CN=87 Runoff=1.36 cfs 4,156 cf
Subcatchment LOT 18: DUPLEX ROOF	Runoff Area=4,929 sf 100.00% Impervious Runoff Depth>4.87" Tc=6.0 min CN=98 Runoff=0.60 cfs 2,002 cf
Subcatchment LOT 19: DUPLEX ROOF	Runoff Area=4,059 sf 100.00% Impervious Runoff Depth>4.87" Tc=6.0 min CN=98 Runoff=0.49 cfs 1,649 cf
Subcatchment LOT 2: Single Family House	Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>4.87" Tc=6.0 min CN=98 Runoff=0.14 cfs 487 cf
Subcatchment LOT 20: DUPLEX ROOF	Runoff Area=4,059 sf 100.00% Impervious Runoff Depth>4.87" Tc=6.0 min CN=98 Runoff=0.49 cfs 1,649 cf
Subcatchment LOT 21: DUPLEX ROOF	Runoff Area=4,059 sf 100.00% Impervious Runoff Depth>4.87" Tc=6.0 min CN=98 Runoff=0.49 cfs 1,649 cf
Subcatchment LOT 3: Single Family House	Se Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>4.87" Tc=6.0 min CN=98 Runoff=0.14 cfs 487 cf
Subcatchment LOT 8: Single Family House	Se Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>4.87" Tc=6.0 min CN=98 Runoff=0.14 cfs 487 cf

Runoff Area=51,731 sf 56.02% Impervious Runoff Depth>3.21"

Tc=6.0 min CN=81 Runoff=4.66 cfs 13,859 cf

POST-DEVELOPMENT-REV1 Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 Hydroc	Type III 24-hr 25-Year Rainfall=5.50" Printed 3/9/2020 CAD Software Solutions LLC Page 189
Subcatchment PRDA-10: TO ILSF	Runoff Area=14,656 sf 0.00% Impervious Runoff Depth>0.63" Flow Length=85' Tc=15.0 min CN=47 Runoff=0.13 cfs 767 cf
Subcatchment PRDA-11: BASIN A DIRECT	Runoff Area=8,000 sf 49.79% Impervious Runoff Depth>4.02" Tc=0.0 min CN=89 Runoff=1.01 cfs 2,682 cf
Subcatchment PRDA-12: BASIN B DIRECT	Runoff Area=16,454 sf 45.11% Impervious Runoff Depth>2.93" Tc=6.0 min CN=78 Runoff=1.36 cfs 4,021 cf
Subcatchment PRDA-13: BASIN C DIRECT	Runoff Area=31,453 sf 49.69% Impervious Runoff Depth>3.03" Tc=6.0 min CN=79 Runoff=2.68 cfs 7,930 cf
Subcatchment PRDA-2: TO BASIN A	Runoff Area=10,459 sf 100.00% Impervious Runoff Depth>4.87" Tc=0.0 min CN=98 Runoff=1.48 cfs 4,249 cf
	Runoff Area=151,241 sf 36.39% Impervious Runoff Depth>1.90" ow Length=520' Tc=13.1 min CN=66 Runoff=6.46 cfs 23,953 cf
	Runoff Area=156,821 sf 48.34% Impervious Runoff Depth>2.57" bw Length=370' Tc=10.4 min CN=74 Runoff=9.97 cfs 33,563 cf
Subcatchment PRDA-5: BASIN D DIRECT	Runoff Area=38,283 sf 32.09% Impervious Runoff Depth>3.41" Tc=6.0 min CN=83 Runoff=3.63 cfs 10,876 cf
Subcatchment PRDA-6: TO BASIN E	Runoff Area=135,239 sf 32.90% Impervious Runoff Depth>2.57" Tc=6.0 min CN=74 Runoff=9.87 cfs 28,989 cf

Subcatchment PRDA-7: TO WETLAND Runoff Area=149,386 sf 2.80% Impervious Runoff Depth>0.92" Tc=6.0 min CN=52 Runoff=3.23 cfs 11,466 cf

Subcatchment PRDA-8: TO WETLAND Runoff Area=46,646 sf 0.00% Impervious Runoff Depth>1.05" Tc=6.0 min CN=54 Runoff=1.21 cfs 4,069 cf

Subcatchment PRDA-9: TO WETLAND Runoff Area=80,362 sf 11.49% Impervious Runoff Depth>3.03" Tc=6.0 min CN=79 Runoff=6.85 cfs 20,262 cf

Reach 1R: ILSF

Inflow=0.13 cfs 767 cf

Outflow=0.13 cfs 767 cf

Reach 14R: WETLAND SOUTHInflow=6.85 cfs 20,262 cf

Outflow=6.85 cfs 20,262 cf

Reach 15R: WETLAND SOUTHEAST Inflow=1.21 cfs 6,608 cf
Outflow=1.21 cfs 6,608 cf

Reach 21R: WETLAND NORTHInflow=3.23 cfs 13,169 cf
Outflow=3.23 cfs 13,169 cf

Pond 1P: Roof Recharge Peak Elev=1.62' Storage=1,723 cf Inflow=1.26 cfs 4,244 cf

Outflow=0.10 cfs 3,967 cf

Pond 2P: Roof Recharge Peak Elev=1.86' Storage=2,977 cf Inflow=1.58 cfs 5,305 cf

Outflow=0.06 cfs 2,828 cf

POST-DEVELOPMENT-REV1 Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD	Type III 24-hr 25-Year Rainfall=5.50" Printed 3/9/2020 Software Solutions LLC Page 190
Pond 3P: Roof Recharge	Peak Elev=1.86' Storage=2,977 cf Inflow=1.58 cfs 5,305 cf Outflow=0.06 cfs 2,828 cf
Pond 4P: Roof Recharge	Peak Elev=1.89' Storage=1,652 cf Inflow=0.88 cfs 2,963 cf Outflow=0.04 cfs 1,611 cf
Pond 5P: Roof Recharge	Peak Elev=1.96' Storage=252 cf Inflow=0.14 cfs 487 cf Outflow=0.01 cfs 322 cf
Pond 6P: Roof Recharge	Peak Elev=1.38' Storage=811 cf Inflow=0.49 cfs 1,649 cf Outflow=0.03 cfs 1,133 cf
Pond 7P: Roof Recharge	Peak Elev=1.73' Storage=1,059 cf Inflow=0.60 cfs 2,002 cf Outflow=0.03 cfs 1,216 cf

Peak Elev=1.96' Storage=252 cf Inflow=0.14 cfs 487 cf Pond 8P: Roof Recharge Outflow=0.01 cfs 322 cf

Peak Elev=240.93' Storage=16,617 cf Inflow=9.87 cfs 28,989 cf Pond 9P: BASIN E Discarded=0.40 cfs 12,939 cf Primary=0.37 cfs 1,703 cf Outflow=0.77 cfs 14,643 cf

Peak Elev=1.96' Storage=252 cf Inflow=0.14 cfs 487 cf Pond 10P: Roof Recharge

Outflow=0.01 cfs 322 cf

Peak Elev=1.38' Storage=811 cf Inflow=0.49 cfs 1,649 cf Pond 11P: Roof Recharge

Outflow=0.03 cfs 1,133 cf

Peak Elev=1.96' Storage=252 cf Inflow=0.14 cfs 487 cf Pond 13P: Roof Recharge

Outflow=0.01 cfs 322 cf

Peak Elev=1.49' Storage=843 cf Inflow=0.49 cfs 1,649 cf Pond 14P: Roof Recharge

Outflow=0.02 cfs 1,053 cf

Peak Elev=2.12' Storage=2,525 cf Inflow=1.36 cfs 4,156 cf Pond 17P: Roof Recharge

Outflow=0.05 cfs 1,939 cf

Peak Elev=2.15' Storage=1,777 cf Inflow=1.24 cfs 3,823 cf Pond 19P: Roof Recharge

Outflow=0.09 cfs 3,076 cf

Peak Elev=227.93' Storage=15,017 cf Inflow=7.35 cfs 27,974 cf Pond 21P: BASIN B

Discarded=0.39 cfs 12,228 cf Primary=0.47 cfs 2,539 cf Outflow=0.85 cfs 14,768 cf

Peak Elev=2.16' Storage=1,692 cf Inflow=0.94 cfs 2,779 cf Pond 22P: Roof Recharge

Outflow=0.04 cfs 1,314 cf

Pond 30P: BASIN A Peak Elev=239.69' Storage=3,039 cf Inflow=2.49 cfs 6,931 cf

Discarded=0.21 cfs 6,497 cf Primary=0.00 cfs 0 cf Outflow=0.21 cfs 6,497 cf

Pond 31P: BASIN D Peak Elev=240.61' Storage=11,557 cf Inflow=8.29 cfs 24,735 cf

Discarded=0.72 cfs 21,915 cf Primary=0.00 cfs 0 cf Outflow=0.72 cfs 21,915 cf

Type III 24-hr 25-Year Rainfall=5.50"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Page 191

Pond 32P: BASIN C Peak Elev=225.78' Storage=18,870 cf Inflow=12.19 cfs 41,493 cf Discarded=1.55 cfs 37,848 cf Primary=0.00 cfs 0 cf Outflow=1.55 cfs 37,848 cf

Total Runoff Area = 992,682 sf Runoff Volume = 204,159 cf Average Runoff Depth = 2.47" 64.94% Pervious = 644,625 sf 35.06% Impervious = 348,057 sf

Page 192

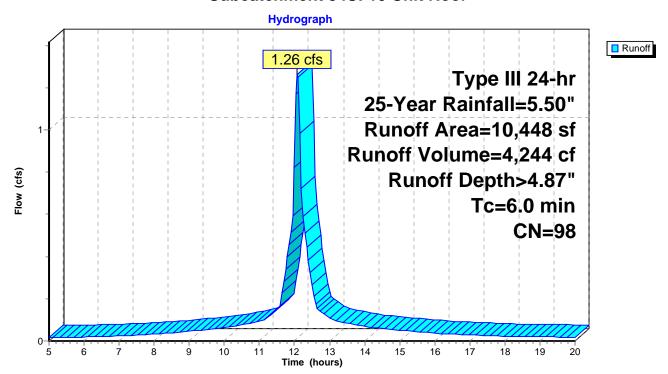
Summary for Subcatchment 34S: 10 Unit Roof

Runoff = 1.26 cfs @ 12.09 hrs, Volume= 4,244 cf, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

_	Α	rea (sf)	CN	Description		
*		10,448	98	Impervious		
		10,448		100.00% lm	npervious A	Area
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment 34S: 10 Unit Roof



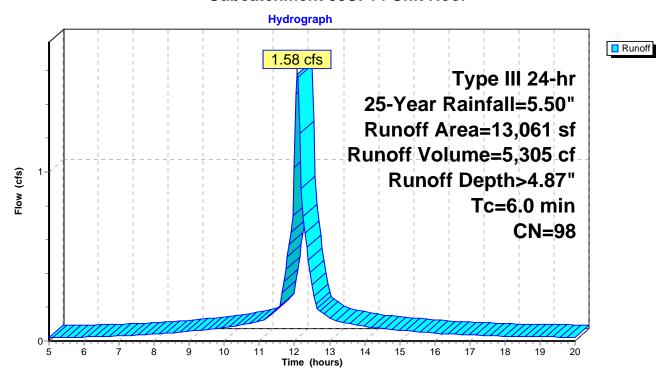
Summary for Subcatchment 35S: 14 Unit Roof

Runoff = 1.58 cfs @ 12.09 hrs, Volume= 5,305 cf, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

_	Α	rea (sf)	CN	Description		
*		13,061	98	mpervious		
		13,061		100.00% Im	npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment 35S: 14 Unit Roof



Page 194

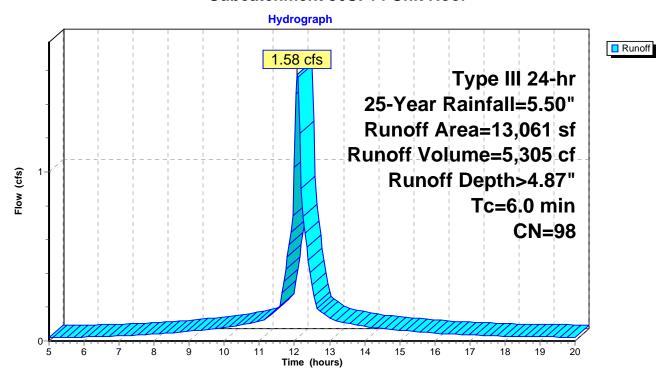
Summary for Subcatchment 36S: 14 Unit Roof

Runoff = 1.58 cfs @ 12.09 hrs, Volume= 5,305 cf, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

_	Α	rea (sf)	CN	Description		
*		13,061	98	Impervious		
		13,061		100.00% lm	npervious A	Area
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment 36S: 14 Unit Roof



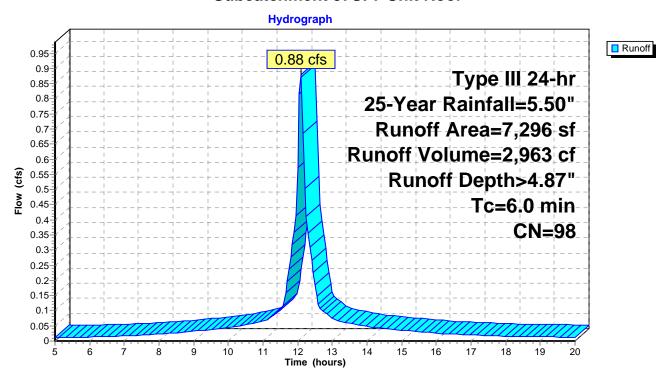
Summary for Subcatchment 37S: 7 Unit Roof

Runoff = 0.88 cfs @ 12.09 hrs, Volume= 2,963 cf, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Α	rea (sf)	CN I	Description		
*		7,296	98	mpervious		
		7,296		100.00% Im	npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment 37S: 7 Unit Roof



Page 196

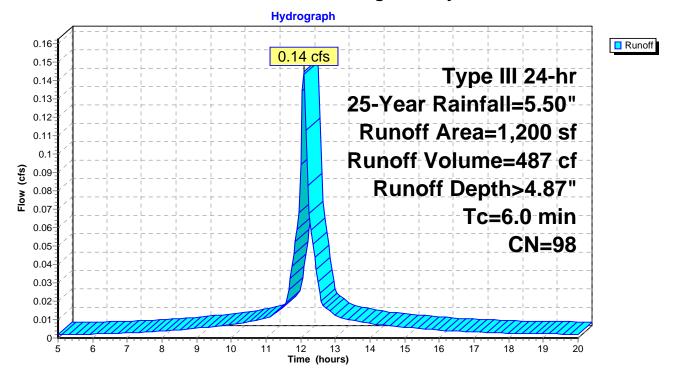
Summary for Subcatchment LOT 1: Single Family House

Runoff = 0.14 cfs @ 12.09 hrs, Volume= 487 cf, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

_	Α	rea (sf)	CN	Description					
*		1,200	98	Impervious					
		1,200	,	100.00% Impervious Area					
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 1: Single Family House



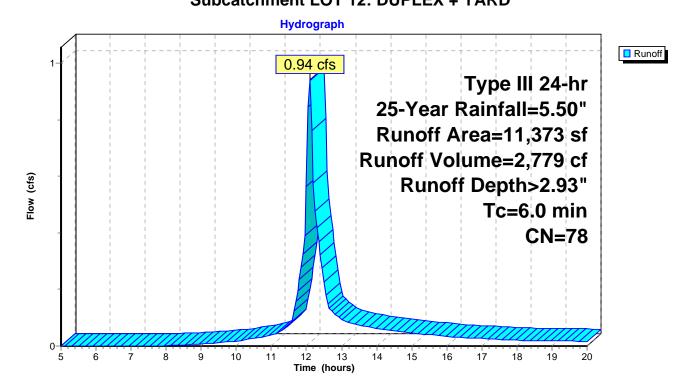
Summary for Subcatchment LOT 12: DUPLEX + YARD

Runoff = 0.94 cfs @ 12.09 hrs, Volume= 2,779 cf, Depth> 2.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Area (sf)	CN	Description							
*	4,929	98	Impervious							
	490	80	>75% Grass cover, Good, HSG D							
	5,954	61	>75% Grass	>75% Grass cover, Good, HSG B						
	11,373	78	Weighted Average							
	6,444		56.66% Pervious Area							
	4,929		43.34% Imp	ervious Ar	ea					
Tc (min)	- 3	Slope	,	Capacity (cfs)	Description					
	(.001)	(1010	, (.9000)	(0.0)	Direct Entry.					
Tc (min) 6.0	,		e Velocity							

Subcatchment LOT 12: DUPLEX + YARD



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 198

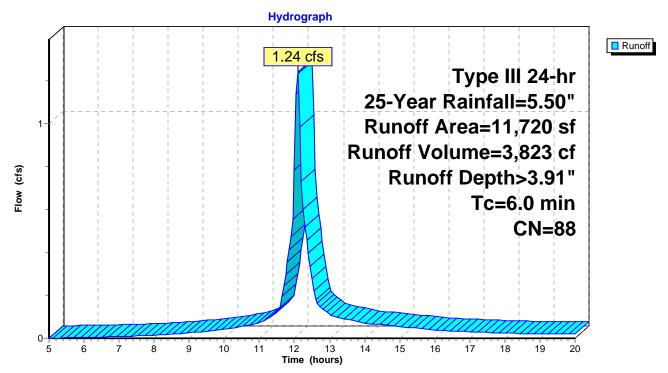
Summary for Subcatchment LOT 13: DUPLEX + YARD

Runoff = 1.24 cfs @ 12.09 hrs, Volume= 3,823 cf, Depth> 3.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Α	rea (sf)	CN	Description						
*		4,929	98	Impervious						
		6,791	80	>75% Grass cover, Good, HSG D						
		11,720	88	Veighted Average						
		6,791		57.94% Pervious Area						
		4,929		42.06% lmp	pervious Ar	rea				
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft	(ft/sec)	(cfs)					
	6.0					Direct Entry,				

Subcatchment LOT 13: DUPLEX + YARD



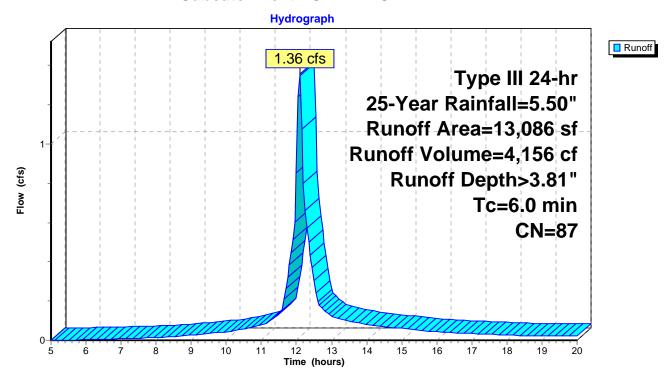
Summary for Subcatchment LOT 14: DUPLEX + YARD

Runoff = 1.36 cfs @ 12.09 hrs, Volume= 4,156 cf, Depth> 3.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Area	(sf) CN	N D	Description						
*	4,9	929 98	8 In	Impervious						
	8,1	57 80	0 >	>75% Grass cover, Good, HSG D						
	13,0)86 87	7 W	eighted A	verage					
	8,1	57	62	62.33% Pervious Area						
	4,9	929	37	7.67% Imp	ervious Are	rea				
	Tc Le	ngth S	Slope	Velocity	Capacity	Description				
_	(min) (1	eet) ((ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry,				

Subcatchment LOT 14: DUPLEX + YARD



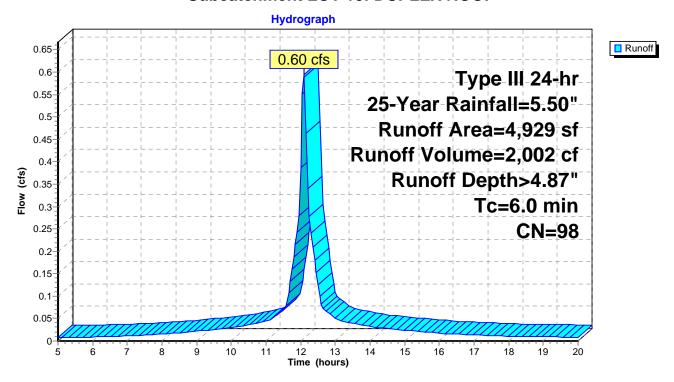
Summary for Subcatchment LOT 18: DUPLEX ROOF

Runoff = 0.60 cfs @ 12.09 hrs, Volume= 2,002 cf, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Α	rea (sf)	CN I	Description					
*		4,929	98	Impervious					
		4,929		100.00% Im	npervious A	Area			
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 18: DUPLEX ROOF



Page 201

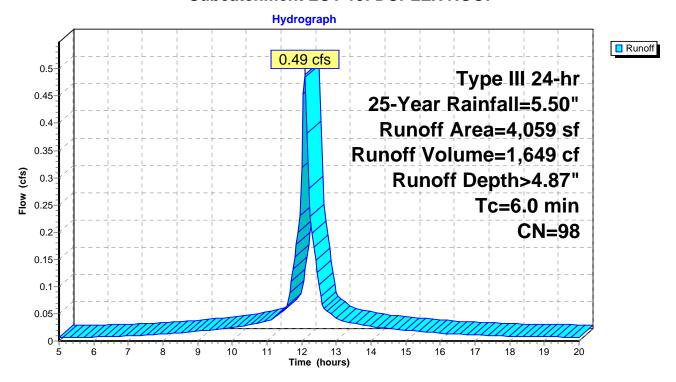
Summary for Subcatchment LOT 19: DUPLEX ROOF

Runoff = 0.49 cfs @ 12.09 hrs, Volume= 1,649 cf, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Α	rea (sf)	CN I	Description		
*		4,059	98 I	mpervious		
		4,059	•	100.00% Im	npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment LOT 19: DUPLEX ROOF



Page 202

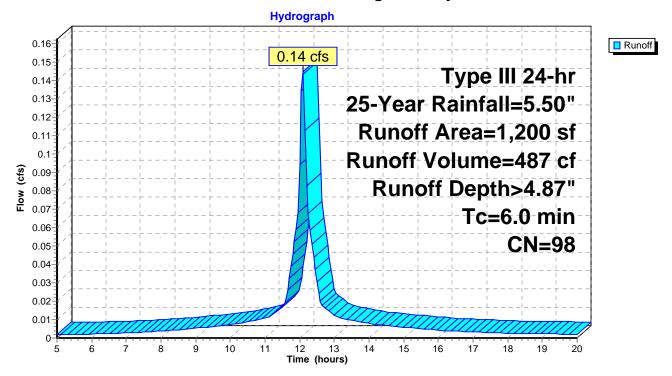
Summary for Subcatchment LOT 2: Single Family House

Runoff = 0.14 cfs @ 12.09 hrs, Volume= 487 cf, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Α	rea (sf)	CN I	Description				
*		1,200	98	Impervious				
		1,200		100.00% Im	npervious A	Area		
	Тс	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	6.0					Direct Entry,		

Subcatchment LOT 2: Single Family House



Page 203

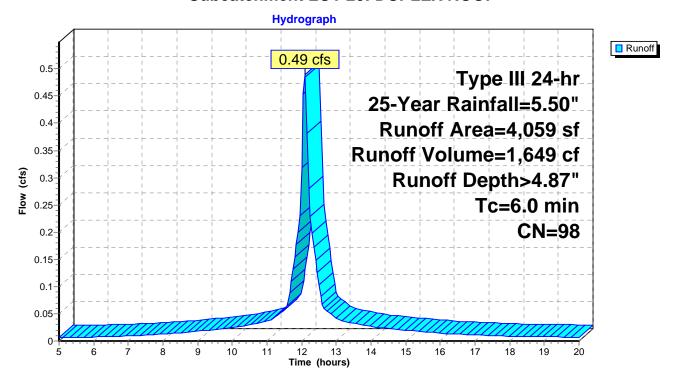
Summary for Subcatchment LOT 20: DUPLEX ROOF

Runoff = 0.49 cfs @ 12.09 hrs, Volume= 1,649 cf, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Α	rea (sf)	CN	Description		
*		4,059	98	mpervious		
		4,059	,	100.00% Im	npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment LOT 20: DUPLEX ROOF



Page 204

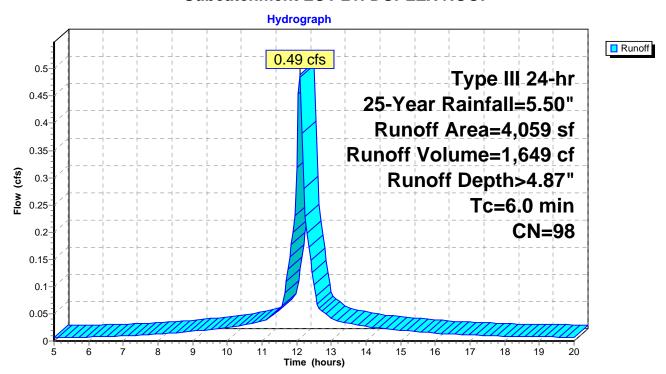
Summary for Subcatchment LOT 21: DUPLEX ROOF

Runoff = 0.49 cfs @ 12.09 hrs, Volume= 1,649 cf, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

_	Α	rea (sf)	CN	Description					
*		4,059	98	Impervious					
		4,059		100.00% Impervious Area					
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 21: DUPLEX ROOF



Page 205

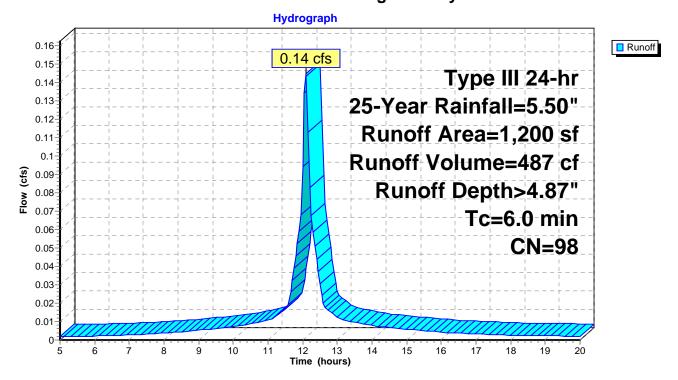
Summary for Subcatchment LOT 3: Single Family House

Runoff = 0.14 cfs @ 12.09 hrs, Volume= 487 cf, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

_	Α	rea (sf)	CN	Description					
*		1,200	98	Impervious					
		1,200	,	100.00% Impervious Area					
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 3: Single Family House



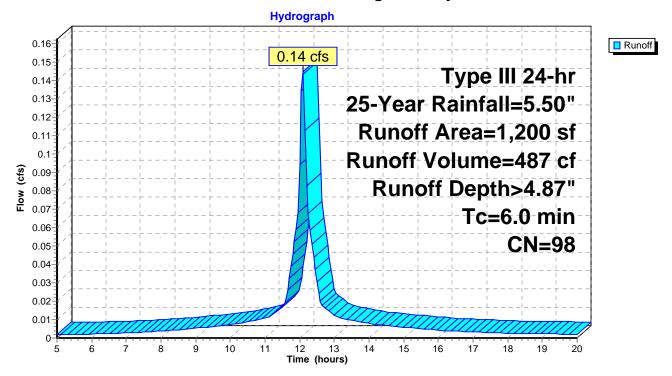
Summary for Subcatchment LOT 8: Single Family House

Runoff = 0.14 cfs @ 12.09 hrs, Volume= 487 cf, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Α	rea (sf)	CN I	Description					
*		1,200	98	mpervious					
		1,200		100.00% Impervious Area					
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 8: Single Family House



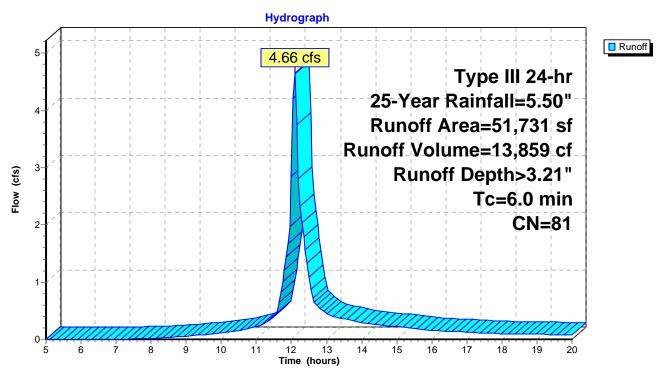
Summary for Subcatchment PRDA-1: TO BASIN D

Runoff = 4.66 cfs @ 12.09 hrs, Volume= 13,859 cf, Depth> 3.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Area (sf)	CN	Description					
*	25,896	98	Impervious					
	6,014	54	1/2 acre lot	s, 25% imp	p, HSG A			
	6,314	80	1/2 acre lot	s, 25% imp	p, HSG C			
	6,846	39	>75% Gras	s cover, Go	Good, HSG A			
	462	74	>75% Gras	s cover, Go	Good, HSG C			
	6,199	80	>75% Gras	s cover, Go	Good, HSG D			
	51,731	81	81 Weighted Average					
	22,753		43.98% Pervious Area					
	28,978		56.02% Impervious Area					
	Tc Length	n Slop	oe Velocity	Capacity	Description			
(m	in) (feet)	(ft/	ft) (ft/sec)	(cfs)				
6	5.0				Direct Entry,			

Subcatchment PRDA-1: TO BASIN D



Page 208

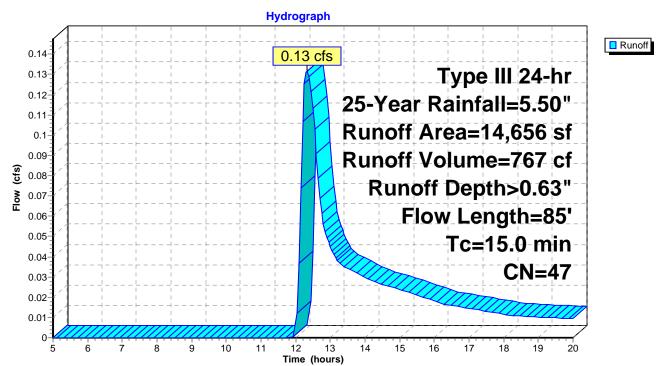
Summary for Subcatchment PRDA-10: TO ILSF

Runoff = 0.13 cfs @ 12.32 hrs, Volume= 767 cf, Depth> 0.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

_	Α	rea (sf)	CN I	CN Description				
		1,166	39 :	>75% Gras	s cover, Go	ood, HSG A		
		2,000	80 :	>75% Gras	s cover, Go	ood, HSG D		
8,622 30 Woods, Good, HSG A								
2,868 77 Woods, Good, HSG D								
14,656 47 Weighted Average				Neighted A	verage			
14,656 100.00% Pervious Area					a			
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	14.2	50	0.0140	0.06		Sheet Flow, AB		
						Woods: Light underbrush n= 0.400 P2= 3.20"		
	8.0	35	0.0220	0.74		Shallow Concentrated Flow, BC		
_						Woodland Kv= 5.0 fps		
	15.0	85	Total					

Subcatchment PRDA-10: TO ILSF



Page 209

Summary for Subcatchment PRDA-11: BASIN A DIRECT

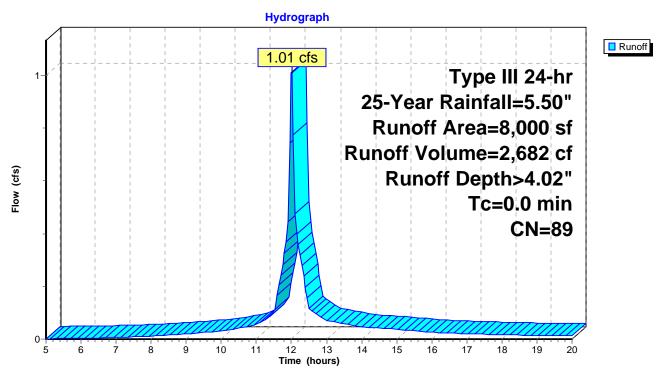
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 1.01 cfs @ 12.00 hrs, Volume= 2,682 cf, Depth> 4.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

Area (sf)	CN	Description
3,983	98	Water Surface, HSG B
4,017	80	>75% Grass cover, Good, HSG D
8,000	89	Weighted Average
4,017		50.21% Pervious Area
3,983		49.79% Impervious Area

Subcatchment PRDA-11: BASIN A DIRECT



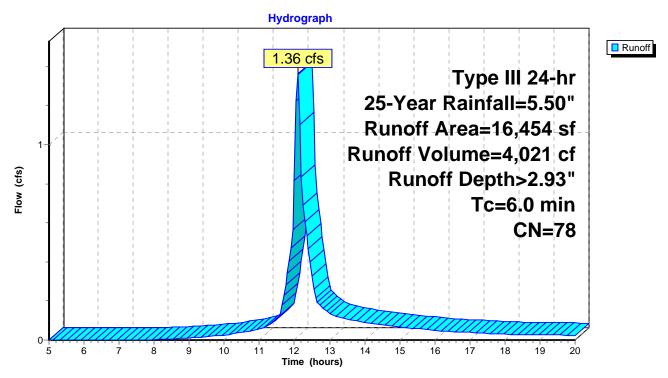
Summary for Subcatchment PRDA-12: BASIN B DIRECT

Runoff = 1.36 cfs @ 12.09 hrs, Volume= 4,021 cf, Depth> 2.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

Area (sf)	CN	Description				
7,423	98	Water Surface, HSG D				
5,050	80	>75% Grass cover, Good, HSG D				
3,981	39	>75% Grass cover, Good, HSG A				
16,454	78	Weighted Average				
9,031		54.89% Pervious Area				
7,423		45.11% Impervious Area				
Tc Length	Slop	pe Velocity Capacity Description				
(min) (feet)	(ft/	/ft) (ft/sec) (cfs)				
6.0		Direct Entry.				

Subcatchment PRDA-12: BASIN B DIRECT



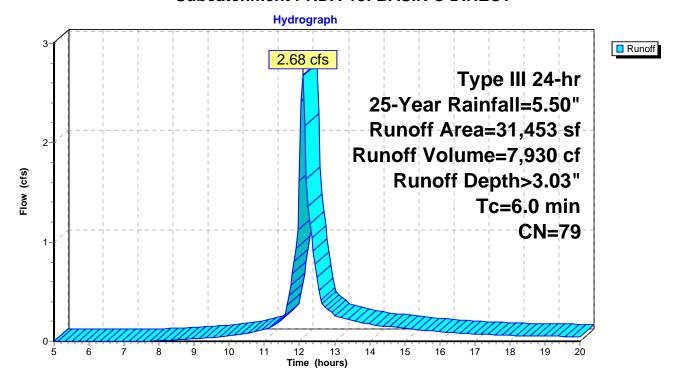
Summary for Subcatchment PRDA-13: BASIN C DIRECT

Runoff = 2.68 cfs @ 12.09 hrs, Volume= 7,930 cf, Depth> 3.03"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

Are	ea (sf)	CN	Description				
1:	5,825	61	>75% Grass	s cover, Go	ood, HSG B		
1	5,628	98	Water Surfa	ice, HSG B	В		
3	1,453	79	Weighted A				
1:	5,825		50.31% Pervious Area				
1:	5,628		49.69% Imp	rea			
Tc I (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	•		
6.0	,,	,	,,	()	Direct Entry,		

Subcatchment PRDA-13: BASIN C DIRECT



HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 212

Summary for Subcatchment PRDA-2: TO BASIN A

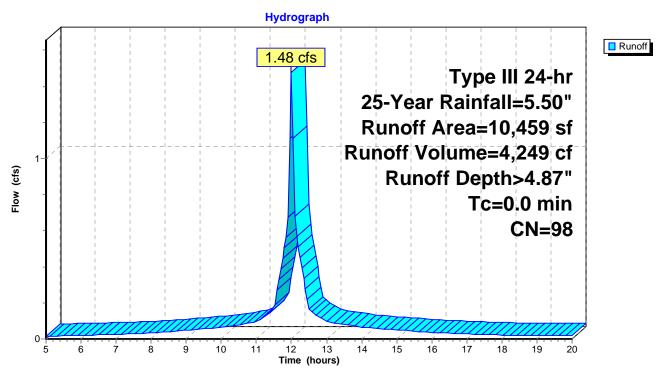
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 1.48 cfs @ 12.00 hrs, Volume= 4,249 cf, Depth> 4.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Area (sf)	CN	Description
*	10,459	98	IMPERVIOUS
	10,459		100.00% Impervious Area

Subcatchment PRDA-2: TO BASIN A



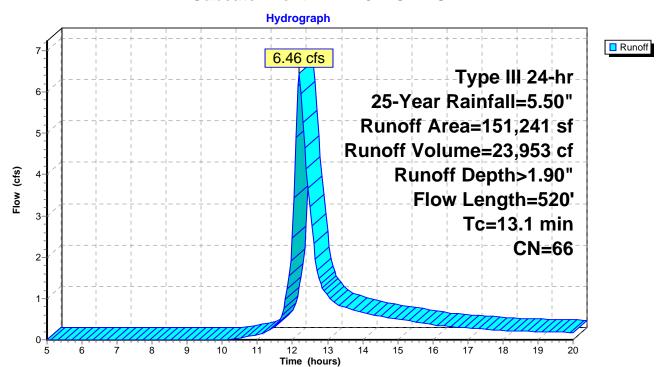
Summary for Subcatchment PRDA-3: TO BASIN B

Runoff = 6.46 cfs @ 12.19 hrs, Volume= 23,953 cf, Depth> 1.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Α	rea (sf)	CN D	escription							
*		40,248	98 II	MPERVIO	JS						
		14,787	98 F	Roofs, HSG A							
		77,313	39 >	75% Gras	s cover, Go	ood, HSG A					
		92	61 >	75% Gras	s cover, Go	ood, HSG B					
		18,801	80 >	75% Gras	s cover, Go	ood, HSG D					
	1	51,241	66 V	66 Weighted Average							
		96,206	6								
		55,035	3	6.39% Imp	ervious Ar	ea					
	_										
	Тс	Length	Slope	Velocity	Capacity	Description					
((min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	6.3	50	0.0380	0.13		Sheet Flow,					
						Grass: Dense n= 0.240 P2= 3.20"					
	6.5	406	0.0220	1.04		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	0.3	64	0.0360	3.85		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	13.1	520	Total								

Subcatchment PRDA-3: TO BASIN B



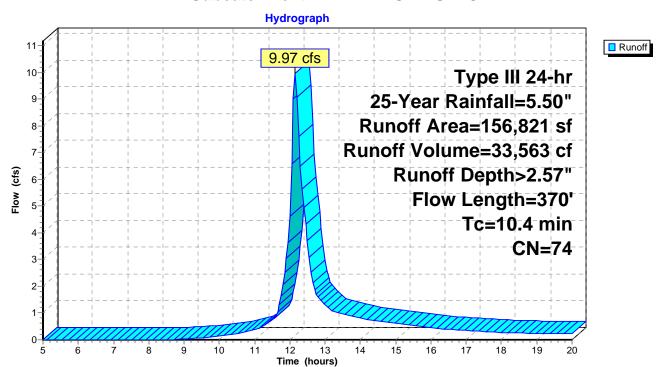
Summary for Subcatchment PRDA-4: TO BASIN C

Runoff = 9.97 cfs @ 12.15 hrs, Volume= 33,563 cf, Depth> 2.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Area (sf)	CN E	escription							
*	70,874	98 lı	98 Impervious							
	4,929	98 F	8 Roofs, HSG B							
	35,430	39 >	75% Gras	s cover, Go	ood, HSG A					
	44,834	61 >	75% Gras	s cover, Go	ood, HSG B					
	754	80 >	75% Gras	s cover, Go	ood, HSG D					
	156,821	74 V	74 Weighted Average							
	81,018	5	1.66% Per	vious Area	l					
	75,803	4	8.34% Imp	ervious Ar	ea					
To	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.8	50	0.0320	0.12		Sheet Flow,					
					Grass: Dense n= 0.240 P2= 3.20"					
3.1	217	0.0280	1.17		Shallow Concentrated Flow,					
					Short Grass Pasture Kv= 7.0 fps					
0.5	103	0.0270	3.34		Shallow Concentrated Flow,					
					Paved Kv= 20.3 fps					
10.4	370	Total								

Subcatchment PRDA-4: TO BASIN C



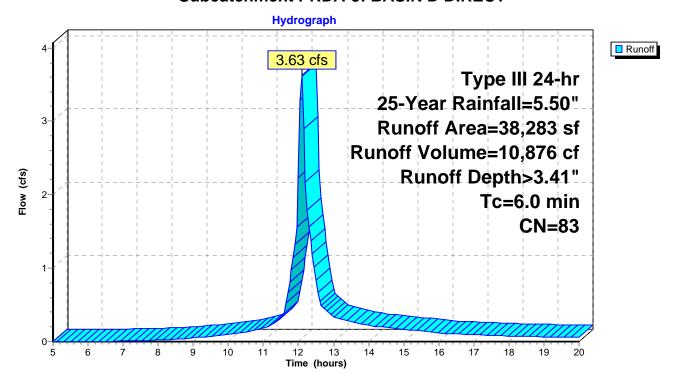
Summary for Subcatchment PRDA-5: BASIN D DIRECT

Runoff = 3.63 cfs @ 12.09 hrs, Volume= 10,876 cf, Depth> 3.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Area (sf)	CN	Description					
*	1,132	98	Impervious					
	7,697	80	1/2 acre lots	, 25% imp	o, HSG C			
	374	39	>75% Grass					
	7,237	74	>75% Grass	cover, Go	ood, HSG C			
	6,781	80	>75% Grass	cover, Go	ood, HSG D			
	9,227	98	Water Surfa	ce, HSG A	A			
	4,080	77	Woods, Goo	d, HSG D				
	1,323	70	Woods, Goo	d, HSG C				
	432	96	Gravel surfa	ce, HSG C	C			
	38,283	83	Weighted Av	verage				
	26,000		67.91% Per	vious Area	a			
	12,283		32.09% Impervious Area					
	To Longth	Slop	oe Velocity	Capacity	Description			
(r	Tc Length min) (feet)	Siop (ft/		(cfs)	·			
		(III)	11) (11/360)	(613)				
	6.0				Direct Entry.			

Subcatchment PRDA-5: BASIN D DIRECT



HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 216

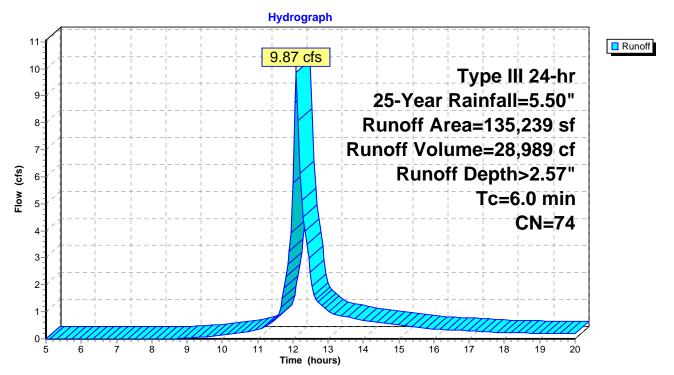
Summary for Subcatchment PRDA-6: TO BASIN E

Runoff = 9.87 cfs @ 12.09 hrs, Volume= 28,989 cf, Depth> 2.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	rea (sf)	CN	Description				
*	19,869	98	IMPERVIOU	JS			
	57,855	80	1/2 acre lots	s, 25% imp	o, HSG C		
	2,327	54	1/2 acre lots	s, 25% imp	o, HSG A		
	3,600	98	Roofs, HSG	S A			
	27,519	39	>75% Gras	s cover, Go	lood, HSG A		
	16,283	74	>75% Gras	s cover, Go	lood, HSG C		
	1,810	80	>75% Gras	s cover, Go	lood, HSG D		
	5,976	98	Water Surfa	ace, HSG D	D		
•	135,239	74	Weighted A	verage			
	90,749		67.10% Per	vious Area	a		
	44,491		32.90% Imp	ervious Ar	rea		
_		01		0 :	D		
Tc	Length	Slop	•	Capacity	·		
<u>(min)</u>	(feet)	(ft/f	t) (ft/sec)	(cfs)			
6.0					Direct Entry,		

Subcatchment PRDA-6: TO BASIN E



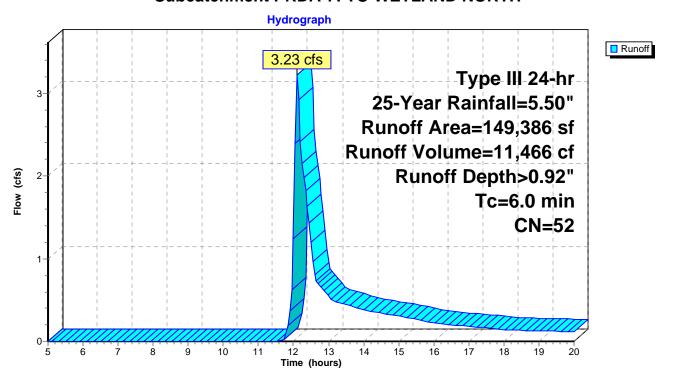
Summary for Subcatchment PRDA-7: TO WETLAND NORTH

Runoff = 3.23 cfs @ 12.11 hrs, Volume= 11,466 cf, Depth> 0.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Area (sf)	CN	Description					
	2,951	80	1/2 acre lots, 25% imp, HSG C					
	8,254	54	1/2 acre lots, 25% imp, HSG A					
	52,416	39	>75% Grass cover, Good, HSG A					
	27,465	61	>75% Grass cover, Good, HSG B					
	538	74	>75% Grass cover, Good, HSG C					
	9,934	80	>75% Grass cover, Good, HSG D					
	22,059	30	Woods, Good, HSG A					
	14,418	70	Woods, Good, HSG C					
	9,973	77	Woods, Good, HSG D					
*	1,378	98	impervious					
•	149,386	52	Weighted Average					
•	145,207		97.20% Pervious Area					
	4,179		2.80% Impervious Area					
Tc	Length	Slop	pe Velocity Capacity Description					
(min)	(feet)	(ft/1						
6.0			Direct Entry,	_				

Subcatchment PRDA-7: TO WETLAND NORTH



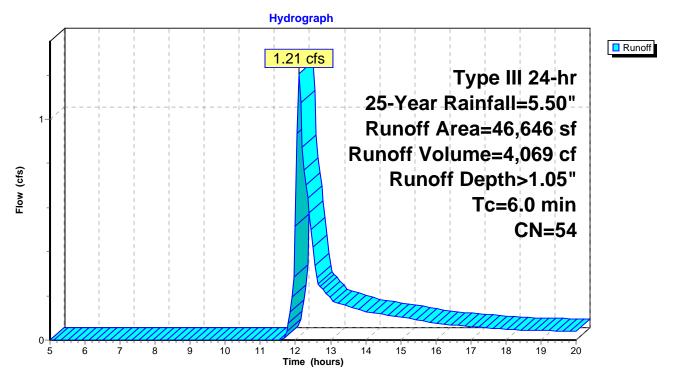
Summary for Subcatchment PRDA-8: TO WETLAND SOUTHEAST

Runoff = 1.21 cfs @ 12.11 hrs, Volume= 4,069 cf, Depth> 1.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

Ar	ea (sf)	CN	Description				
	21,559	39	>75% Grass	s cover, Go	ood, HSG A		
	7,434	80	>75% Grass	s cover, Go	ood, HSG D		
•	12,118	77	Woods, God	od, HSG D			
	5,535	30	Woods, God	Woods, Good, HSG A			
4	46,646	54	Weighted Average				
4	46,646		100.00% Pe	rvious Are	а		
_		٠.					
Tc	Length	Slop	,	Capacity	Description		
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)			
6.0					Direct Entry,		

Subcatchment PRDA-8: TO WETLAND SOUTHEAST



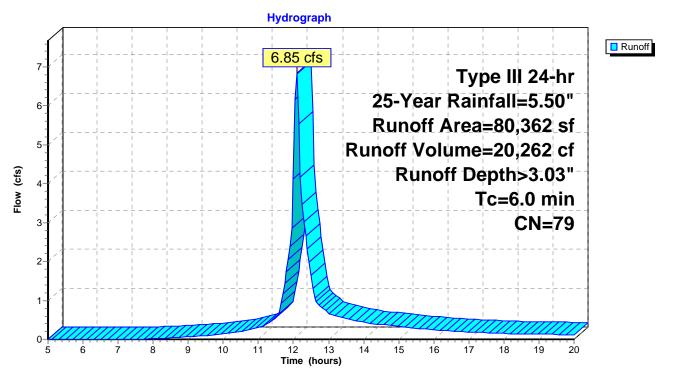
Summary for Subcatchment PRDA-9: TO WETLAND SOUTH

Runoff = 6.85 cfs @ 12.09 hrs, Volume= 20,262 cf, Depth> 3.03"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=5.50"

	Area (sf)	CN	Description					
	274	39	>75% Grass	s cover, Go	ood, HSG A			
	20,073	80	>75% Grass	s cover, Go	ood, HSG D			
	7,904	70	Woods, God	od, HSG C				
	34,217	77	Woods, God	od, HSG D				
	11,544	80	1/2 acre lots	s, 25% imp	, HSG C			
*	6,350	98	IMPERVIOL	IMPERVIOUS				
	80,362	79	Weighted A	Weighted Average				
	71,126		88.51% Per	vious Area				
	9,236		11.49% Imp	11.49% Impervious Area				
Т	c Length	Slop	e Velocity	Capacity	Description			
(mir	n) (feet)	(ft/f	t) (ft/sec)	(cfs)				
6.	0				Direct Entry,			

Subcatchment PRDA-9: TO WETLAND SOUTH



Summary for Reach 1R: ILSF

[40] Hint: Not Described (Outflow=Inflow)

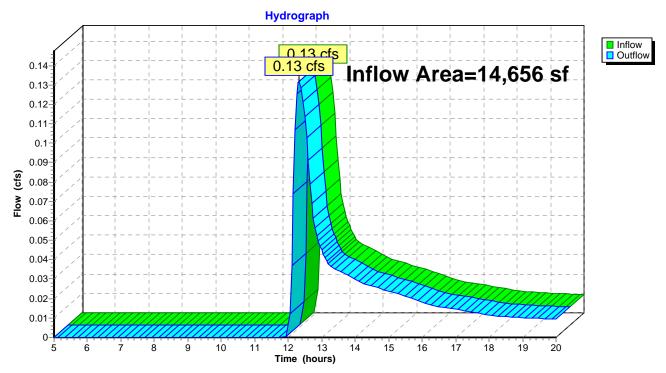
Inflow Area = 14,656 sf, 0.00% Impervious, Inflow Depth > 0.63" for 25-Year event

Inflow = 0.13 cfs @ 12.32 hrs, Volume= 767 cf

Outflow = 0.13 cfs @ 12.32 hrs, Volume= 767 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 1R: ILSF



HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 221

Summary for Reach 14R: WETLAND SOUTH

[40] Hint: Not Described (Outflow=Inflow)

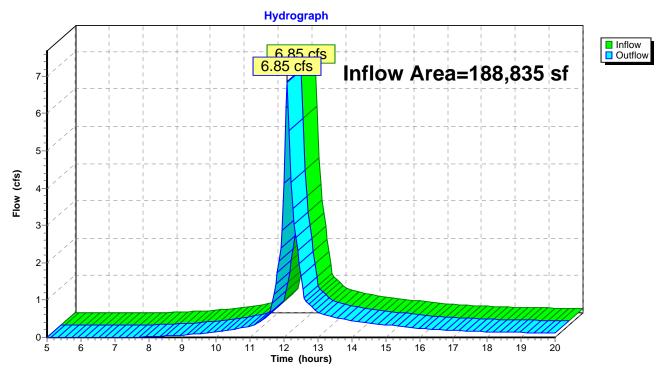
Inflow Area = 188,835 sf, 34.39% Impervious, Inflow Depth > 1.29" for 25-Year event

Inflow = 6.85 cfs @ 12.09 hrs, Volume= 20,262 cf

Outflow = 6.85 cfs @ 12.09 hrs, Volume= 20,262 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 14R: WETLAND SOUTH



Summary for Reach 15R: WETLAND SOUTHEAST

[40] Hint: Not Described (Outflow=Inflow)

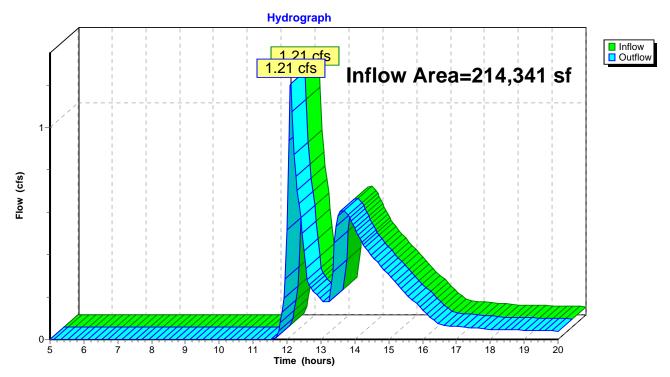
Inflow Area = 214,341 sf, 29.14% Impervious, Inflow Depth > 0.37" for 25-Year event

Inflow = 1.21 cfs @ 12.11 hrs, Volume= 6,608 cf

Outflow = 1.21 cfs @ 12.11 hrs, Volume= 6,608 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 15R: WETLAND SOUTHEAST



Summary for Reach 21R: WETLAND NORTH

[40] Hint: Not Described (Outflow=Inflow)

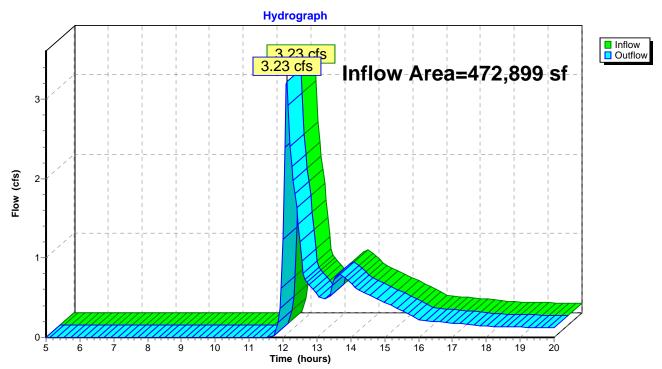
Inflow Area = 472,899 sf, 29.63% Impervious, Inflow Depth > 0.33" for 25-Year event

Inflow = 3.23 cfs @ 12.11 hrs, Volume= 13,169 cf

Outflow = 3.23 cfs @ 12.11 hrs, Volume= 13,169 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 21R: WETLAND NORTH



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 224

Summary for Pond 1P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 10,448 sf,100.00% Impervious, Inflow Depth > 4.87" for 25-Year event

Inflow = 1.26 cfs @ 12.09 hrs, Volume= 4,244 cf

Outflow = 0.10 cfs @ 13.05 hrs, Volume= 3,967 cf, Atten= 92%, Lag= 57.9 min

Discarded = 0.10 cfs @ 13.05 hrs, Volume= 3,967 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.62' @ 13.05 hrs Surf.Area= 1,531 sf Storage= 1,723 cf

Plug-Flow detention time= 137.2 min calculated for 3,953 cf (93% of inflow)

Center-of-Mass det. time= 112.5 min (847.1 - 734.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,317 cf	20.83'W x 73.50'L x 3.54'H Field A
			5,423 cf Overall - 2,131 cf Embedded = 3,292 cf x 40.0% Voids
#2A	0.50'	2,131 cf	Cultec R-330XLHD x 40 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

3,448 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.10 cfs @ 13.05 hrs HW=1.62' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.10 cfs)

Pond 1P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

10 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 71.50' Row Length +12.0" End Stone x 2 = 73.50' Base Length

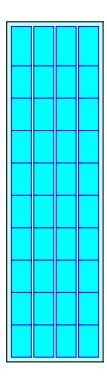
4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

40 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 2,131.0 cf Chamber Storage

5,423.2 cf Field - 2,131.0 cf Chambers = 3,292.2 cf Stone x 40.0% Voids = 1,316.9 cf Stone Storage

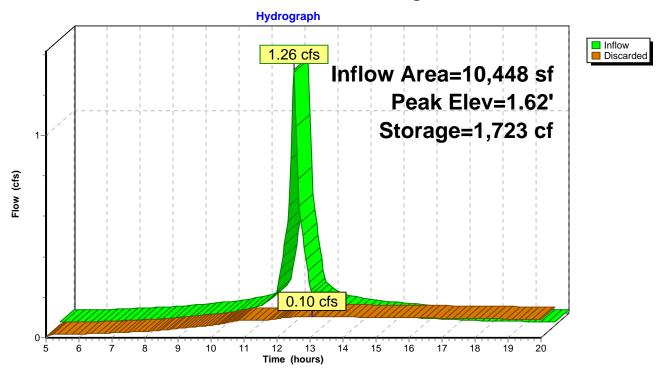
Chamber Storage + Stone Storage = 3,447.9 cf = 0.079 af Overall Storage Efficiency = 63.6% Overall System Size = 73.50' x 20.83' x 3.54'

40 Chambers 200.9 cy Field 121.9 cy Stone





Pond 1P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 227

Summary for Pond 2P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 13,061 sf,100.00% Impervious, Inflow Depth > 4.87" for 25-Year event

Inflow = 1.58 cfs @ 12.09 hrs, Volume= 5,305 cf

Outflow = 0.06 cfs @ 15.05 hrs, Volume= 2,828 cf, Atten= 96%, Lag= 178.0 min

Discarded = 0.06 cfs @ 15.05 hrs, Volume= 2,828 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.86' @ 15.05 hrs Surf.Area= 2,246 sf Storage= 2,977 cf

Plug-Flow detention time= 168.4 min calculated for 2,826 cf (53% of inflow)

Center-of-Mass det. time= 77.4 min (812.0 - 734.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,907 cf	25.67'W x 87.50'L x 3.54'H Field A
			7,954 cf Overall - 3,185 cf Embedded = 4,769 cf \times 40.0% Voids
#2A	0.50'	3,185 cf	Cultec R-330XLHD x 60 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 5 rows

5,093 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.06 cfs @ 15.05 hrs HW=1.86' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.06 cfs)

Pond 2P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 5 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

12 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 85.50' Row Length +12.0" End Stone x 2 = 87.50' Base Length

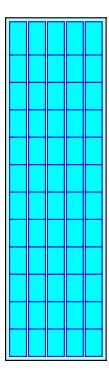
5 Rows x 52.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 25.67' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

60 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 5 Rows = 3,185.3 cf Chamber Storage

7,954.0 cf Field - 3,185.3 cf Chambers = 4,768.7 cf Stone x 40.0% Voids = 1,907.5 cf Stone Storage

Chamber Storage + Stone Storage = 5,092.8 cf = 0.117 af Overall Storage Efficiency = 64.0% Overall System Size = 87.50' x 25.67' x 3.54'

60 Chambers 294.6 cy Field 176.6 cy Stone

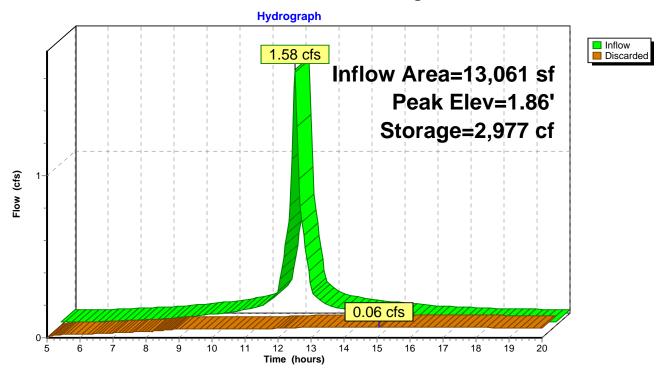




HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 229

Pond 2P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 230

Summary for Pond 3P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 13,061 sf,100.00% Impervious, Inflow Depth > 4.87" for 25-Year event

Inflow = 1.58 cfs @ 12.09 hrs, Volume= 5,305 cf

Outflow = 0.06 cfs @ 15.05 hrs, Volume= 2,828 cf, Atten= 96%, Lag= 178.0 min

Discarded = 0.06 cfs @ 15.05 hrs, Volume= 2,828 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.86' @ 15.05 hrs Surf.Area= 2,246 sf Storage= 2,977 cf

Plug-Flow detention time= 168.4 min calculated for 2,826 cf (53% of inflow)

Center-of-Mass det. time= 77.4 min (812.0 - 734.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,907 cf	25.67'W x 87.50'L x 3.54'H Field A
			7,954 cf Overall - 3,185 cf Embedded = 4,769 cf \times 40.0% Voids
#2A	0.50'	3,185 cf	Cultec R-330XLHD x 60 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 5 rows
•			

5,093 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.06 cfs @ 15.05 hrs HW=1.86' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.06 cfs)

Pond 3P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 5 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

12 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 85.50' Row Length +12.0" End Stone x 2 = 87.50' Base Length

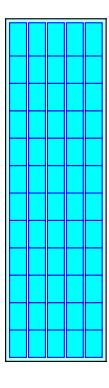
5 Rows x 52.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 25.67' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

60 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 5 Rows = 3,185.3 cf Chamber Storage

7,954.0 cf Field - 3,185.3 cf Chambers = 4,768.7 cf Stone x 40.0% Voids = 1,907.5 cf Stone Storage

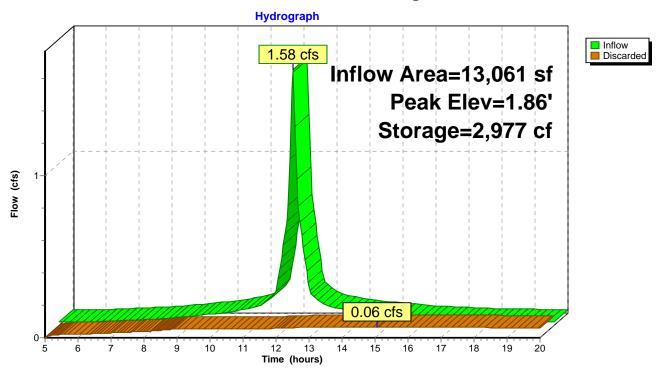
Chamber Storage + Stone Storage = 5,092.8 cf = 0.117 af Overall Storage Efficiency = 64.0% Overall System Size = 87.50' x 25.67' x 3.54'

60 Chambers 294.6 cy Field 176.6 cy Stone





Pond 3P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 233

Summary for Pond 4P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 7,296 sf,100.00% Impervious, Inflow Depth > 4.87" for 25-Year event

Inflow = 0.88 cfs @ 12.09 hrs, Volume= 2,963 cf

Outflow = 0.04 cfs @ 14.94 hrs, Volume= 1,611 cf, Atten= 96%, Lag= 170.9 min

Discarded = 0.04 cfs @ 14.94 hrs, Volume= 1,611 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.89' @ 14.94 hrs Surf.Area= 1,240 sf Storage= 1,652 cf

Plug-Flow detention time= 169.0 min calculated for 1,605 cf (54% of inflow)

Center-of-Mass det. time= 80.6 min (815.1 - 734.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,071 cf	20.83'W x 59.50'L x 3.54'H Field A
			4,390 cf Overall - 1,714 cf Embedded = 2,676 cf \times 40.0% Voids
#2A	0.50'	1,714 cf	Cultec R-330XLHD x 32 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

2,784 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.04 cfs @ 14.94 hrs HW=1.89' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.04 cfs)

Pond 4P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

8 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 57.50' Row Length +12.0" End Stone x 2 = 59.50' Base Length

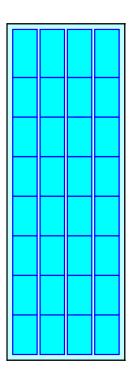
4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

32 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 1,713.7 cf Chamber Storage

4,390.2 cf Field - 1,713.7 cf Chambers = 2,676.5 cf Stone x 40.0% Voids = 1,070.6 cf Stone Storage

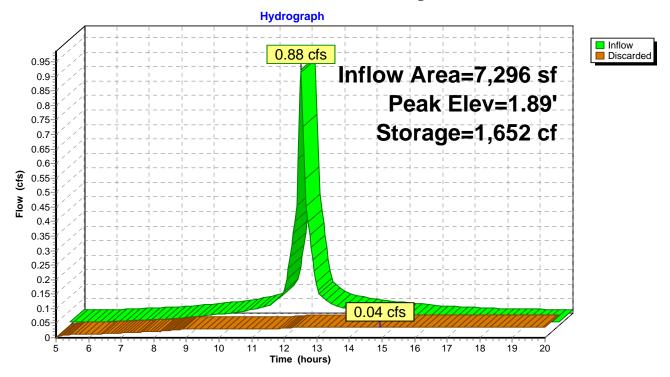
Chamber Storage + Stone Storage = 2,784.3 cf = 0.064 af Overall Storage Efficiency = 63.4% Overall System Size = 59.50' x 20.83' x 3.54'

32 Chambers 162.6 cy Field 99.1 cy Stone





Pond 4P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 236

Summary for Pond 5P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 4.87" for 25-Year event

Inflow = 0.14 cfs @ 12.09 hrs, Volume= 487 cf

Outflow = 0.01 cfs @ 13.89 hrs, Volume= 322 cf, Atten= 94%, Lag= 108.1 min

Discarded = 0.01 cfs @ 13.89 hrs, Volume= 322 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.96' @ 13.89 hrs Surf.Area= 200 sf Storage= 252 cf

Plug-Flow detention time= 174.6 min calculated for 320 cf (66% of inflow)

Center-of-Mass det. time= 101.9 min (836.5 - 734.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 13.89 hrs HW=1.96' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.01 cfs)

Pond 5P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

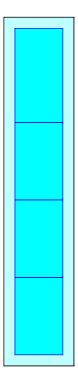
Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

- 4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length
- 1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height
- 4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

4 Chambers 26.2 cy Field 18.0 cy Stone

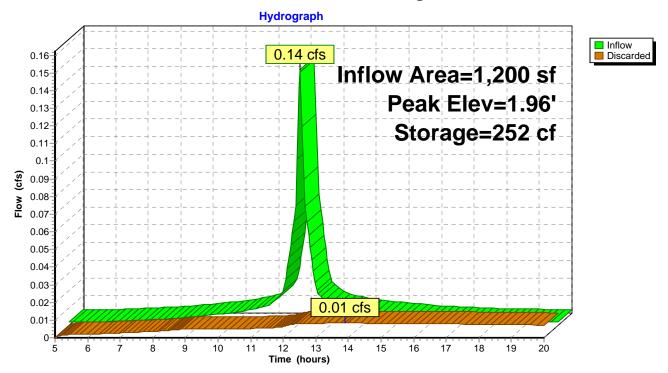




HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 238

Pond 5P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 239

Summary for Pond 6P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 4,059 sf,100.00% Impervious, Inflow Depth > 4.87" for 25-Year event

Inflow = 0.49 cfs @ 12.09 hrs, Volume= 1,649 cf

Outflow = 0.03 cfs @ 13.94 hrs, Volume= 1,133 cf, Atten= 94%, Lag= 111.1 min

Discarded = 0.03 cfs @ 13.94 hrs, Volume= 1,133 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.38' @ 13.94 hrs Surf.Area= 899 sf Storage= 811 cf

Plug-Flow detention time= 166.3 min calculated for 1,133 cf (69% of inflow)

Center-of-Mass det. time= 96.1 min (830.6 - 734.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	806 cf	11.17'W x 80.50'L x 3.54'H Field A
			$3,184 \text{ cf Overall - } 1,170 \text{ cf Embedded = } 2,014 \text{ cf } \times 40.0\% \text{ Voids}$
#2A	0.50'	1,170 cf	Cultec R-330XLHD x 22 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
	<u> </u>		

1,975 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.03 cfs @ 13.94 hrs HW=1.38' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.03 cfs)

Pond 6P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

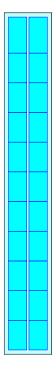
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

22 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,169.8 cf Chamber Storage

3,183.7 cf Field - 1,169.8 cf Chambers = 2,013.9 cf Stone x 40.0% Voids = 805.5 cf Stone Storage

Chamber Storage + Stone Storage = 1,975.3 cf = 0.045 af Overall Storage Efficiency = 62.0% Overall System Size = 80.50' x 11.17' x 3.54'

22 Chambers 117.9 cy Field 74.6 cy Stone

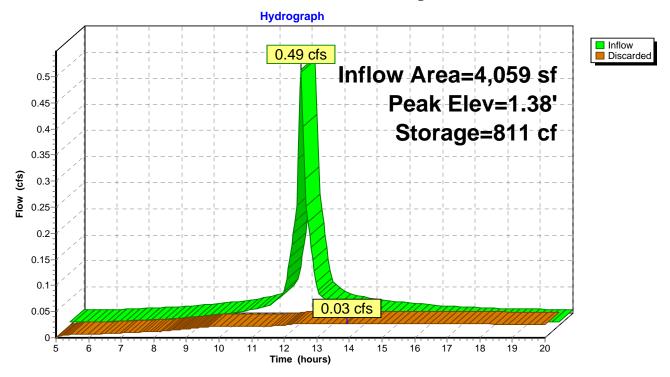




HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 241

Pond 6P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 242

Summary for Pond 7P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 4,929 sf,100.00% Impervious, Inflow Depth > 4.87" for 25-Year event

Inflow = 0.60 cfs @ 12.09 hrs, Volume= 2,002 cf

Outflow = 0.03 cfs @ 14.37 hrs, Volume= 1,216 cf, Atten= 95%, Lag= 136.7 min

Discarded = 0.03 cfs @ 14.37 hrs, Volume= 1,216 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.73' @ 14.37 hrs Surf.Area= 899 sf Storage= 1,059 cf

Plug-Flow detention time= 169.6 min calculated for 1,212 cf (61% of inflow)

Center-of-Mass det. time= 90.3 min (824.8 - 734.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	806 cf	11.17'W x 80.50'L x 3.54'H Field A
			$3,184 \text{ cf Overall - } 1,170 \text{ cf Embedded = } 2,014 \text{ cf } \times 40.0\% \text{ Voids}$
#2A	0.50'	1,170 cf	Cultec R-330XLHD x 22 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
	<u> </u>		

1,975 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.03 cfs @ 14.37 hrs HW=1.73' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.03 cfs)

Pond 7P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

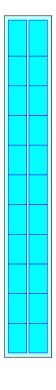
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

22 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,169.8 cf Chamber Storage

3,183.7 cf Field - 1,169.8 cf Chambers = 2,013.9 cf Stone x 40.0% Voids = 805.5 cf Stone Storage

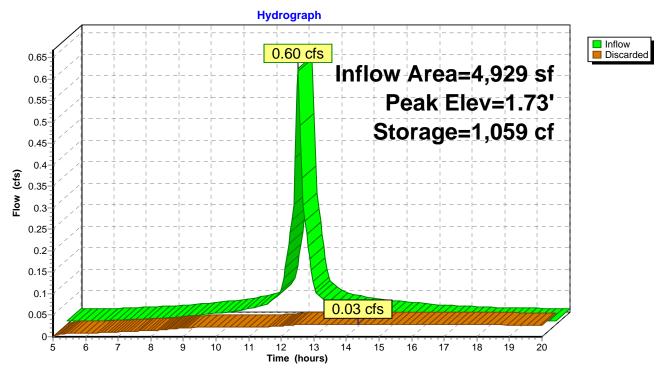
Chamber Storage + Stone Storage = 1,975.3 cf = 0.045 af Overall Storage Efficiency = 62.0% Overall System Size = 80.50' x 11.17' x 3.54'

22 Chambers 117.9 cy Field 74.6 cy Stone





Pond 7P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 245

Printed 3/9/2020

Summary for Pond 8P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 4.87" for 25-Year event

Inflow = 0.14 cfs @ 12.09 hrs, Volume= 487 cf

Outflow = 0.01 cfs @ 13.89 hrs, Volume= 322 cf, Atten= 94%, Lag= 108.1 min

Discarded = 0.01 cfs @ 13.89 hrs, Volume= 322 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.96' @ 13.89 hrs Surf.Area= 200 sf Storage= 252 cf

Plug-Flow detention time= 174.6 min calculated for 320 cf (66% of inflow)

Center-of-Mass det. time= 101.9 min (836.5 - 734.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows
•			

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 13.89 hrs HW=1.96' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.01 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 246

Pond 8P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

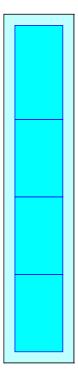
Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

- 4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length
- 1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height
- 4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

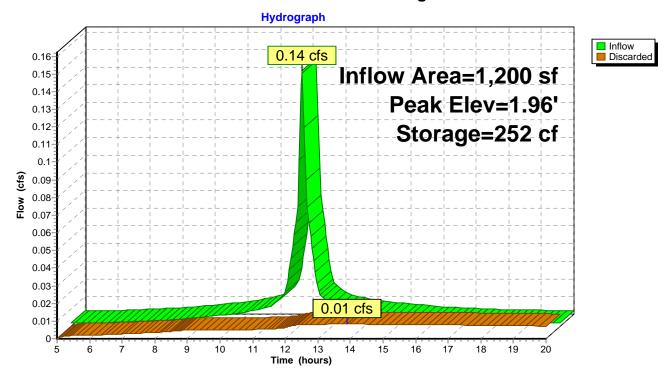
Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

4 Chambers 26.2 cy Field 18.0 cy Stone





Pond 8P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 248

Summary for Pond 9P: BASIN E

Inflow Area = 135,239 sf, 32.90% Impervious, Inflow Depth > 2.57" for 25-Year event Inflow = 9.87 cfs @ 12.09 hrs, Volume= 28,989 cf Outflow = 0.77 cfs @ 13.68 hrs, Volume= 14,643 cf, Atten= 92%, Lag= 95.0 min Discarded = 0.40 cfs @ 13.68 hrs, Volume= 12,939 cf Primary = 0.37 cfs @ 13.68 hrs, Volume= 1,703 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 240.93' @ 13.68 hrs Surf.Area= 7,013 sf Storage= 16,617 cf

Plug-Flow detention time= 202.3 min calculated for 14,643 cf (51% of inflow) Center-of-Mass det. time= 119.2 min (915.1 - 795.9)

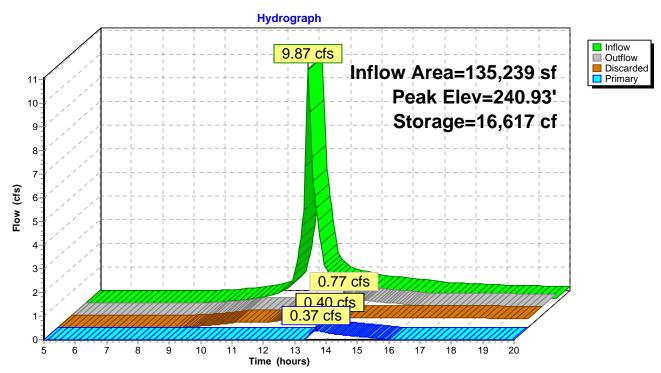
Volume	Invert	t Avai	I.Storage	Storage Description	on		
#1	237.80	'	24,841 cf	Custom Stage Da	ata (Irregular)Liste	ed below (Recalc)	
Elevatio		urf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
237.8	30	3,709	322.6	0	0	3,709	
238.0	00	3,904	326.4	761	761	3,916	
239.0	00	4,911	345.3	4,398	5,159	4,980	
240.0	00	5,976	364.3	5,435	10,594	6,110	
241.0	00	7,097	383.1	6,528	17,122	7,289	
241.9	90	10,147	430.2	7,719	24,841	10,360	
Device	Routing	In	vert Outle	et Devices			
#1	Discarded	237	.80' 2.41	0 in/hr Exfiltration	over Wetted area	a	
#2	Primary	ry 240.90' 25.0		25.0' long x 18.0' breadth Broad-Crested Rectangular Weir			
			Head	d (feet) 0.20 0.40	0.60 0.80 1.00	1.20 1.40 1.60	
			Coef	. (English) 2.68 2	.70 2.70 2.64 2.6	63 2.64 2.64 2.63	

Discarded OutFlow Max=0.40 cfs @ 13.68 hrs HW=240.93' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.40 cfs)

Primary OutFlow Max=0.32 cfs @ 13.68 hrs HW=240.93' (Free Discharge)

2=Broad-Crested Rectangular Weir (Weir Controls 0.32 cfs @ 0.45 fps)

Pond 9P: BASIN E



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 250

Summary for Pond 10P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 4.87" for 25-Year event

Inflow = 0.14 cfs @ 12.09 hrs, Volume= 487 cf

Outflow = 0.01 cfs @ 13.89 hrs, Volume= 322 cf, Atten= 94%, Lag= 108.1 min

Discarded = 0.01 cfs @ 13.89 hrs, Volume= 322 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.96' @ 13.89 hrs Surf.Area= 200 sf Storage= 252 cf

Plug-Flow detention time= 174.6 min calculated for 320 cf (66% of inflow)

Center-of-Mass det. time= 101.9 min (836.5 - 734.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 13.89 hrs HW=1.96' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.01 cfs)

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 251

Pond 10P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length

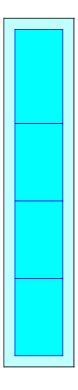
1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

4 Chambers 26.2 cy Field 18.0 cy Stone

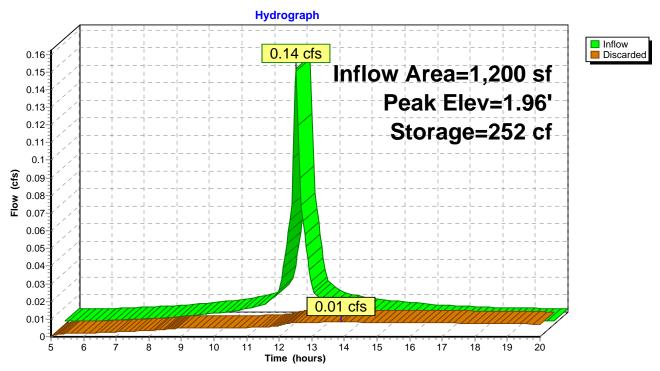




HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 252

Pond 10P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 253

Summary for Pond 11P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 4,059 sf,100.00% Impervious, Inflow Depth > 4.87" for 25-Year event

Inflow = 0.49 cfs @ 12.09 hrs, Volume= 1,649 cf

Outflow = 0.03 cfs @ 13.94 hrs, Volume= 1,133 cf, Atten= 94%, Lag= 111.1 min

Discarded = 0.03 cfs @ 13.94 hrs, Volume= 1,133 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.38' @ 13.94 hrs Surf.Area= 899 sf Storage= 811 cf

Plug-Flow detention time= 166.3 min calculated for 1,133 cf (69% of inflow)

Center-of-Mass det. time= 96.1 min (830.6 - 734.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	806 cf	11.17'W x 80.50'L x 3.54'H Field A
			$3,184 \text{ cf Overall - } 1,170 \text{ cf Embedded = } 2,014 \text{ cf } \times 40.0\% \text{ Voids}$
#2A	0.50'	1,170 cf	Cultec R-330XLHD x 22 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
	<u> </u>		—

1,975 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.03 cfs @ 13.94 hrs HW=1.38' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.03 cfs)

Pond 11P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

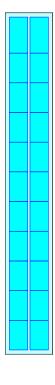
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

22 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,169.8 cf Chamber Storage

3,183.7 cf Field - 1,169.8 cf Chambers = 2,013.9 cf Stone x 40.0% Voids = 805.5 cf Stone Storage

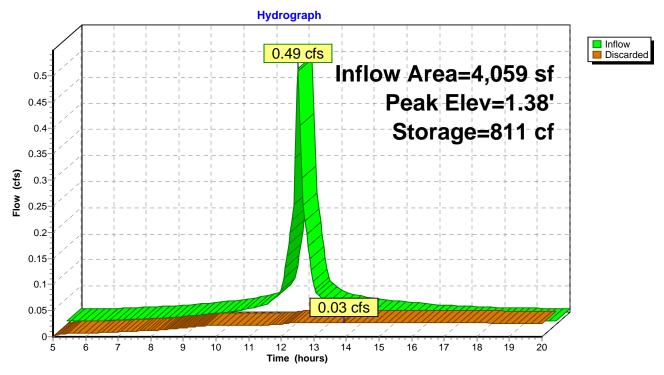
Chamber Storage + Stone Storage = 1,975.3 cf = 0.045 af Overall Storage Efficiency = 62.0% Overall System Size = 80.50' x 11.17' x 3.54'

22 Chambers 117.9 cy Field 74.6 cy Stone





Pond 11P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 256

Summary for Pond 13P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 4.87" for 25-Year event

Inflow = 0.14 cfs @ 12.09 hrs, Volume= 487 cf

Outflow = 0.01 cfs @ 13.89 hrs, Volume= 322 cf, Atten= 94%, Lag= 108.1 min

Discarded = 0.01 cfs @ 13.89 hrs, Volume= 322 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.96' @ 13.89 hrs Surf.Area= 200 sf Storage= 252 cf

Plug-Flow detention time= 174.6 min calculated for 320 cf (66% of inflow)

Center-of-Mass det. time= 101.9 min (836.5 - 734.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8 "W x 30.0 "H => 7.45 sf x 7.00 'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 13.89 hrs HW=1.96' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.01 cfs)

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 257

Pond 13P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length

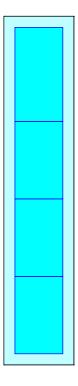
1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

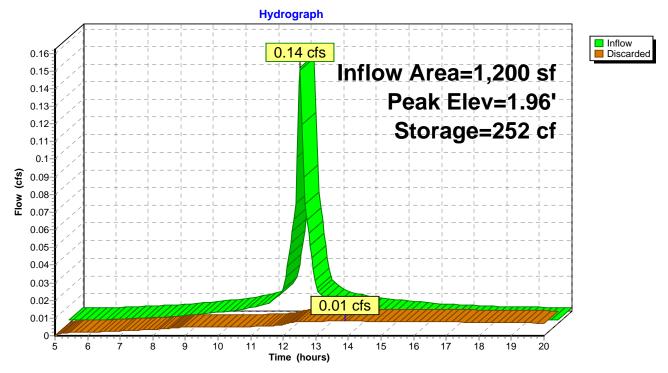
Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

4 Chambers 26.2 cy Field 18.0 cy Stone





Pond 13P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 259

Summary for Pond 14P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 4,059 sf,100.00% Impervious, Inflow Depth > 4.87" for 25-Year event

Inflow = 0.49 cfs @ 12.09 hrs, Volume= 1,649 cf

Outflow = 0.02 cfs @ 14.20 hrs, Volume= 1,053 cf, Atten= 95%, Lag= 126.8 min

Discarded = 0.02 cfs @ 14.20 hrs, Volume= 1,053 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.49' @ 14.20 hrs Surf.Area= 840 sf Storage= 843 cf

Plug-Flow detention time= 166.8 min calculated for 1,053 cf (64% of inflow)

Center-of-Mass det. time= 90.6 min (825.2 - 734.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	738 cf	16.00'W x 52.50'L x 3.54'H Field A
			2,975 cf Overall - 1,129 cf Embedded = 1,846 cf x 40.0% Voids
#2A	0.50'	1,129 cf	Cultec R-330XLHD x 21 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 3 rows
•		1 007 (T . I A . II I I O.

1,867 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.02 cfs @ 14.20 hrs HW=1.49' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.02 cfs)

Pond 14P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 3 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

7 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 50.50' Row Length +12.0" End Stone x 2 = 52.50' Base Length

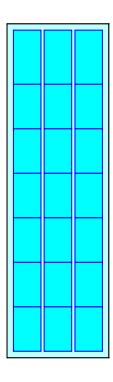
3 Rows x 52.0" Wide + 6.0" Spacing x 2 + 12.0" Side Stone x 2 = 16.00' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

21 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 3 Rows = 1,128.8 cf Chamber Storage

2,975.0 cf Field - 1,128.8 cf Chambers = 1,846.2 cf Stone x 40.0% Voids = 738.5 cf Stone Storage

Chamber Storage + Stone Storage = 1,867.3 cf = 0.043 af Overall Storage Efficiency = 62.8% Overall System Size = 52.50' x 16.00' x 3.54'

21 Chambers 110.2 cy Field 68.4 cy Stone

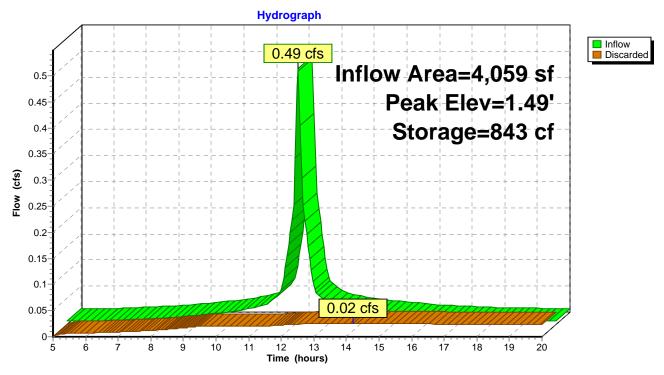




HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 261

Pond 14P: Roof Recharge



POST-DEVELOPMENT-REV1 Type III 24-hr 25-Year Rainfall=5.50"

Prepared by HP

Printed 3/9/2020

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 262

Summary for Pond 17P: Roof Recharge

Inflow Area = 13,086 sf, 37.67% Impervious, Inflow Depth > 3.81" for 25-Year event

Inflow = 1.36 cfs @ 12.09 hrs, Volume= 4,156 cf

Outflow = 0.05 cfs @ 15.59 hrs, Volume= 1,939 cf, Atten= 96%, Lag= 209.8 min

Discarded = 0.05 cfs @ 15.59 hrs, Volume= 1,939 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 2.12' @ 15.59 hrs Surf.Area= 1,677 sf Storage= 2,525 cf

Plug-Flow detention time= 185.8 min calculated for 1,939 cf (47% of inflow)

Center-of-Mass det. time= 97.7 min (864.4 - 766.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,440 cf	20.83'W x 80.50'L x 3.54'H Field A
			5,940 cf Overall - 2,340 cf Embedded = 3,600 cf x 40.0% Voids
#2A	0.50'	2,340 cf	Cultec R-330XLHD x 44 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

3,780 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.05 cfs @ 15.59 hrs HW=2.12' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.05 cfs)

Pond 17P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

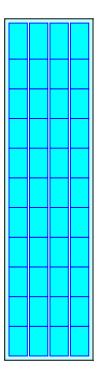
4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

44 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 2,339.6 cf Chamber Storage

5,939.7 cf Field - 2,339.6 cf Chambers = 3,600.1 cf Stone x 40.0% Voids = 1,440.0 cf Stone Storage

Chamber Storage + Stone Storage = 3,779.6 cf = 0.087 af Overall Storage Efficiency = 63.6% Overall System Size = 80.50' x 20.83' x 3.54'

44 Chambers 220.0 cy Field 133.3 cy Stone

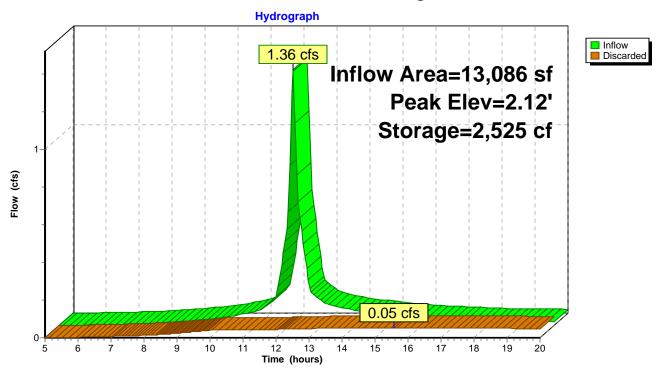




HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 264

Pond 17P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020

Page 265

Summary for Pond 19P: Roof Recharge

Inflow Area = 11,720 sf, 42.06% Impervious, Inflow Depth > 3.91" for 25-Year event

Inflow = 1.24 cfs @ 12.09 hrs, Volume= 3,823 cf

Outflow = 0.09 cfs @ 13.54 hrs, Volume= 3,076 cf, Atten= 93%, Lag= 86.8 min

Discarded = 0.09 cfs @ 13.54 hrs, Volume= 3,076 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 2.15' @ 13.54 hrs Surf.Area= 1,176 sf Storage= 1,777 cf

Plug-Flow detention time= 168.4 min calculated for 3,076 cf (80% of inflow)

Center-of-Mass det. time= 115.9 min (879.8 - 763.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,027 cf	16.00'W x 73.50'L x 3.54'H Field A
			$4,165 \text{ cf Overall - } 1,598 \text{ cf Embedded = } 2,567 \text{ cf } \times 40.0\% \text{ Voids}$
#2A	0.50'	1,598 cf	Cultec R-330XLHD x 30 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 3 rows
		0.005 (T

2,625 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.09 cfs @ 13.54 hrs HW=2.15' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.09 cfs)

Pond 19P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 3 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

10 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 71.50' Row Length +12.0" End Stone x 2 = 73.50' Base Length

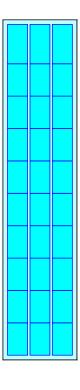
3 Rows x 52.0" Wide + 6.0" Spacing x 2 + 12.0" Side Stone x 2 = 16.00' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

30 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 3 Rows = 1,598.2 cf Chamber Storage

4,165.0 cf Field - 1,598.2 cf Chambers = 2,566.8 cf Stone x 40.0% Voids = 1,026.7 cf Stone Storage

Chamber Storage + Stone Storage = 2,624.9 cf = 0.060 af Overall Storage Efficiency = 63.0% Overall System Size = 73.50' x 16.00' x 3.54'

30 Chambers 154.3 cy Field 95.1 cy Stone

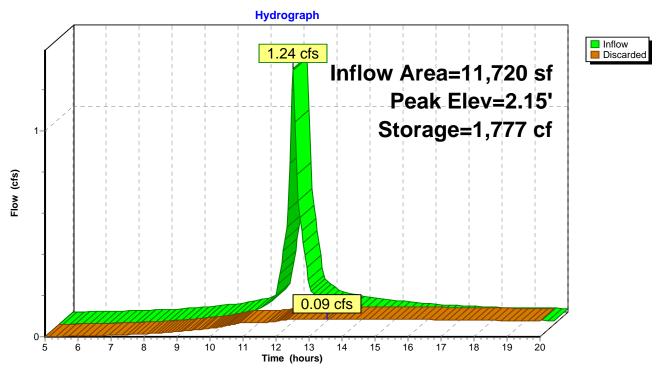




HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 267

Pond 19P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 268

Summary for Pond 21P: BASIN B

Inflow Area = 167,695 sf, 37.24% Impervious, Inflow Depth > 2.00" for 25-Year event Inflow 7.35 cfs @ 12.18 hrs. Volume= 27.974 cf 0.85 cfs @ 13.65 hrs, Volume= Outflow 14,768 cf, Atten= 88%, Lag= 88.1 min 0.39 cfs @ 13.65 hrs, Volume= Discarded = 12,228 cf Primary = 0.47 cfs @ 13.65 hrs, Volume= 2,539 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 227.93' @ 13.65 hrs Surf.Area= 7,356 sf Storage= 15,017 cf

Plug-Flow detention time= 192.5 min calculated for 14,719 cf (53% of inflow)

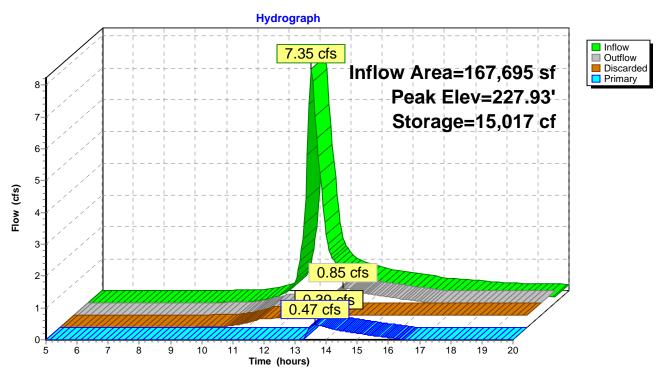
Center-of-Mass det. time= 108.5 min (921.1 - 812.5)

Volume	Inve	rt Avai	I.Storage	Storage Description	on		
#1	225.50)'	23,381 cf	Custom Stage Da	ata (Irregular) List	ed below (Recalc)	
Elevatio		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
225.5	50	5,039	294.3	0	0	5,039	
226.0	00	5,488	303.7	2,631	2,631	5,511	
227.0	00	6,427	322.5	5,951	8,582	6,498	
228.0	00	7,423	328.3	6,919	15,501	6,941	
228.9	00	10,160	388.5	7,880	23,381	10,390	
Device #1 #2	Routing Discarded Primary		.50' 2.41 .90' 25.0	et Devices 0 in/hr Exfiltration ' long x 21.0' brea d (feet) 0.20 0.40	dth Broad-Crest	ed Rectangular Weir	
			Coef	f. (English) 2.68 2	.70 2.70 2.64 2.	63 2.64 2.64 2.63	

Discarded OutFlow Max=0.39 cfs @ 13.65 hrs HW=227.93' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.39 cfs)

Primary OutFlow Max=0.43 cfs @ 13.65 hrs HW=227.93' (Free Discharge) -2=Broad-Crested Rectangular Weir (Weir Controls 0.43 cfs @ 0.50 fps)

Pond 21P: BASIN B



Type III 24-hr 25-Year Rainfall=5.50"

POST-DEVELOPMENT-REV1

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020

Page 270

Summary for Pond 22P: Roof Recharge

Inflow Area = 11,373 sf, 43.34% Impervious, Inflow Depth > 2.93" for 25-Year event

Inflow = 0.94 cfs @ 12.09 hrs, Volume= 2,779 cf

Outflow = 0.04 cfs @ 15.62 hrs, Volume= 1,314 cf, Atten= 96%, Lag= 211.7 min

Discarded = 0.04 cfs @ 15.62 hrs, Volume= 1,314 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 2.16' @ 15.62 hrs Surf.Area= 1,133 sf Storage= 1,692 cf

Plug-Flow detention time= 200.9 min calculated for 1,309 cf (47% of inflow)

Center-of-Mass det. time= 117.5 min (905.4 - 787.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,013 cf	11.17'W x 101.50'L x 3.54'H Field A
			4,014 cf Overall - 1,483 cf Embedded = 2,531 cf x 40.0% Voids
#2A	0.50'	1,483 cf	Cultec R-330XLHD x 28 Inside #1
			Effective Size= 47.8 "W x 30.0 "H => 7.45 sf x 7.00 'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
		0.405 -(Total A silable Otanana

2,495 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.04 cfs @ 15.62 hrs HW=2.16' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.04 cfs)

Pond 22P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

14 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 99.50' Row Length +12.0" End Stone x 2 = 101.50' Base Length

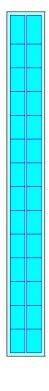
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

28 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,482.7 cf Chamber Storage

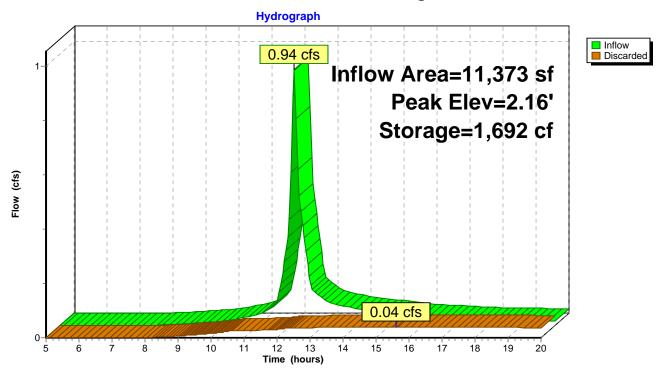
4,014.2 cf Field - 1,482.7 cf Chambers = 2,531.4 cf Stone x 40.0% Voids = 1,012.6 cf Stone Storage

Chamber Storage + Stone Storage = 2,495.3 cf = 0.057 af Overall Storage Efficiency = 62.2% Overall System Size = 101.50' x 11.17' x 3.54'

28 Chambers 148.7 cy Field 93.8 cy Stone



Pond 22P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 273

Summary for Pond 30P: BASIN A

[82] Warning: Early inflow requires earlier time span

Inflow Area = 18,459 sf, 78.24% Impervious, Inflow Depth > 4.51" for 25-Year event Inflow = 2.49 cfs @ 12.00 hrs, Volume= 6,931 cf
Outflow = 0.21 cfs @ 12.82 hrs, Volume= 6,497 cf, Atten= 92%, Lag= 49.0 min Discarded = 0.21 cfs @ 12.82 hrs, Volume= 6,497 cf
Primary = 0.00 cfs @ 5.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 239.69' @ 12.82 hrs Surf.Area= 2,703 sf Storage= 3,039 cf

Plug-Flow detention time= 141.6 min calculated for 6,496 cf (94% of inflow)

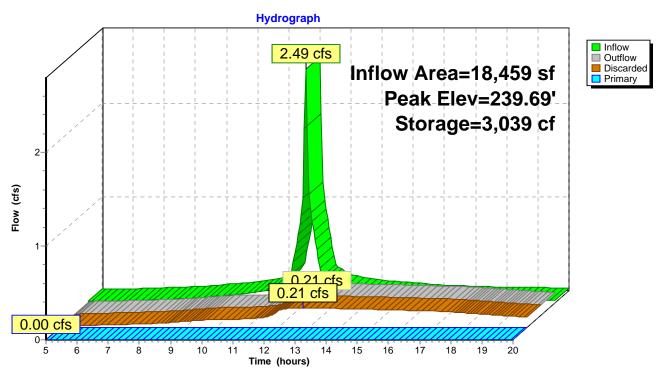
Center-of-Mass det. time= 117.6 min (857.9 - 740.3)

Volume	Invert	Avail.	Storage	Storage Descriptio	n		
#1	238.20'		7,401 cf	Custom Stage Da	ı ta (Irregular) Listo	ed below (Recalc)	
Elevatio (fee		rf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
238.2 239.0 240.0 241.0	0	1,411 2,083 3,005 3,983	262.1 297.8 316.6 335.5	0 1,389 2,530 3,483	0 1,389 3,919 7,401	1,411 3,017 3,986 5,020	
Device	Routing	Inv	ert Outle	et Devices			
#1 #2	Discarded Primary	238.2 240.0	00' 25.0 ' Head	0 in/hr Exfiltration l long x 21.0' bread d (feet) 0.20 0.40 d (English) 2.68 2.	dth Broad-Cresto 0.60 0.80 1.00	ed Rectangular Weir 1.20 1.40 1.60	

Discarded OutFlow Max=0.21 cfs @ 12.82 hrs HW=239.69' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.21 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=238.20' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 30P: BASIN A



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 275

Summary for Pond 31P: BASIN D

Inflow Area = 90,014 sf, 45.84% Impervious, Inflow Depth > 3.30" for 25-Year event
Inflow = 8.29 cfs @ 12.09 hrs, Volume= 24,735 cf
Outflow = 0.72 cfs @ 13.12 hrs, Volume= 21,915 cf, Atten= 91%, Lag= 61.8 min
Discarded = 0.72 cfs @ 13.12 hrs, Volume= 21,915 cf
Primary = 0.00 cfs @ 5.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 240.61' @ 13.12 hrs Surf.Area= 8,732 sf Storage= 11,557 cf

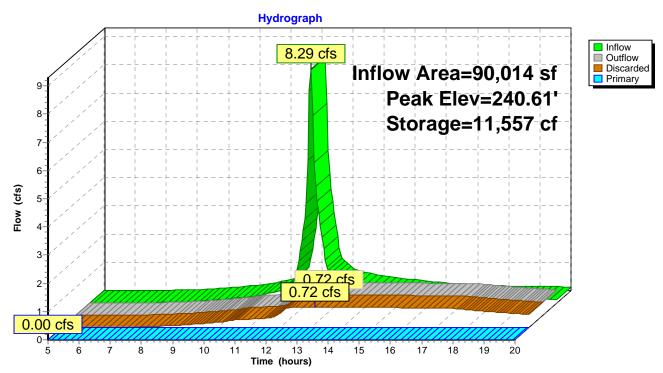
Plug-Flow detention time= 162.3 min calculated for 21,915 cf (89% of inflow) Center-of-Mass det. time= 126.1 min (905.6 - 779.5)

Volume	Inver	t Avail.	Storage	Storage Description	on		
#1	239.00)' 24	4,822 cf	Custom Stage Da	ata (Irregular) List	ed below (Recalc)	
Elevatio (fee	_	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
239.0	0	5,028	276.6	0	0	5,028	
240.0	0	7,979	406.4	6,447	6,447	12,091	
241.0	0	9,227	425.3	8,595	15,042	13,409	
241.9	0	12,593	472.4	9,780	24,822	16,798	
Device	Routing	Inve	ert Outle	et Devices			
#1	Discarded	239.0	0' 2.41	0 in/hr Exfiltration	over Wetted are	a	
#2	Primary	240.9	Head	d (feet) 0.20 0.40	0.60 0.80 1.00	ed Rectangular Wei 1.20 1.40 1.60 63 2.64 2.64 2.63	

Discarded OutFlow Max=0.72 cfs @ 13.12 hrs HW=240.61' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.72 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=239.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 31P: BASIN D



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 277

Summary for Pond 32P: BASIN C

Inflow Area = 188,274 sf, 48.56% Impervious, Inflow Depth > 2.64" for 25-Year event

Inflow = 12.19 cfs @ 12.14 hrs, Volume= 41,493 cf

Outflow = 1.55 cfs @ 13.01 hrs, Volume= 37,848 cf, Atten= 87%, Lag= 52.1 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 225.78' @ 13.01 hrs Surf.Area= 14,878 sf Storage= 18,870 cf

Plug-Flow detention time= 144.8 min calculated for 37,848 cf (91% of inflow)

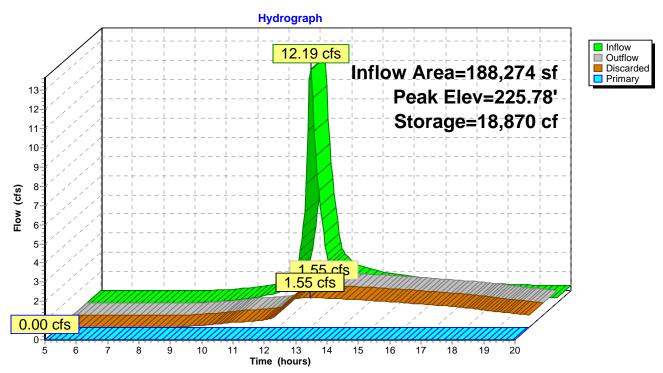
Center-of-Mass det. time= 115.7 min (912.5 - 796.8)

Volume	Inver	t Avail.	Storage	Storage Description				
#1	224.00	' 38	8,476 cf	Custom Stage Da	ata (Irregular)Liste	ed below (Recalc)		
Elevatio		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
224.0	00	4,753	268.4	0	0	4,753		
225.0 226.0		12,400 15,628	506.3 628.5	8,277 13,983	8,277 22,260	19,424 30,474		
226.9	90	20,519	675.7	16,216	38,476	35,408		
Device	Routing	Inve	ert Outle	et Devices				
#1	Discarded	224.0	00' 2.41	0 in/hr Exfiltration	over Wetted are	a		
#2	Primary	225.9	Head	25.0' long x 21.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63				

Discarded OutFlow Max=1.55 cfs @ 13.01 hrs HW=225.78' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 1.55 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=224.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 32P: BASIN C



Subcatchment PRDA-1: TO BASIN D

Prepared by HP

Type III 24-hr 100-Year Rainfall=6.70" Printed 3/9/2020

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 279

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Reach routing by Stor-ind	Thethod - I ond rodding by Stor-Ind Method
Subcatchment 34S: 10 Unit Roof	Runoff Area=10,448 sf 100.00% Impervious Runoff Depth>5.97" Tc=6.0 min CN=98 Runoff=1.54 cfs 5,194 cf
Subcatchment 35S: 14 Unit Roof	Runoff Area=13,061 sf 100.00% Impervious Runoff Depth>5.97" Tc=6.0 min CN=98 Runoff=1.92 cfs 6,493 cf
Subcatchment 36S: 14 Unit Roof	Runoff Area=13,061 sf 100.00% Impervious Runoff Depth>5.97" Tc=6.0 min CN=98 Runoff=1.92 cfs 6,493 cf
Subcatchment 37S: 7 Unit Roof	Runoff Area=7,296 sf 100.00% Impervious Runoff Depth>5.97" Tc=6.0 min CN=98 Runoff=1.07 cfs 3,627 cf
Subcatchment LOT 1: Single Family Hous	Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>5.97" Tc=6.0 min CN=98 Runoff=0.18 cfs 597 cf
Subcatchment LOT 12: DUPLEX + YARD	Runoff Area=11,373 sf 43.34% Impervious Runoff Depth>3.94" Tc=6.0 min CN=78 Runoff=1.26 cfs 3,734 cf
Subcatchment LOT 13: DUPLEX + YARD	Runoff Area=11,720 sf 42.06% Impervious Runoff Depth>5.01" Tc=6.0 min CN=88 Runoff=1.57 cfs 4,894 cf
Subcatchment LOT 14: DUPLEX + YARD	Runoff Area=13,086 sf 37.67% Impervious Runoff Depth>4.90" Tc=6.0 min CN=87 Runoff=1.72 cfs 5,347 cf
Subcatchment LOT 18: DUPLEX ROOF	Runoff Area=4,929 sf 100.00% Impervious Runoff Depth>5.97" Tc=6.0 min CN=98 Runoff=0.73 cfs 2,450 cf
Subcatchment LOT 19: DUPLEX ROOF	Runoff Area=4,059 sf 100.00% Impervious Runoff Depth>5.97" Tc=6.0 min CN=98 Runoff=0.60 cfs 2,018 cf
Subcatchment LOT 2: Single Family Hous	Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>5.97" Tc=6.0 min CN=98 Runoff=0.18 cfs 597 cf
Subcatchment LOT 20: DUPLEX ROOF	Runoff Area=4,059 sf 100.00% Impervious Runoff Depth>5.97" Tc=6.0 min CN=98 Runoff=0.60 cfs 2,018 cf
Subcatchment LOT 21: DUPLEX ROOF	Runoff Area=4,059 sf 100.00% Impervious Runoff Depth>5.97" Tc=6.0 min CN=98 Runoff=0.60 cfs 2,018 cf
Subcatchment LOT 3: Single Family House	Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>5.97" Tc=6.0 min CN=98 Runoff=0.18 cfs 597 cf
Subcatchment LOT 8: Single Family Hous	Se Runoff Area=1,200 sf 100.00% Impervious Runoff Depth>5.97" Tc=6.0 min CN=98 Runoff=0.18 cfs 597 cf

Runoff Area=51,731 sf 56.02% Impervious Runoff Depth>4.26"

Tc=6.0 min CN=81 Runoff=6.10 cfs 18,345 cf

	_					
D	ЛСТ	DEI			$I \subseteq N \mid T$	-RFV1
	יוכגנו	-I /C \	, – 1	T J P N	/I	-R C V I

Type III 24-hr 100-Year Rainfall=6.70"

Prepared by HP	Printed 3/9/2020
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC	Page 280

Subcatchment PRDA-10: TO ILSF

Runoff Area=14,656 sf 0.00% Impervious Runoff Depth>1.11"
Flow Length=85' Tc=15.0 min CN=47 Runoff=0.29 cfs 1,357 cf

Subcatchment PRDA-11: BASIN A DIRECT Runoff Area=8,000 sf 49.79% Impervious Runoff Depth>5.12"
Tc=0.0 min CN=89 Runoff=1.27 cfs 3,417 cf

Subcatchment PRDA-12: BASIN B DIRECT Runoff Area=16,454 sf 45.11% Impervious Runoff Depth>3.94"

Tc=6.0 min CN=78 Runoff=1.82 cfs 5,402 cf

Subcatchment PRDA-13: BASIN C DIRECT Runoff Area=31,453 sf 49.69% Impervious Runoff Depth>4.04" Tc=6.0 min CN=79 Runoff=3.55 cfs 10,600 cf

Subcatchment PRDA-2: TO BASIN A Runoff Area=10,459 sf 100.00% Impervious Runoff Depth>5.97" Tc=0.0 min CN=98 Runoff=1.81 cfs 5,200 cf

Subcatchment PRDA-3: TO BASIN B Runoff Area=151,241 sf 36.39% Impervious Runoff Depth>2.74" Flow Length=520' Tc=13.1 min CN=66 Runoff=9.42 cfs 34,504 cf

Subcatchment PRDA-4: TO BASIN C

Runoff Area=156,821 sf 48.34% Impervious Runoff Depth>3.52"

Flow Length=370' Tc=10.4 min CN=74 Runoff=13.64 cfs 46,053 cf

Subcatchment PRDA-5: BASIN D DIRECT Runoff Area=38,283 sf 32.09% Impervious Runoff Depth>4.47"

Tc=6.0 min CN=83 Runoff=4.70 cfs 14,259 cf

Subcatchment PRDA-6: TO BASIN E Runoff Area=135,239 sf 32.90% Impervious Runoff Depth>3.53" Tc=6.0 min CN=74 Runoff=13.49 cfs 39,773 cf

Subcatchment PRDA-7: TO WETLAND Runoff Area=149,386 sf 2.80% Impervious Runoff Depth>1.51" Tc=6.0 min CN=52 Runoff=5.87 cfs 18,769 cf

Subcatchment PRDA-8: TO WETLAND Runoff Area=46,646 sf 0.00% Impervious Runoff Depth>1.67" Tc=6.0 min CN=54 Runoff=2.08 cfs 6,501 cf

Subcatchment PRDA-9: TO WETLAND Runoff Area=80,362 sf 11.49% Impervious Runoff Depth>4.04" Tc=6.0 min CN=79 Runoff=9.08 cfs 27,084 cf

Reach 1R: ILSFInflow=0.29 cfs 1,357 cf

Outflow=0.29 cfs 1,357 cf

Reach 14R: WETLAND SOUTH Inflow=9.08 cfs 29,537 cf Outflow=9.08 cfs 29,537 cf

Reach 15R: WETLAND SOUTHEAST Inflow=5.20 cfs 19,298 cf Outflow=5.20 cfs 19,298 cf

Reach 21R: WETLAND NORTHInflow=10.29 cfs 36,353 cf
Outflow=10.29 cfs 36,353 cf

Pond 1P: Roof Recharge Peak Elev=2.09' Storage=2,271 cf Inflow=1.54 cfs 5,194 cf

Outflow=0.11 cfs 4,290 cf

Pond 2P: Roof Recharge

Peak Elev=2.43' Storage=3,907 cf Inflow=1.92 cfs 6,493 cf
Outflow=0.07 cfs 3,014 cf

Type III 24-hr 100-Year Rainfall=6.70"

POST-DEVELOPMENT-REVI	Type III 24-III 100-Year Rainial=0.70
Prepared by HP	Printed 3/9/2020
HydroCAD® 10.00-24 s/n 09450 © 2018 Hyd	roCAD Software Solutions LLC Page 281
Pond 3P: Roof Recharge	Peak Elev=2.43' Storage=3,907 cf Inflow=1.92 cfs 6,493 cf
	Outflow=0.07 cfs 3,014 cf
	D -1 Fl- 0 47 00 0 404 (1 fl- 4 07 (0 007 (
Pond 4P: Roof Recharge	Peak Elev=2.47' Storage=2,164 cf Inflow=1.07 cfs 3,627 cf
	Outflow=0.04 cfs 1,729 cf
Dand FD: Doof Dooborns	Dook Flove 2.541 Storage 226 of Inflowed 19 of 507 of
Pond 5P: Roof Recharge	Peak Elev=2.54' Storage=326 cf Inflow=0.18 cfs 597 cf Outflow=0.01 cfs 360 cf
	Outilow=0.01 cls 300 cl
Pond 6P: Roof Recharge	Peak Elev=1.75' Storage=1,068 cf Inflow=0.60 cfs 2,018 cf
i ond or . Noor Necharge	Outflow=0.03 cfs 1,223 cf
	Outhow=0.00 010 1,220 01
Pond 7P: Roof Recharge	Peak Elev=2.23' Storage=1,386 cf Inflow=0.73 cfs 2,450 cf
	Outflow=0.03 cfs 1,321 cf
Pond 8P: Roof Recharge	Peak Elev=2.54' Storage=326 cf Inflow=0.18 cfs 597 cf
_	Outflow=0.01 cfs 360 cf
Pond 9P: BASIN E	Peak Elev=241.06' Storage=17,545 cf Inflow=13.49 cfs 39,773 cf
Discarded=0.42 cfs	13,774 cf Primary=4.27 cfs 10,769 cf Outflow=4.68 cfs 24,543 cf
	D. J. El 0.541 00
Pond 10P: Roof Recharge	Peak Elev=2.54' Storage=326 cf Inflow=0.18 cfs 597 cf
	Outflow=0.01 cfs 360 cf
Pond 11P: Roof Recharge	Peak Elev=1.75' Storage=1,068 cf Inflow=0.60 cfs 2,018 cf
Folia TTP. Roof Recliarge	Outflow=0.03 cfs 1,223 cf
	Outhor 1,220 of
Pond 13P: Roof Recharge	Peak Elev=2.54' Storage=326 cf Inflow=0.18 cfs 597 cf
	Outflow=0.01 cfs 360 cf
Pond 14P: Roof Recharge	Peak Elev=1.90' Storage=1,110 cf Inflow=0.60 cfs 2,018 cf
	Outflow=0.03 cfs 1,131 cf
Pond 17P: Roof Recharge	Peak Elev=3.05' Storage=3,450 cf Inflow=1.72 cfs 5,347 cf
	Outflow=0.05 cfs 2,164 cf
Dand 40D: Doof Dook area	Deal, Floy 2 421 Storage 2 420 of Juffers 4 57 of 4 204 of
Pond 19P: Roof Recharge	Peak Elev=3.13' Storage=2,430 cf Inflow=1.57 cfs 4,894 cf Outflow=0.10 cfs 3,483 cf
	Outilow=0.10 cis 3,463 ci
Pond 21P: BASIN B	Peak Elev=228.07' Storage=16,003 cf Inflow=10.67 cfs 39,906 cf
	13,015 cf Primary=4.58 cfs 12,797 cf Outflow=4.98 cfs 25,812 cf
D10001000-01-0 010	. 5,5 . 5 5. 1 mmary 1100 010 12,101 01 Outilon-1100 010 20,012 01

Pond 22P: Roof Recharge Peak Elev=3.35' Storage=2,407 cf Inflow=1.26 cfs 3,734 cf

Outflow=0.04 cfs 1,549 cf

Pond 30P: BASIN A Peak Elev=240.01' Storage=3,942 cf Inflow=3.08 cfs 8,616 cf

Discarded=0.22 cfs 7,462 cf Primary=0.07 cfs 87 cf Outflow=0.30 cfs 7,549 cf

Pond 31P: BASIN D

Peak Elev=240.98' Storage=14,812 cf Inflow=10.80 cfs 32,604 cf

Discarded=0.75 cfs 24,595 cf Primary=1.38 cfs 2,366 cf Outflow=2.13 cfs 26,961 cf

Type III 24-hr 100-Year Rainfall=6.70" Printed 3/9/2020

Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 282

Pond 32P: BASIN C Peak Elev=226.06' Storage=23,166 cf Inflow=16.60 cfs 56,653 cf Discarded=1.72 cfs 44,037 cf Primary=4.19 cfs 6,815 cf Outflow=5.91 cfs 50,852 cf

Total Runoff Area = 992,682 sf Runoff Volume = 277,934 cf Average Runoff Depth = 3.36" 64.94% Pervious = 644,625 sf 35.06% Impervious = 348,057 sf

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 283

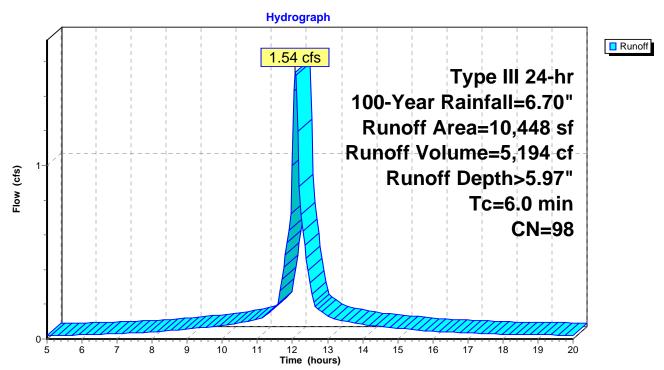
Summary for Subcatchment 34S: 10 Unit Roof

Runoff = 1.54 cfs @ 12.09 hrs, Volume= 5,194 cf, Depth> 5.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Α	rea (sf)	CN	Description		
*		10,448	98	Impervious		
		10,448		100.00% lm	npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment 34S: 10 Unit Roof



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 284

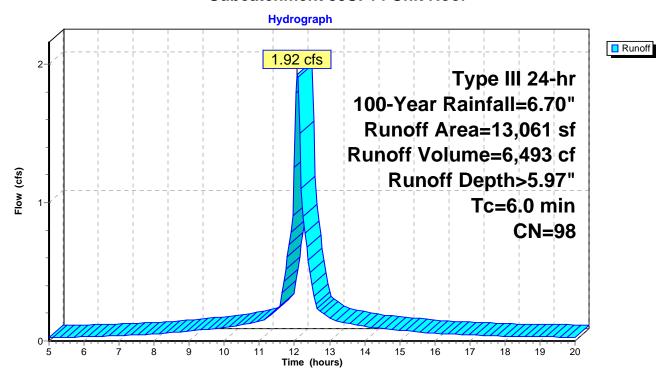
Summary for Subcatchment 35S: 14 Unit Roof

Runoff = 1.92 cfs @ 12.09 hrs, Volume= 6,493 cf, Depth> 5.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Α	rea (sf)	CN	Description		
*		13,061	98	mpervious		
	13,061 100.00% Impervious Are					Area
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	· ·
	6.0					Direct Entry,

Subcatchment 35S: 14 Unit Roof



Page 285

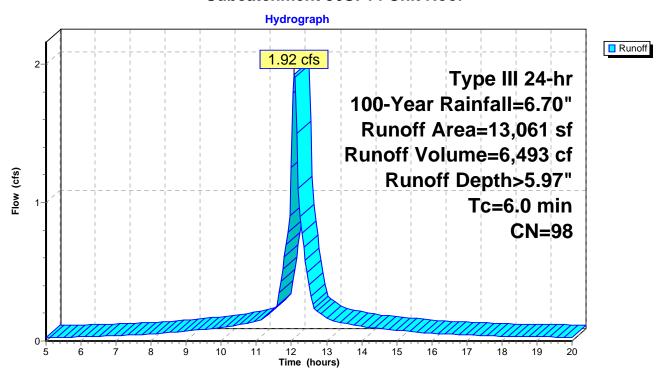
Summary for Subcatchment 36S: 14 Unit Roof

Runoff = 1.92 cfs @ 12.09 hrs, Volume= 6,493 cf, Depth> 5.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

_	Α	rea (sf)	CN	Description		
*		13,061	98	mpervious		
		13,061		100.00% Im	npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment 36S: 14 Unit Roof



Page 286

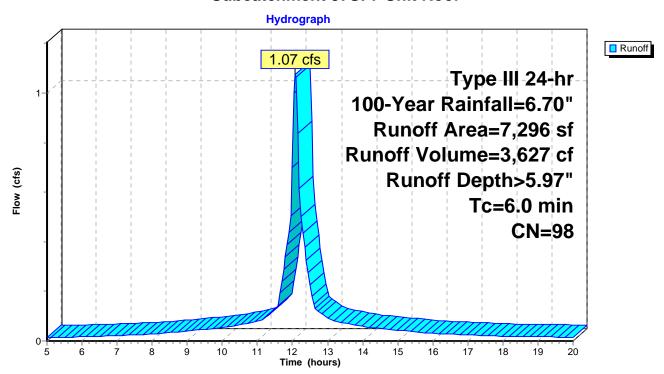
Summary for Subcatchment 37S: 7 Unit Roof

Runoff = 1.07 cfs @ 12.09 hrs, Volume= 3,627 cf, Depth> 5.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

_	Α	rea (sf)	CN	Description					
*		7,296	98	Impervious					
		7,296		100.00% Impervious Area					
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment 37S: 7 Unit Roof



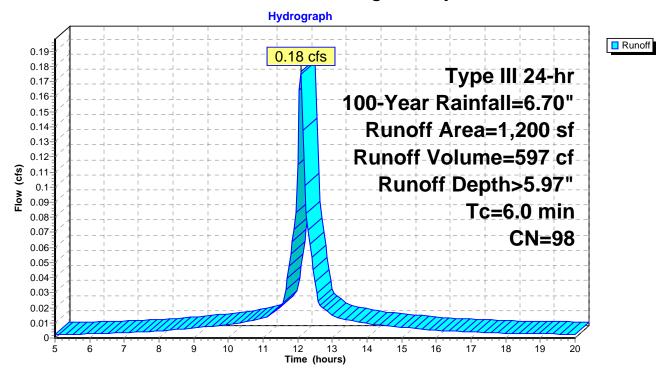
Summary for Subcatchment LOT 1: Single Family House

Runoff = 0.18 cfs @ 12.09 hrs, Volume= 597 cf, Depth> 5.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Α	rea (sf)	CN I	Description				
*		1,200	98	Impervious				
		1,200		100.00% Im	npervious A	Area		
	Тс	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	6.0					Direct Entry,		

Subcatchment LOT 1: Single Family House



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 288

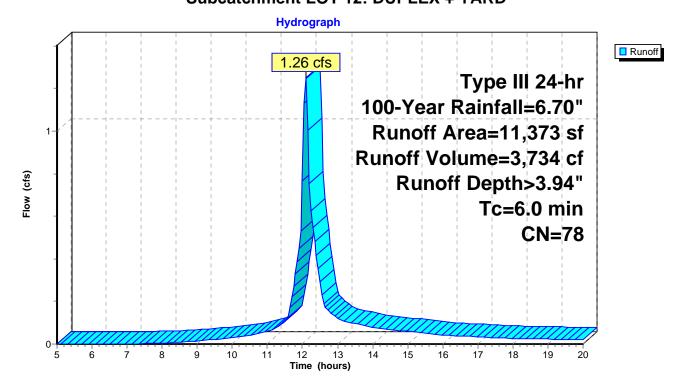
Summary for Subcatchment LOT 12: DUPLEX + YARD

Runoff = 1.26 cfs @ 12.09 hrs, Volume= 3,734 cf, Depth> 3.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Area (sf)	CN	Description							
*	4,929	98	Impervious							
	490	80	>75% Grass cover, Good, HSG D							
	5,954	61	>75% Grass	75% Grass cover, Good, HSG B						
	11,373	78	Weighted Average							
	6,444		56.66% Pervious Area							
	4,929		43.34% Imp	ervious Ar	ea					
Tc (min)	- 3	Slope	,	Capacity (cfs)	Description					
	(.001)	(1010	, (.3000)	(0.0)	Direct Entry.					
Tc (min) 6.0	,		e Velocity							

Subcatchment LOT 12: DUPLEX + YARD



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 289

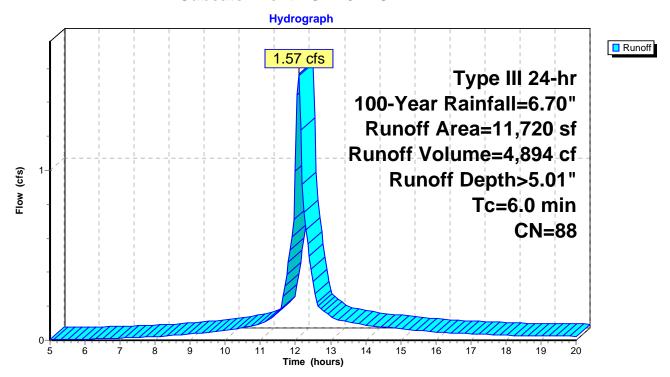
Summary for Subcatchment LOT 13: DUPLEX + YARD

Runoff = 1.57 cfs @ 12.09 hrs, Volume= 4,894 cf, Depth> 5.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Α	rea (sf)	CN	Description						
*		4,929	98	Impervious						
_		6,791	80	>75% Grass cover, Good, HSG D						
		11,720	88	Veighted Average						
		6,791		57.94% Pervious Area						
		4,929		42.06% Imp	ervious Ar	rea				
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry,				

Subcatchment LOT 13: DUPLEX + YARD



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 290

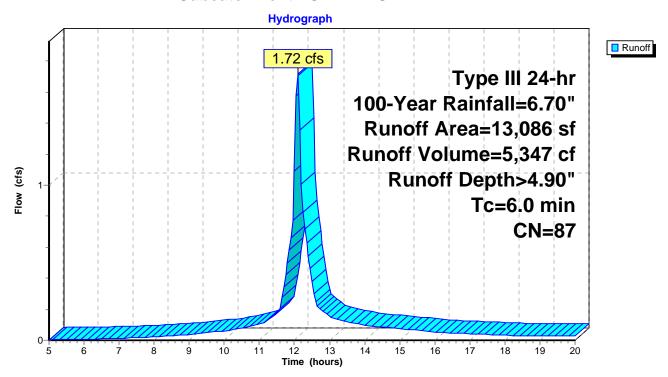
Summary for Subcatchment LOT 14: DUPLEX + YARD

Runoff = 1.72 cfs @ 12.09 hrs, Volume= 5,347 cf, Depth> 4.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Α	rea (sf)	CN I	Description						
*		4,929	98	Impervious						
_		8,157	80 :	>75% Grass cover, Good, HSG D						
		13,086	87 Y	Veighted Average						
		8,157		62.33% Pervious Area						
		4,929	;	37.67% Imp	ervious Ar	rea				
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry,				

Subcatchment LOT 14: DUPLEX + YARD



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 291

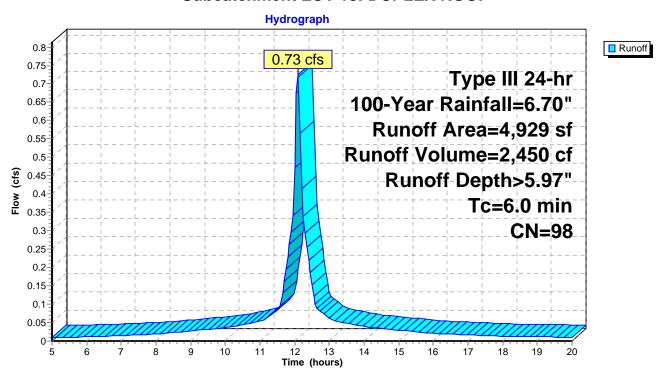
Summary for Subcatchment LOT 18: DUPLEX ROOF

Runoff = 0.73 cfs @ 12.09 hrs, Volume= 2,450 cf, Depth> 5.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Α	rea (sf)	CN I	Description				
*		4,929	98	Impervious				
		4,929		100.00% Im	npervious A	Area		
	Тс	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	6.0					Direct Entry,		

Subcatchment LOT 18: DUPLEX ROOF



Page 292

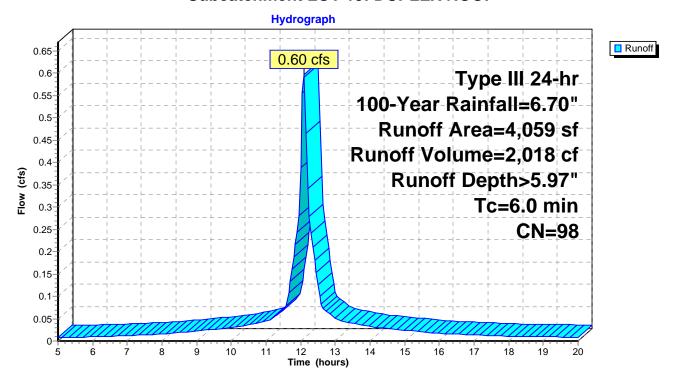
Summary for Subcatchment LOT 19: DUPLEX ROOF

Runoff = 0.60 cfs @ 12.09 hrs, Volume= 2,018 cf, Depth> 5.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Α	rea (sf)	CN I	Description					
*		4,059	98 I	mpervious					
		4,059	•	100.00% Impervious Area					
	Тс	Length	Slope	,	Capacity	•			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 19: DUPLEX ROOF



Page 293

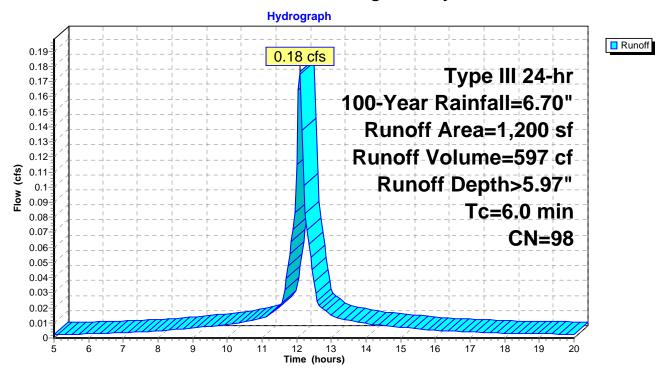
Summary for Subcatchment LOT 2: Single Family House

Runoff = 0.18 cfs @ 12.09 hrs, Volume= 597 cf, Depth> 5.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Α	rea (sf)	CN I	Description				
*		1,200	98	Impervious				
		1,200		100.00% Im	npervious A	Area		
	Тс	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	6.0					Direct Entry,		

Subcatchment LOT 2: Single Family House



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 294

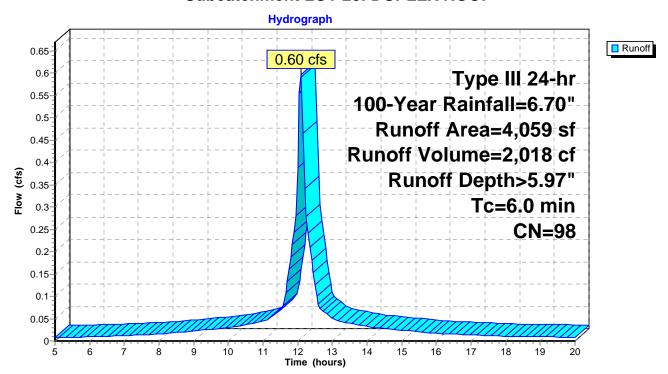
Summary for Subcatchment LOT 20: DUPLEX ROOF

Runoff = 0.60 cfs @ 12.09 hrs, Volume= 2,018 cf, Depth> 5.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Α	rea (sf)	CN I	Description					
*		4,059	98 I	mpervious					
		4,059	•	100.00% Impervious Area					
	Тс	Length	Slope	,	Capacity	•			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 20: DUPLEX ROOF



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 295

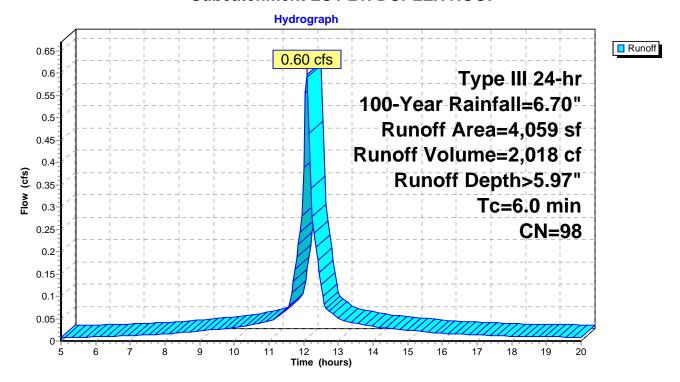
Summary for Subcatchment LOT 21: DUPLEX ROOF

Runoff = 0.60 cfs @ 12.09 hrs, Volume= 2,018 cf, Depth> 5.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Α	rea (sf)	CN	Description		
*		4,059	98	mpervious		
		4,059	,	100.00% Im	npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment LOT 21: DUPLEX ROOF



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 296

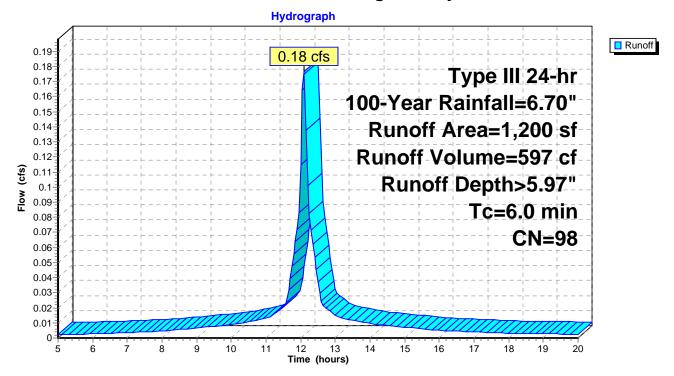
Summary for Subcatchment LOT 3: Single Family House

Runoff = 0.18 cfs @ 12.09 hrs, Volume= 597 cf, Depth> 5.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Α	rea (sf)	CN I	Description					
*		1,200	98	mpervious					
		1,200		100.00% Impervious Area					
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Subcatchment LOT 3: Single Family House



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 297

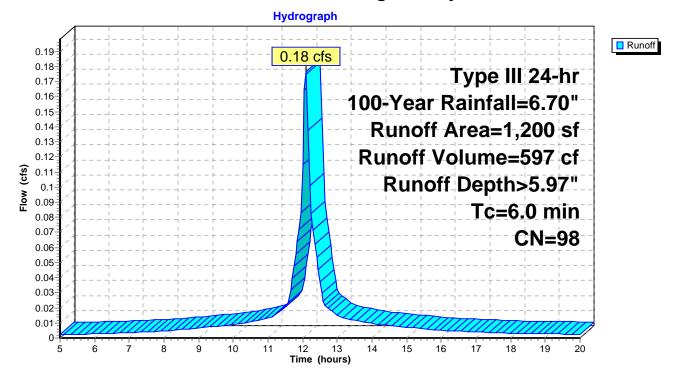
Summary for Subcatchment LOT 8: Single Family House

Runoff = 0.18 cfs @ 12.09 hrs, Volume= 597 cf, Depth> 5.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

_	Α	rea (sf)	CN I	Description		
*		1,200	98	mpervious		
		1,200		100.00% Im	npervious A	Area
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Subcatchment LOT 8: Single Family House



Page 298

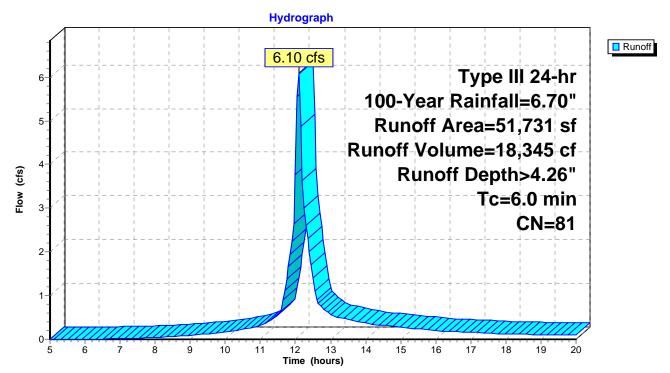
Summary for Subcatchment PRDA-1: TO BASIN D

Runoff = 6.10 cfs @ 12.09 hrs, Volume= 18,345 cf, Depth> 4.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Area (sf)	CN	Description				
*	25,896	98	Impervious				
	6,014	54	1/2 acre lots	s, 25% imp	, HSG A		
	6,314	80	1/2 acre lots	s, 25% imp	, HSG C		
	6,846	39	>75% Gras	s cover, Go	ood, HSG A		
	462	74	>75% Gras	s cover, Go	ood, HSG C		
	6,199	80	>75% Gras	s cover, Go	ood, HSG D		
	51,731	81	Weighted A	verage			
	22,753		43.98% Per	vious Area			
	28,978		56.02% Imp	ervious Ar	ea		
To	Length	Slop	e Velocity	Capacity	Description		
(min	(feet)	(ft/f	t) (ft/sec)	(cfs)			
6.0)				Direct Entry,		

Subcatchment PRDA-1: TO BASIN D



Page 299

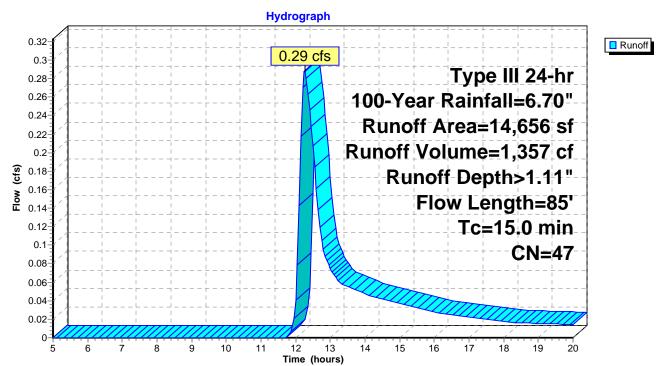
Summary for Subcatchment PRDA-10: TO ILSF

Runoff = 0.29 cfs @ 12.26 hrs, Volume= 1,357 cf, Depth> 1.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

_	Α	rea (sf)	CN I	CN Description							
		1,166	39 :	39 >75% Grass cover, Good, HSG A							
		2,000	80 :	>75% Gras	s cover, Go	ood, HSG D					
		8,622	30 \	Noods, Go	od, HSG A						
_		2,868	77 \	Noods, Go	od, HSG D						
		14,656	47 ١	Neighted A	verage						
		14,656	•	100.00% Pe	ervious Are	a					
	Tc	Length	Slope	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	14.2	50	0.0140	0.06		Sheet Flow, AB					
						Woods: Light underbrush n= 0.400 P2= 3.20"					
	8.0	35	0.0220	0.74		Shallow Concentrated Flow, BC					
_						Woodland Kv= 5.0 fps					
	15.0	85	Total		·						

Subcatchment PRDA-10: TO ILSF



Page 300

Summary for Subcatchment PRDA-11: BASIN A DIRECT

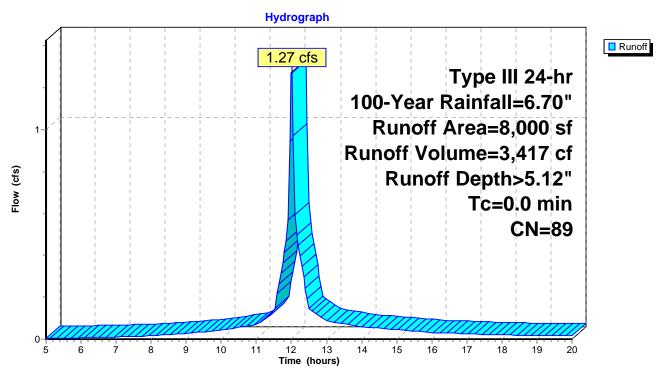
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 1.27 cfs @ 12.00 hrs, Volume= 3,417 cf, Depth> 5.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

/	Area (sf)	CN	Description			
	3,983	98	Water Surface, HSG B			
	4,017	80	>75% Grass cover, Good, HSG D			
	8,000	89	Weighted Average			
	4,017		50.21% Pervious Area			
	3,983		49.79% Impervious Area			

Subcatchment PRDA-11: BASIN A DIRECT



Page 301

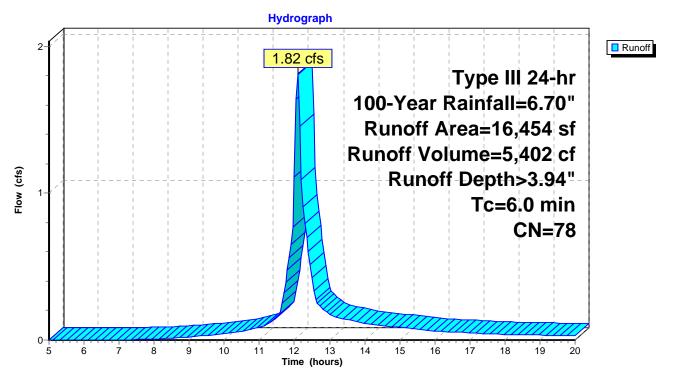
Summary for Subcatchment PRDA-12: BASIN B DIRECT

Runoff = 1.82 cfs @ 12.09 hrs, Volume= 5,402 cf, Depth> 3.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

Area (sf)	CN	Description						
7,423	98	Water Surface, HSG D						
5,050	80	>75% Grass cover, Good, HSG D						
3,981	39	>75% Grass cover, Good, HSG A						
16,454	78	Weighted Average	Weighted Average					
9,031		54.89% Pervious Area						
7,423		45.11% Impervious Area						
Tc Length	Slop	pe Velocity Capacity Description						
(min) (feet)	(ft/	/ft) (ft/sec) (cfs)						
6.0		Direct Entry.						

Subcatchment PRDA-12: BASIN B DIRECT



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 302

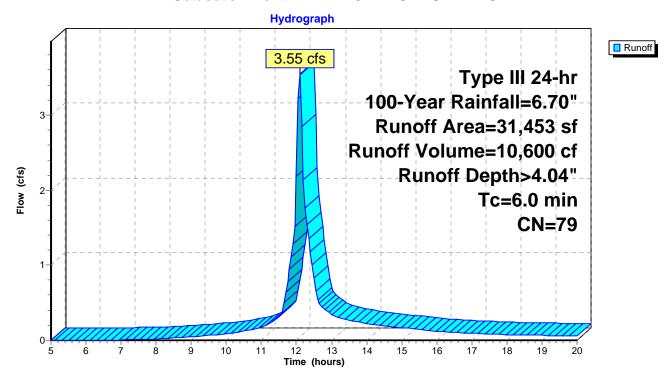
Summary for Subcatchment PRDA-13: BASIN C DIRECT

Runoff = 3.55 cfs @ 12.09 hrs, Volume= 10,600 cf, Depth> 4.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

Are	ea (sf)	CN	Description						
1	5,825	61	>75% Grass cover, Good, HSG B						
1	5,628	98	Water Surfa	ice, HSG B					
3	31,453	79	Weighted A	Weighted Average					
1	5,825		50.31% Per	vious Area					
1	5,628		49.69% Imp	ervious Are	ea				
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft							
6.0					Direct Entry,				

Subcatchment PRDA-13: BASIN C DIRECT



Page 303

Summary for Subcatchment PRDA-2: TO BASIN A

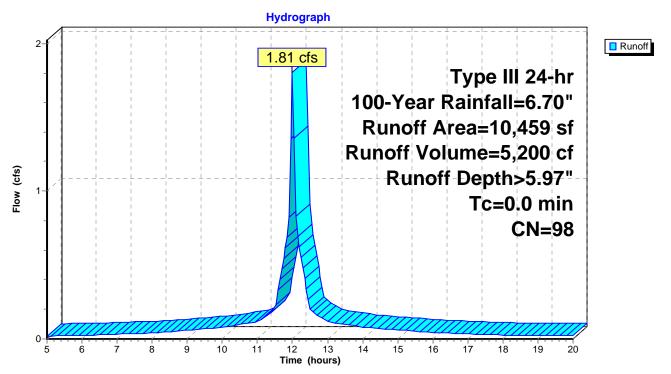
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 1.81 cfs @ 12.00 hrs, Volume= 5,200 cf, Depth> 5.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Area (sf)	CN	Description			
*	10,459	98	IMPERVIOUS			
	10,459		100.00% Impervious Area			

Subcatchment PRDA-2: TO BASIN A



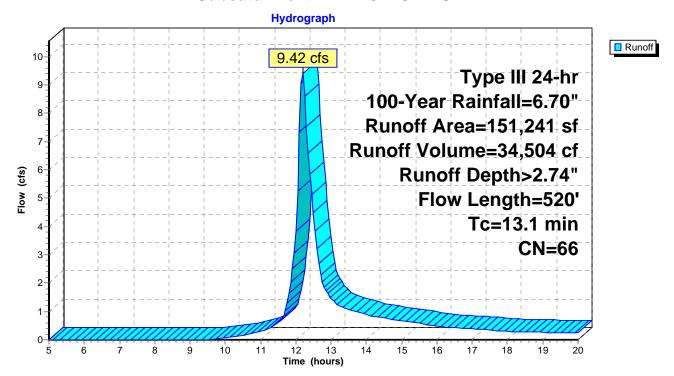
Summary for Subcatchment PRDA-3: TO BASIN B

Runoff = 9.42 cfs @ 12.19 hrs, Volume= 34,504 cf, Depth> 2.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Α	rea (sf)	CN E	escription							
*		40,248	98 II	8 IMPERVIOUS							
		14,787	98 F	Roofs, HSG	βA						
		77,313	39 >	75% Gras	s cover, Go	ood, HSG A					
		92	61 >	75% Gras	s cover, Go	ood, HSG B					
		18,801	80 >	75% Gras	s cover, Go	ood, HSG D					
	1	51,241		Veighted A							
	96,206 63.61% Pervious Area										
		55,035	3	6.39% Imp	ervious Ar	ea					
	_		01			B 1.0					
	Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	6.3	50	0.0380	0.13		Sheet Flow,					
						Grass: Dense n= 0.240 P2= 3.20"					
	6.5	406	0.0220	1.04		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	0.3	64	0.0360	3.85		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	13.1	520	Total								

Subcatchment PRDA-3: TO BASIN B



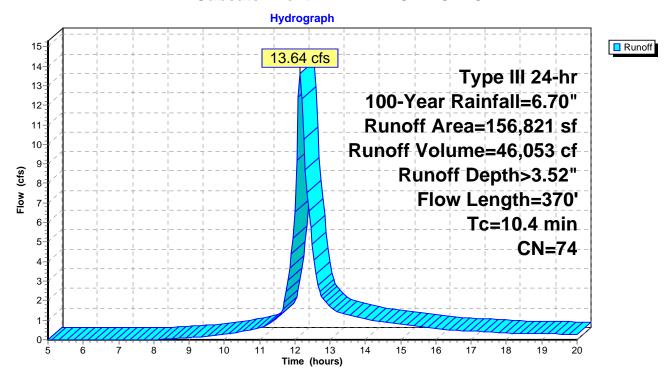
Summary for Subcatchment PRDA-4: TO BASIN C

Runoff = 13.64 cfs @ 12.15 hrs, Volume= 46,053 cf, Depth> 3.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Α	rea (sf)	CN D	escription		
*		70,874	98 lı	npervious		
		4,929	98 F	oofs, HSC	ВВ	
		35,430	39 >	75% Gras	s cover, Go	ood, HSG A
		44,834	61 >	75% Gras	s cover, Go	ood, HSG B
		754	80 >	75% Gras	s cover, Go	ood, HSG D
	1	56,821	74 V	Veighted A	verage	
	81,018 51.66% Pervious Area					
		75,803	4	8.34% Imp	ervious Ar	ea
	_				_	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.8	50	0.0320	0.12		Sheet Flow,
						Grass: Dense n= 0.240 P2= 3.20"
	3.1	217	0.0280	1.17		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.5	103	0.0270	3.34		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	10.4	370	Total			

Subcatchment PRDA-4: TO BASIN C



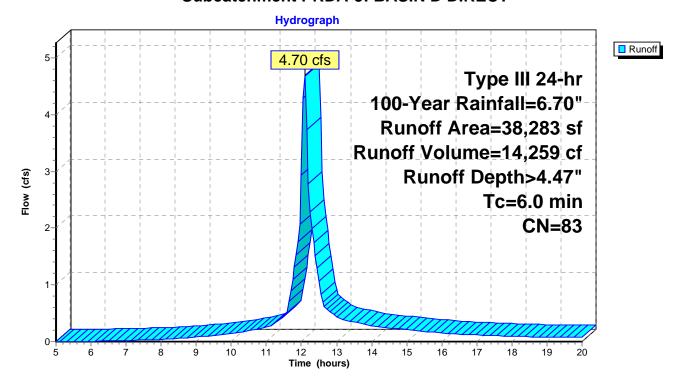
Summary for Subcatchment PRDA-5: BASIN D DIRECT

Runoff = 4.70 cfs @ 12.09 hrs, Volume= 14,259 cf, Depth> 4.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Area (sf)	CN	Description							
*	1,132	98	Impervious	Impervious						
	7,697	80	1/2 acre lot	s, 25% imp	o, HSG C					
	374	39	>75% Gras	s cover, Go	ood, HSG A					
	7,237	74	>75% Gras	s cover, Go	lood, HSG C					
	6,781	80	>75% Gras	s cover, Go	lood, HSG D					
	9,227	98	Water Surfa	ace, HSG A	A					
	4,080	77	Woods, Go	od, HSG D)					
	1,323	70	Woods, Go	,						
	432	96	Gravel surfa	ace, HSG C	<u>C</u>					
	38,283	83	Weighted A	verage						
	26,000		67.91% Per	vious Area	a					
	12,283		32.09% lmp	pervious Ar	rea					
_		٠.			-					
	c Length	Slop		Capacity	·					
(min	, , ,	(ft/	ft) (ft/sec)	(cfs)						
6.	0				Direct Entry,					

Subcatchment PRDA-5: BASIN D DIRECT



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 307

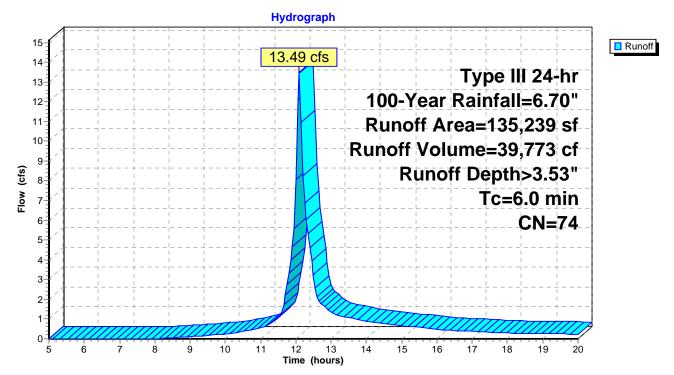
Summary for Subcatchment PRDA-6: TO BASIN E

Runoff = 13.49 cfs @ 12.09 hrs, Volume= 39,773 cf, Depth> 3.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	rea (sf)	CN	Description					
*	19,869	98	IMPERVIO	JS				
	57,855	80	1/2 acre lots	s, 25% imp	o, HSG C			
	2,327	54	1/2 acre lot	s, 25% imp	o, HSG A			
	3,600	98	Roofs, HSG	βA				
	27,519	39	>75% Gras	s cover, Go	ood, HSG A			
	16,283	74	>75% Gras	s cover, Go	ood, HSG C			
	1,810	80	>75% Grass cover, Good, HSG D					
	5,976	98	Water Surface, HSG D					
•	135,239 74 Weighted Average		verage					
	90,749 67.1		67.10% Pei	67.10% Pervious Area				
44,491			32.90% Impervious Area					
_		01		•				
Tc	Length	Slop		Capacity	Description			
<u>(min)</u>	(feet)	(ft/f	t) (ft/sec)	(cfs)				
6.0					Direct Entry,			

Subcatchment PRDA-6: TO BASIN E



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 308

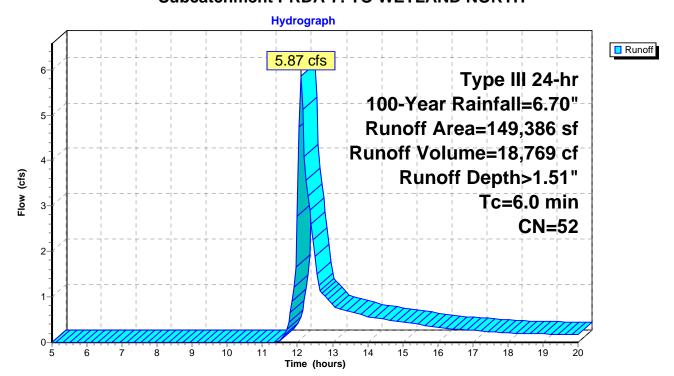
Summary for Subcatchment PRDA-7: TO WETLAND NORTH

Runoff = 5.87 cfs @ 12.11 hrs, Volume= 18,769 cf, Depth> 1.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

A	rea (sf)	CN	Description						
	2,951	80	1/2 acre lots, 25% imp, HSG C						
	8,254	54	1/2 acre lots	s, 25% imp	, HSG A				
	52,416	39	>75% Grass	s cover, Go	ood, HSG A				
	27,465	61	>75% Grass	s cover, Go	ood, HSG B				
	538	74	>75% Grass	s cover, Go	ood, HSG C				
	9,934	80	>75% Grass	s cover, Go	ood, HSG D				
	22,059	30	Woods, Go	od, HSG A					
	14,418	70	Woods, Good, HSG C						
	9,973	77	Woods, Good, HSG D						
*	1,378	98	impervious						
1	49,386	52	Weighted A	verage					
1	145,207 97.20% Pervious Ar		vious Area						
	4,179	2.80% Impervious Area							
Tc	Length	Slop		Capacity	Description				
<u>(min)</u>	(feet)	(ft/f	t) (ft/sec)	(cfs)					
6.0					Direct Entry,				

Subcatchment PRDA-7: TO WETLAND NORTH



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 309

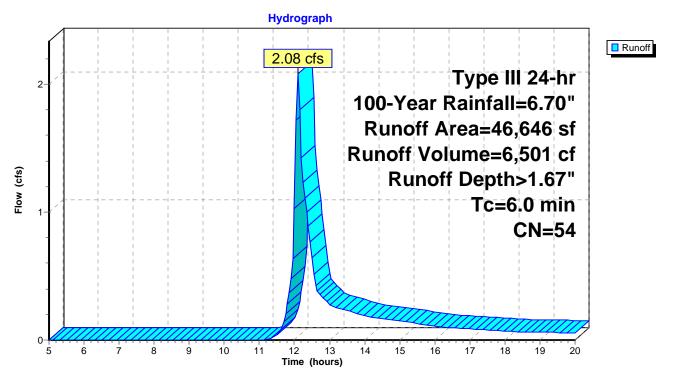
Summary for Subcatchment PRDA-8: TO WETLAND SOUTHEAST

Runoff = 2.08 cfs @ 12.10 hrs, Volume= 6,501 cf, Depth> 1.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

Ar	ea (sf)	CN	Description					
2	21,559	39	>75% Grass	s cover, Go	od, HSG A			
	7,434	80	>75% Grass	s cover, Go	od, HSG D			
•	12,118	77	Woods, Good, HSG D					
	5,535	30	Woods, Good, HSG A					
	46,646	54	Weighted A	verage				
4	46,646	646 100.00% Pervious Area						
Тс	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft	t) (ft/sec)	(cfs)				
6.0					Direct Entry			

Subcatchment PRDA-8: TO WETLAND SOUTHEAST



Page 310

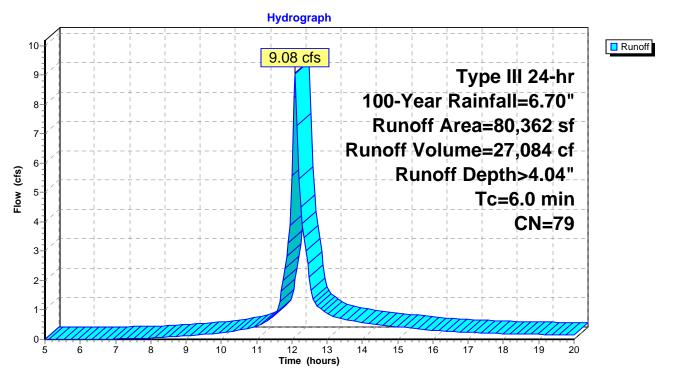
Summary for Subcatchment PRDA-9: TO WETLAND SOUTH

Runoff = 9.08 cfs @ 12.09 hrs, Volume= 27,084 cf, Depth> 4.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=6.70"

	Area (sf)	CN	Description						
	2	74	39	>75% Grass cover, Good, HSG A						
	20,0	73	80	>75% Gras	s cover, Go	od, HSG D				
	7,9	04	70	Woods, Go	od, HSG C					
	34,2	17	77	Woods, Go	od, HSG D					
	11,5	44	80	1/2 acre lots	s, 25% imp	, HSG C				
*	6,3	50	98	IMPERVIOUS						
	80,3	62	79	Weighted Average						
	71,1	26		88.51% Pervious Area						
	9,2	36		11.49% Impervious Area						
	Tc Len	igth	Slope		Capacity	Description				
(m	in) (fe	eet)	(ft/ft)	(ft/sec)	(cfs)					
6	6.0					Direct Entry,				

Subcatchment PRDA-9: TO WETLAND SOUTH



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 311

Summary for Reach 1R: ILSF

[40] Hint: Not Described (Outflow=Inflow)

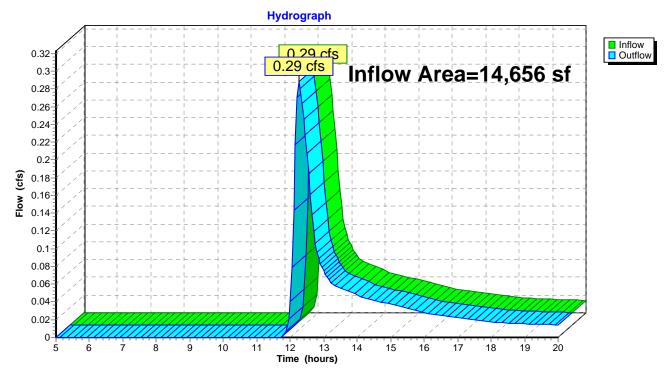
Inflow Area = 14,656 sf, 0.00% Impervious, Inflow Depth > 1.11" for 100-Year event

Inflow = 0.29 cfs @ 12.26 hrs, Volume= 1,357 cf

Outflow = 0.29 cfs @ 12.26 hrs, Volume= 1,357 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 1R: ILSF



Summary for Reach 14R: WETLAND SOUTH

[40] Hint: Not Described (Outflow=Inflow)

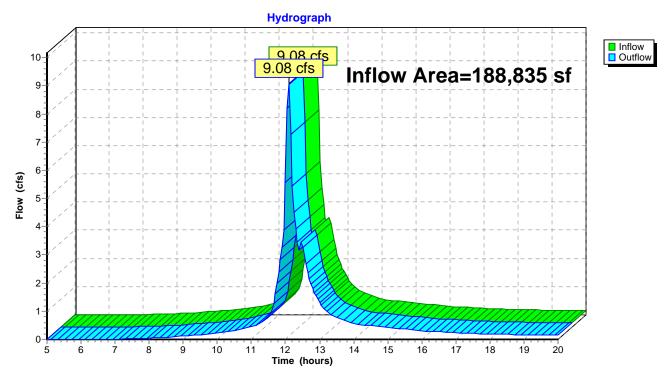
Inflow Area = 188,835 sf, 34.39% Impervious, Inflow Depth > 1.88" for 100-Year event

Inflow = 9.08 cfs @ 12.09 hrs, Volume= 29,537 cf

Outflow = 9.08 cfs @ 12.09 hrs, Volume= 29,537 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 14R: WETLAND SOUTH



Summary for Reach 15R: WETLAND SOUTHEAST

[40] Hint: Not Described (Outflow=Inflow)

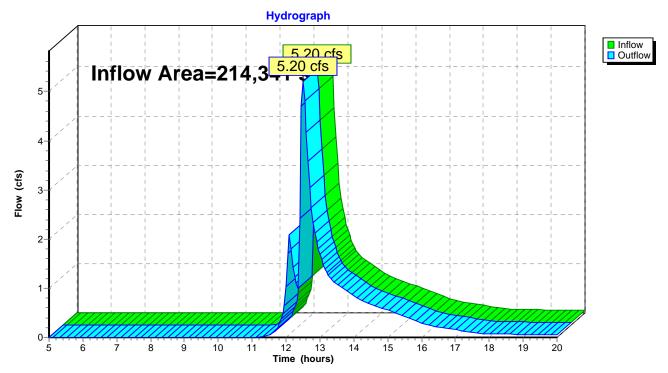
Inflow Area = 214,341 sf, 29.14% Impervious, Inflow Depth > 1.08" for 100-Year event

Inflow = 5.20 cfs @ 12.50 hrs, Volume= 19,298 cf

Outflow = 5.20 cfs @ 12.50 hrs, Volume= 19,298 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 15R: WETLAND SOUTHEAST



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 314

Summary for Reach 21R: WETLAND NORTH

[40] Hint: Not Described (Outflow=Inflow)

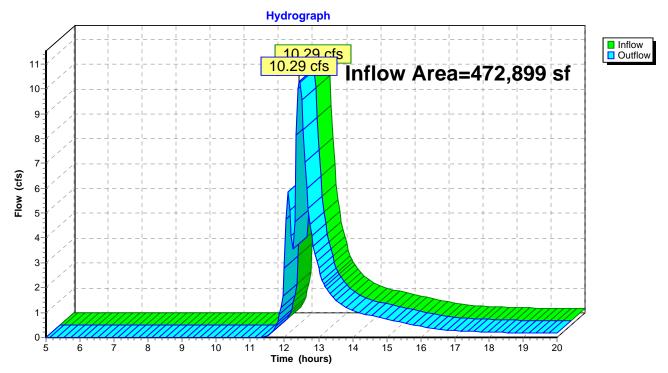
Inflow Area = 472,899 sf, 29.63% Impervious, Inflow Depth > 0.92" for 100-Year event

Inflow = 10.29 cfs @ 12.44 hrs, Volume= 36,353 cf

Outflow = 10.29 cfs @ 12.44 hrs, Volume= 36,353 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach 21R: WETLAND NORTH



Type III 24-hr 100-Year Rainfall=6.70" Printed 3/9/2020

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 315

Summary for Pond 1P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 10,448 sf,100.00% Impervious, Inflow Depth > 5.97" for 100-Year event

Inflow = 1.54 cfs @ 12.09 hrs, Volume= 5,194 cf

Outflow = 0.11 cfs @ 13.37 hrs, Volume= 4,290 cf, Atten= 93%, Lag= 76.9 min

Discarded = 0.11 cfs @ 13.37 hrs, Volume= 4,290 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 2.09' @ 13.37 hrs Surf.Area= 1,531 sf Storage= 2,271 cf

Plug-Flow detention time= 153.8 min calculated for 4,274 cf (82% of inflow)

Center-of-Mass det. time= 104.0 min (837.7 - 733.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,317 cf	20.83'W x 73.50'L x 3.54'H Field A
			5,423 cf Overall - 2,131 cf Embedded = 3,292 cf x 40.0% Voids
#2A	0.50'	2,131 cf	Cultec R-330XLHD x 40 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 4 rows
•			

3,448 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.11 cfs @ 13.37 hrs HW=2.09' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.11 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 316

Pond 1P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

10 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 71.50' Row Length +12.0" End Stone x 2 = 73.50' Base Length

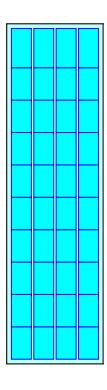
4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

40 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 2,131.0 cf Chamber Storage

5,423.2 cf Field - 2,131.0 cf Chambers = 3,292.2 cf Stone x 40.0% Voids = 1,316.9 cf Stone Storage

Chamber Storage + Stone Storage = 3,447.9 cf = 0.079 af Overall Storage Efficiency = 63.6% Overall System Size = 73.50' x 20.83' x 3.54'

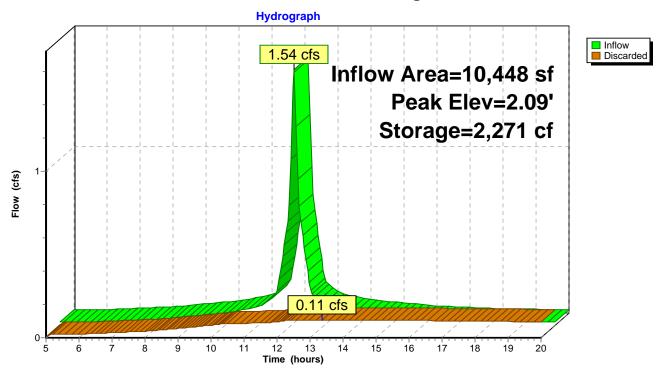
40 Chambers 200.9 cy Field 121.9 cy Stone





Page 317

Pond 1P: Roof Recharge



Type III 24-hr 100-Year Rainfall=6.70"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Page 318

Summary for Pond 2P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 13,061 sf,100.00% Impervious, Inflow Depth > 5.97" for 100-Year event

Inflow = 1.92 cfs @ 12.09 hrs, Volume= 6,493 cf

Outflow = 0.07 cfs @ 15.52 hrs, Volume= 3,014 cf, Atten= 97%, Lag= 205.7 min

Discarded = 0.07 cfs @ 15.52 hrs, Volume= 3,014 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 2.43' @ 15.52 hrs Surf.Area= 2,246 sf Storage= 3,907 cf

Plug-Flow detention time= 173.1 min calculated for 3,002 cf (46% of inflow)

Center-of-Mass det. time= 70.2 min (803.9 - 733.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,907 cf	25.67'W x 87.50'L x 3.54'H Field A
			7,954 cf Overall - 3,185 cf Embedded = 4,769 cf \times 40.0% Voids
#2A	0.50'	3,185 cf	Cultec R-330XLHD x 60 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 5 rows
•			

5,093 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.07 cfs @ 15.52 hrs HW=2.43' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.07 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 319

Pond 2P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 5 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

12 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 85.50' Row Length +12.0" End Stone x 2 = 87.50' Base Length

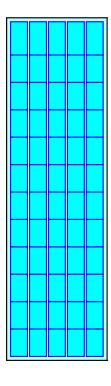
5 Rows x 52.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 25.67' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

60 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 5 Rows = 3,185.3 cf Chamber Storage

7,954.0 cf Field - 3,185.3 cf Chambers = 4,768.7 cf Stone x 40.0% Voids = 1,907.5 cf Stone Storage

Chamber Storage + Stone Storage = 5,092.8 cf = 0.117 af Overall Storage Efficiency = 64.0% Overall System Size = 87.50' x 25.67' x 3.54'

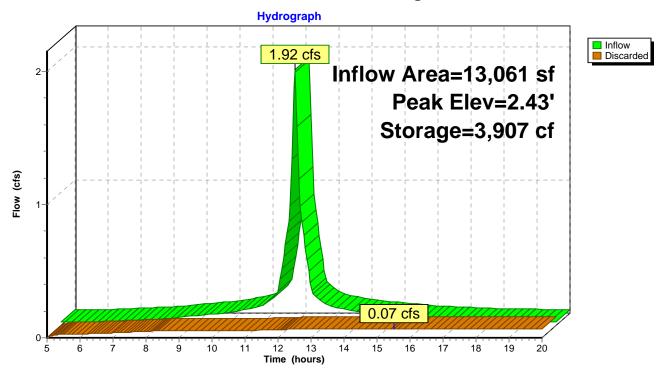
60 Chambers 294.6 cy Field 176.6 cy Stone





Page 320

Pond 2P: Roof Recharge



Type III 24-hr 100-Year Rainfall=6.70"

Prepared by HP Printed 3/9/2020

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 321

Summary for Pond 3P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 13,061 sf,100.00% Impervious, Inflow Depth > 5.97" for 100-Year event

Inflow = 1.92 cfs @ 12.09 hrs, Volume= 6,493 cf

Outflow = 0.07 cfs @ 15.52 hrs, Volume= 3,014 cf, Atten= 97%, Lag= 205.7 min

Discarded = 0.07 cfs @ 15.52 hrs, Volume= 3,014 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 2.43' @ 15.52 hrs Surf.Area= 2,246 sf Storage= 3,907 cf

Plug-Flow detention time= 173.1 min calculated for 3,002 cf (46% of inflow)

Center-of-Mass det. time= 70.2 min (803.9 - 733.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,907 cf	25.67'W x 87.50'L x 3.54'H Field A
			7,954 cf Overall - 3,185 cf Embedded = 4,769 cf \times 40.0% Voids
#2A	0.50'	3,185 cf	Cultec R-330XLHD x 60 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 5 rows
•			

5,093 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.07 cfs @ 15.52 hrs HW=2.43' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.07 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 322

Pond 3P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 5 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

12 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 85.50' Row Length +12.0" End Stone x 2 = 87.50' Base Length

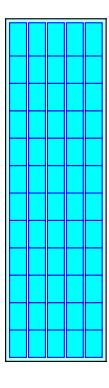
5 Rows x 52.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 25.67' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

60 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 5 Rows = 3,185.3 cf Chamber Storage

7,954.0 cf Field - 3,185.3 cf Chambers = 4,768.7 cf Stone x 40.0% Voids = 1,907.5 cf Stone Storage

Chamber Storage + Stone Storage = 5,092.8 cf = 0.117 af Overall Storage Efficiency = 64.0% Overall System Size = 87.50' x 25.67' x 3.54'

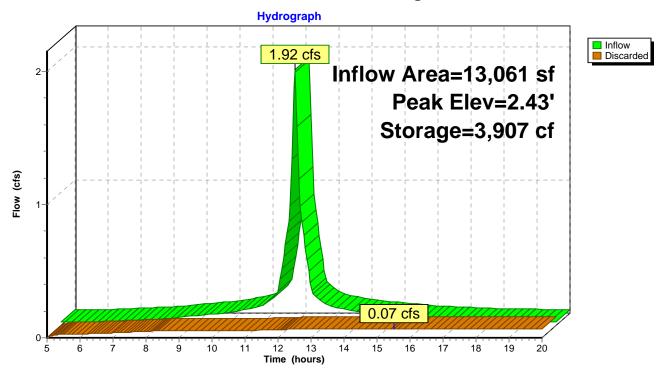
60 Chambers 294.6 cy Field 176.6 cy Stone





Page 323

Pond 3P: Roof Recharge



Type III 24-hr 100-Year Rainfall=6.70" Printed 3/9/2020

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 324

Summary for Pond 4P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 7,296 sf,100.00% Impervious, Inflow Depth > 5.97" for 100-Year event

Inflow = 1.07 cfs @ 12.09 hrs, Volume= 3,627 cf

Outflow = 0.04 cfs @ 15.38 hrs, Volume= 1,729 cf, Atten= 96%, Lag= 197.6 min

Discarded = 0.04 cfs @ 15.38 hrs, Volume= 1,729 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 2.47' @ 15.38 hrs Surf.Area= 1,240 sf Storage= 2,164 cf

Plug-Flow detention time= 175.1 min calculated for 1,722 cf (47% of inflow)

Center-of-Mass det. time= 74.6 min (808.3 - 733.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,071 cf	20.83'W x 59.50'L x 3.54'H Field A
			4,390 cf Overall - 1,714 cf Embedded = 2,676 cf \times 40.0% Voids
#2A	0.50'	1,714 cf	Cultec R-330XLHD x 32 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

2,784 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.04 cfs @ 15.38 hrs HW=2.47' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.04 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 325

Pond 4P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

8 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 57.50' Row Length +12.0" End Stone x 2 = 59.50' Base Length

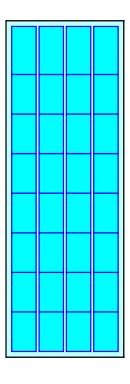
4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

32 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 1,713.7 cf Chamber Storage

4,390.2 cf Field - 1,713.7 cf Chambers = 2,676.5 cf Stone x 40.0% Voids = 1,070.6 cf Stone Storage

Chamber Storage + Stone Storage = 2,784.3 cf = 0.064 af Overall Storage Efficiency = 63.4% Overall System Size = 59.50' x 20.83' x 3.54'

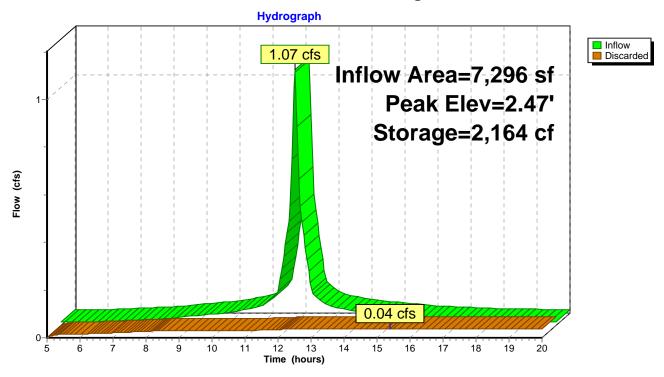
32 Chambers 162.6 cy Field 99.1 cy Stone





Page 326

Pond 4P: Roof Recharge



Type III 24-hr 100-Year Rainfall=6.70"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Page 327

Summary for Pond 5P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 5.97" for 100-Year event

Inflow = 0.18 cfs @ 12.09 hrs, Volume= 597 cf

Outflow = 0.01 cfs @ 14.07 hrs, Volume= 360 cf, Atten= 95%, Lag= 118.8 min

Discarded = 0.01 cfs @ 14.07 hrs, Volume= 360 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 2.54' @ 14.07 hrs Surf.Area= 200 sf Storage= 326 cf

Plug-Flow detention time= 182.5 min calculated for 360 cf (60% of inflow)

Center-of-Mass det. time= 101.4 min (835.1 - 733.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows
			—

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 14.07 hrs HW=2.54' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.01 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 328

Pond 5P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

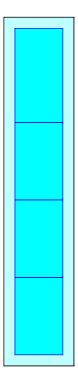
Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

- 4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length
- 1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height
- 4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

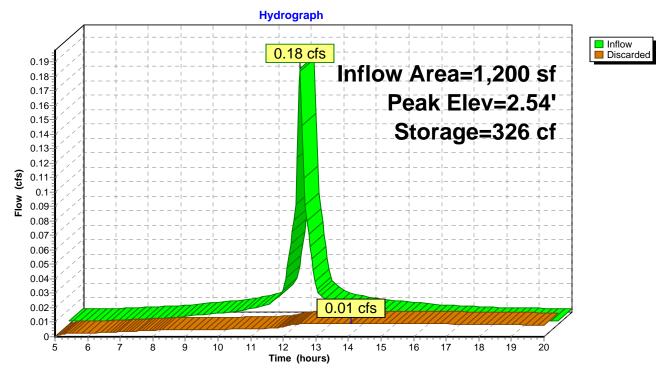
4 Chambers 26.2 cy Field 18.0 cy Stone





Page 329

Pond 5P: Roof Recharge



Type III 24-hr 100-Year Rainfall=6.70" Printed 3/9/2020

Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 330

Summary for Pond 6P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 4,059 sf,100.00% Impervious, Inflow Depth > 5.97" for 100-Year event

Inflow = 0.60 cfs @ 12.09 hrs, Volume= 2,018 cf

Outflow = 0.03 cfs @ 14.37 hrs, Volume= 1,223 cf, Atten= 95%, Lag= 137.2 min

Discarded = 0.03 cfs @ 14.37 hrs, Volume= 1,223 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.75' @ 14.37 hrs Surf.Area= 899 sf Storage= 1,068 cf

Plug-Flow detention time= 169.3 min calculated for 1,219 cf (60% of inflow)

Center-of-Mass det. time= 89.6 min (823.3 - 733.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	806 cf	11.17'W x 80.50'L x 3.54'H Field A
			$3,184 \text{ cf Overall - } 1,170 \text{ cf Embedded = } 2,014 \text{ cf } \times 40.0\% \text{ Voids}$
#2A	0.50'	1,170 cf	Cultec R-330XLHD x 22 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
	<u> </u>		

1,975 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.03 cfs @ 14.37 hrs HW=1.75' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.03 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 331

Pond 6P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

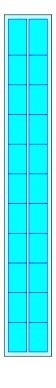
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

22 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,169.8 cf Chamber Storage

3,183.7 cf Field - 1,169.8 cf Chambers = 2,013.9 cf Stone x 40.0% Voids = 805.5 cf Stone Storage

Chamber Storage + Stone Storage = 1,975.3 cf = 0.045 af Overall Storage Efficiency = 62.0% Overall System Size = 80.50' x 11.17' x 3.54'

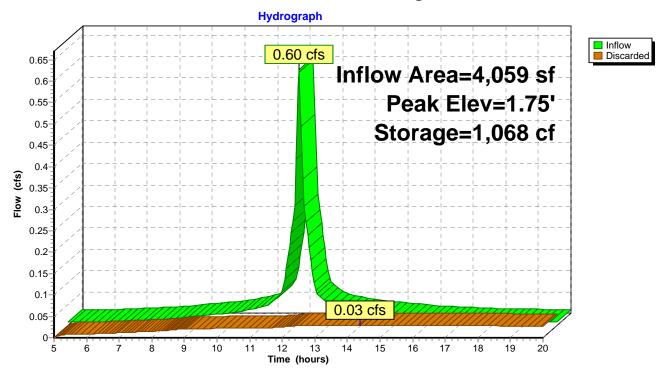
22 Chambers 117.9 cy Field 74.6 cy Stone





Page 332

Pond 6P: Roof Recharge



Type III 24-hr 100-Year Rainfall=6.70" Printed 3/9/2020

Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 333

Summary for Pond 7P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

4,929 sf,100.00% Impervious, Inflow Depth > 5.97" for 100-Year event Inflow Area =

0.73 cfs @ 12.09 hrs, Volume= Inflow 2,450 cf

0.03 cfs @ 14.84 hrs, Volume= Outflow 1,321 cf, Atten= 96%, Lag= 164.9 min

0.03 cfs @ 14.84 hrs, Volume= Discarded = 1,321 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 2.23' @ 14.84 hrs Surf.Area= 899 sf Storage= 1,386 cf

Plug-Flow detention time= 175.3 min calculated for 1,316 cf (54% of inflow)

Center-of-Mass det. time= 85.7 min (819.4 - 733.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	806 cf	11.17'W x 80.50'L x 3.54'H Field A
			$3,184 \text{ cf Overall - } 1,170 \text{ cf Embedded = } 2,014 \text{ cf } \times 40.0\% \text{ Voids}$
#2A	0.50'	1,170 cf	Cultec R-330XLHD x 22 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
	<u> </u>		—

1,975 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.03 cfs @ 14.84 hrs HW=2.23' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.03 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 334

Pond 7P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

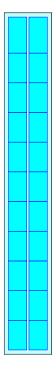
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

22 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,169.8 cf Chamber Storage

3,183.7 cf Field - 1,169.8 cf Chambers = 2,013.9 cf Stone x 40.0% Voids = 805.5 cf Stone Storage

Chamber Storage + Stone Storage = 1,975.3 cf = 0.045 af Overall Storage Efficiency = 62.0% Overall System Size = 80.50' x 11.17' x 3.54'

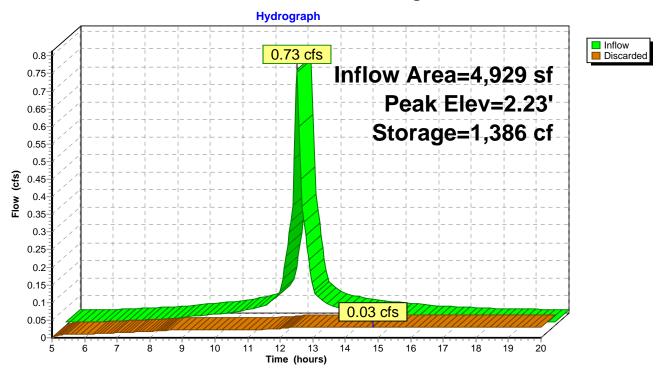
22 Chambers 117.9 cy Field 74.6 cy Stone





Page 335

Pond 7P: Roof Recharge



Type III 24-hr 100-Year Rainfall=6.70"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Page 336

Summary for Pond 8P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 5.97" for 100-Year event

Inflow = 0.18 cfs @ 12.09 hrs, Volume= 597 cf

Outflow = 0.01 cfs @ 14.07 hrs, Volume= 360 cf, Atten= 95%, Lag= 118.8 min

Discarded = 0.01 cfs @ 14.07 hrs, Volume= 360 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 2.54' @ 14.07 hrs Surf.Area= 200 sf Storage= 326 cf

Plug-Flow detention time= 182.5 min calculated for 360 cf (60% of inflow)

Center-of-Mass det. time= 101.4 min (835.1 - 733.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows
			—

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 14.07 hrs HW=2.54' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.01 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 337

Pond 8P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length

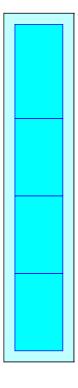
1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

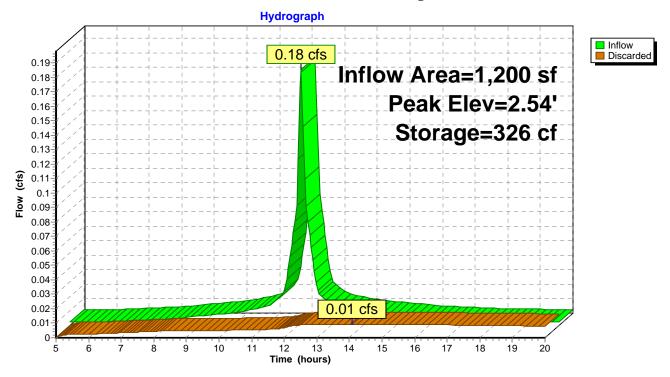
4 Chambers 26.2 cy Field 18.0 cy Stone





Page 338

Pond 8P: Roof Recharge



Type III 24-hr 100-Year Rainfall=6.70"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Page 339

Summary for Pond 9P: BASIN E

Inflow Area = 135,239 sf, 32.90% Impervious, Inflow Depth > 3.53" for 100-Year event

Inflow = 13.49 cfs @ 12.09 hrs, Volume= 39,773 cf

Outflow = 4.68 cfs @ 12.41 hrs, Volume= 24,543 cf, Atten= 65%, Lag= 19.0 min

Discarded = 0.42 cfs @ 12.41 hrs, Volume= 13,774 cf Primary = 4.27 cfs @ 12.41 hrs, Volume= 10,769 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 241.06' @ 12.41 hrs Surf.Area= 7,280 sf Storage= 17,545 cf

Plug-Flow detention time= 143.2 min calculated for 24,461 cf (62% of inflow)

Center-of-Mass det. time= 69.4 min (858.1 - 788.7)

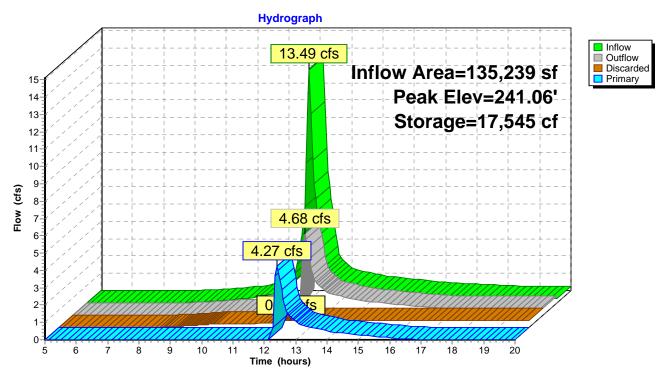
Volume	Inver	t Avai	l.Storage	Storage Description	on		
#1	237.80	237.80' 24,84		Custom Stage Da	ata (Irregular)List	ed below (Recalc)	
Elevatio (fee	-	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
			· · · · · ·	, ,			
237.8	_	3,709 3,904	322.6	0 761	0 761	3,709	
238.0		,	326.4			3,916	
239.0		4,911	345.3	4,398	5,159	4,980	
240.0	0	5,976	364.3	5,435	10,594	6,110	
241.0	0	7,097	383.1	6,528	17,122	7,289	
241.9	0	10,147	430.2	7,719	24,841	10,360	
Device	Routing	lnv	ert Outle	et Devices			
#1	Discarded	arded 237.80' 2		.410 in/hr Exfiltration over Wetted area			
#2	Primary			5.0' long x 18.0' breadth Broad-Crested Rectangular Weir			
	·		Head	d (feet) 0.20 0.40	0.60 0.80 1.00	1.20 1.40 1.60	
				` ,		63 2.64 2.64 2.63	

Discarded OutFlow Max=0.42 cfs @ 12.41 hrs HW=241.06' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.42 cfs)

Primary OutFlow Max=4.17 cfs @ 12.41 hrs HW=241.06' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 4.17 cfs @ 1.06 fps)

Page 340

Pond 9P: BASIN E



Type III 24-hr 100-Year Rainfall=6.70"

Prepared by HP Printed 3/9/2020

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 341

Summary for Pond 10P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 5.97" for 100-Year event

Inflow = 0.18 cfs @ 12.09 hrs, Volume= 597 cf

Outflow = 0.01 cfs @ 14.07 hrs, Volume= 360 cf, Atten= 95%, Lag= 118.8 min

Discarded = 0.01 cfs @ 14.07 hrs, Volume= 360 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 2.54' @ 14.07 hrs Surf.Area= 200 sf Storage= 326 cf

Plug-Flow detention time= 182.5 min calculated for 360 cf (60% of inflow)

Center-of-Mass det. time= 101.4 min (835.1 - 733.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 14.07 hrs HW=2.54' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.01 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 342

Pond 10P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length

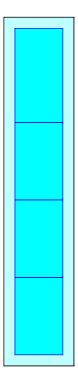
1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

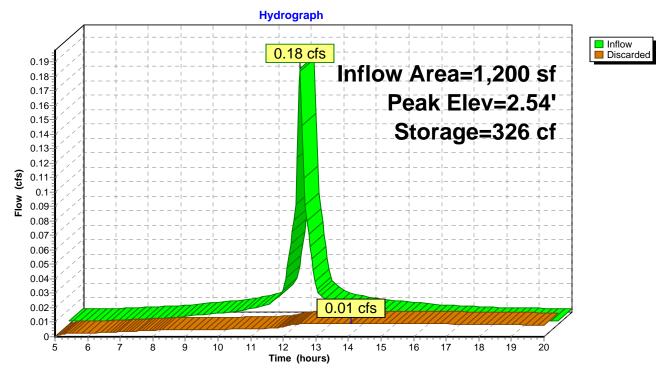
4 Chambers 26.2 cy Field 18.0 cy Stone





Page 343

Pond 10P: Roof Recharge



Prepared by HP

Type III 24-hr 100-Year Rainfall=6.70" Printed 3/9/2020

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 344

Summary for Pond 11P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 4,059 sf,100.00% Impervious, Inflow Depth > 5.97" for 100-Year event

Inflow = 0.60 cfs @ 12.09 hrs, Volume= 2,018 cf

Outflow = 0.03 cfs @ 14.37 hrs, Volume= 1,223 cf, Atten= 95%, Lag= 137.2 min

Discarded = 0.03 cfs @ 14.37 hrs, Volume= 1,223 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.75' @ 14.37 hrs Surf.Area= 899 sf Storage= 1,068 cf

Plug-Flow detention time= 169.3 min calculated for 1,219 cf (60% of inflow)

Center-of-Mass det. time= 89.6 min (823.3 - 733.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	806 cf	11.17'W x 80.50'L x 3.54'H Field A
			$3,184 \text{ cf Overall - } 1,170 \text{ cf Embedded = } 2,014 \text{ cf } \times 40.0\% \text{ Voids}$
#2A	0.50'	1,170 cf	Cultec R-330XLHD x 22 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows
	<u> </u>		

1,975 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.03 cfs @ 14.37 hrs HW=1.75' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.03 cfs)

Page 345

Pond 11P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

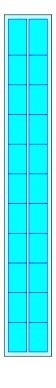
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

22 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,169.8 cf Chamber Storage

3,183.7 cf Field - 1,169.8 cf Chambers = 2,013.9 cf Stone x 40.0% Voids = 805.5 cf Stone Storage

Chamber Storage + Stone Storage = 1,975.3 cf = 0.045 af Overall Storage Efficiency = 62.0% Overall System Size = 80.50' x 11.17' x 3.54'

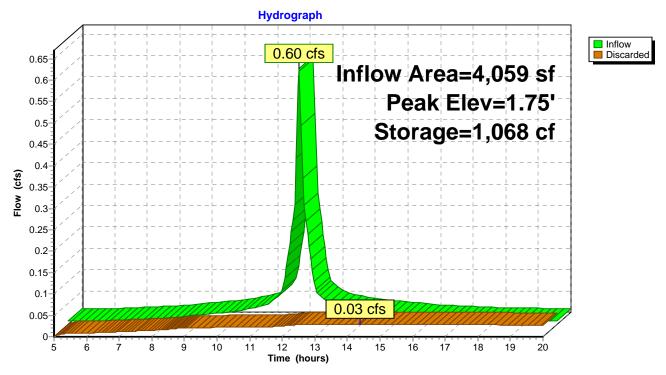
22 Chambers 117.9 cy Field 74.6 cy Stone





Page 346

Pond 11P: Roof Recharge



Type III 24-hr 100-Year Rainfall=6.70"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Page 347

Summary for Pond 13P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 1,200 sf,100.00% Impervious, Inflow Depth > 5.97" for 100-Year event

Inflow = 0.18 cfs @ 12.09 hrs, Volume= 597 cf

Outflow = 0.01 cfs @ 14.07 hrs, Volume= 360 cf, Atten= 95%, Lag= 118.8 min

Discarded = 0.01 cfs @ 14.07 hrs, Volume= 360 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 2.54' @ 14.07 hrs Surf.Area= 200 sf Storage= 326 cf

Plug-Flow detention time= 182.5 min calculated for 360 cf (60% of inflow)

Center-of-Mass det. time= 101.4 min (835.1 - 733.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	195 cf	6.33'W x 31.50'L x 3.54'H Field A
			707 cf Overall - 220 cf Embedded = 487 cf x 40.0% Voids
#2A	0.50'	220 cf	Cultec R-330XLHD x 4 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

415 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.01 cfs @ 14.07 hrs HW=2.54' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.01 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 348

Pond 13P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows

4 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 29.50' Row Length +12.0" End Stone x 2 = 31.50' Base Length

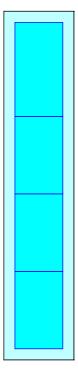
1 Rows x 52.0" Wide + 12.0" Side Stone x 2 = 6.33' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

4 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 1 Rows = 219.8 cf Chamber Storage

706.6 cf Field - 219.8 cf Chambers = 486.8 cf Stone x 40.0% Voids = 194.7 cf Stone Storage

Chamber Storage + Stone Storage = 414.5 cf = 0.010 af Overall Storage Efficiency = 58.7% Overall System Size = 31.50' x 6.33' x 3.54'

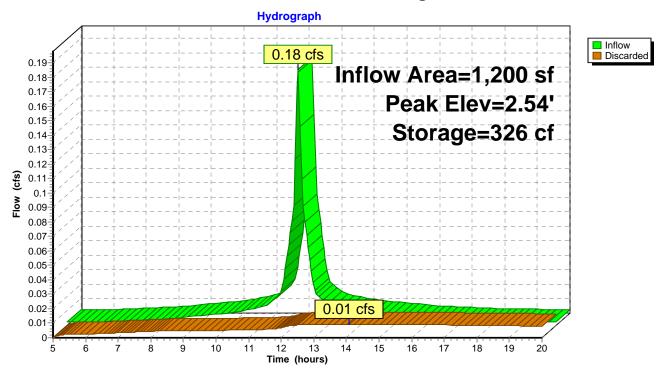
4 Chambers 26.2 cy Field 18.0 cy Stone





Page 349

Pond 13P: Roof Recharge



Type III 24-hr 100-Year Rainfall=6.70"

Prepared by HP Printed 3/9/2020

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 350

Summary for Pond 14P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 4,059 sf,100.00% Impervious, Inflow Depth > 5.97" for 100-Year event

Inflow = 0.60 cfs @ 12.09 hrs, Volume= 2,018 cf

Outflow = 0.03 cfs @ 14.76 hrs, Volume= 1,131 cf, Atten= 96%, Lag= 160.5 min

Discarded = 0.03 cfs @ 14.76 hrs, Volume= 1,131 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.90' @ 14.76 hrs Surf.Area= 840 sf Storage= 1,110 cf

Plug-Flow detention time= 170.9 min calculated for 1,131 cf (56% of inflow)

Center-of-Mass det. time= 83.7 min (817.4 - 733.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	738 cf	16.00'W x 52.50'L x 3.54'H Field A
			2,975 cf Overall - 1,129 cf Embedded = 1,846 cf x 40.0% Voids
#2A	0.50'	1,129 cf	Cultec R-330XLHD x 21 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 3 rows
	•		

1,867 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.03 cfs @ 14.76 hrs HW=1.90' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.03 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 351

Pond 14P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 3 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

7 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 50.50' Row Length +12.0" End Stone x 2 = 52.50' Base Length

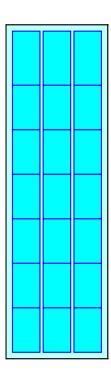
3 Rows x 52.0" Wide + 6.0" Spacing x 2 + 12.0" Side Stone x 2 = 16.00' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

21 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 3 Rows = 1,128.8 cf Chamber Storage

2,975.0 cf Field - 1,128.8 cf Chambers = 1,846.2 cf Stone x 40.0% Voids = 738.5 cf Stone Storage

Chamber Storage + Stone Storage = 1,867.3 cf = 0.043 af Overall Storage Efficiency = 62.8% Overall System Size = 52.50' x 16.00' x 3.54'

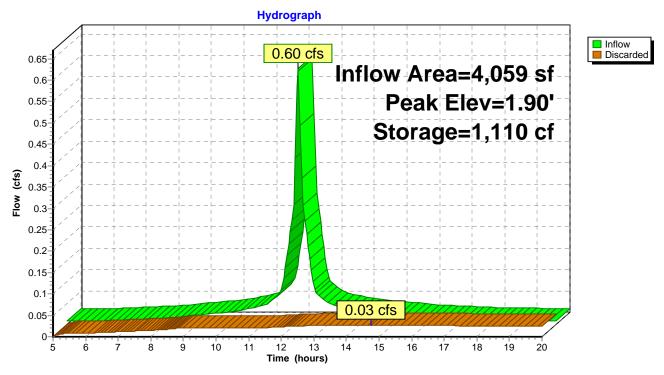
21 Chambers 110.2 cy Field 68.4 cy Stone





Page 352

Pond 14P: Roof Recharge



Type III 24-hr 100-Year Rainfall=6.70"

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Printed 3/9/2020 Page 353

Summary for Pond 17P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 13,086 sf, 37.67% Impervious, Inflow Depth > 4.90" for 100-Year event

Inflow = 1.72 cfs @ 12.09 hrs, Volume= 5,347 cf

Outflow = 0.05 cfs @ 15.92 hrs, Volume= 2,164 cf, Atten= 97%, Lag= 230.1 min

Discarded = 0.05 cfs @ 15.92 hrs, Volume= 2,164 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 3.05' @ 15.92 hrs Surf.Area= 1,677 sf Storage= 3,450 cf

Plug-Flow detention time= 187.7 min calculated for 2,157 cf (40% of inflow)

Center-of-Mass det. time= 90.2 min (851.1 - 760.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,440 cf	20.83'W x 80.50'L x 3.54'H Field A
			5,940 cf Overall - 2,340 cf Embedded = 3,600 cf x 40.0% Voids
#2A	0.50'	2,340 cf	Cultec R-330XLHD x 44 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

3,780 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.05 cfs @ 15.92 hrs HW=3.05' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.05 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 354

Pond 17P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

11 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 78.50' Row Length +12.0" End Stone x 2 = 80.50' Base Length

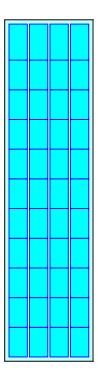
4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

44 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 2,339.6 cf Chamber Storage

5,939.7 cf Field - 2,339.6 cf Chambers = 3,600.1 cf Stone x 40.0% Voids = 1,440.0 cf Stone Storage

Chamber Storage + Stone Storage = 3,779.6 cf = 0.087 af Overall Storage Efficiency = 63.6% Overall System Size = 80.50' x 20.83' x 3.54'

44 Chambers 220.0 cy Field 133.3 cy Stone

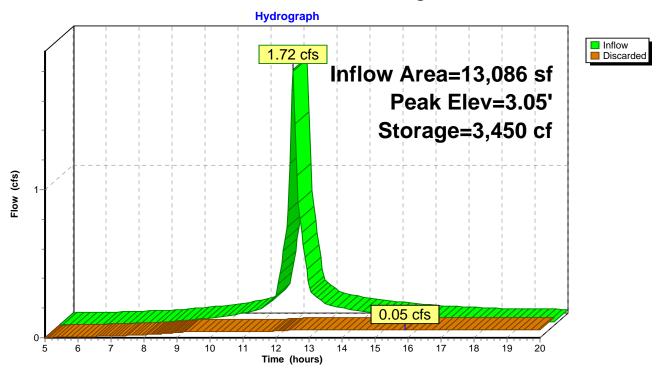




Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 355

Pond 17P: Roof Recharge



Type III 24-hr 100-Year Rainfall=6.70"

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC
Page 356

Summary for Pond 19P: Roof Recharge

[82] Warning: Early inflow requires earlier time span

Inflow Area = 11,720 sf, 42.06% Impervious, Inflow Depth > 5.01" for 100-Year event

Inflow = 1.57 cfs @ 12.09 hrs, Volume= 4,894 cf

Outflow = 0.10 cfs @ 13.81 hrs, Volume= 3,483 cf, Atten= 94%, Lag= 103.3 min

Discarded = 0.10 cfs @ 13.81 hrs, Volume= 3,483 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 3.13' @ 13.81 hrs Surf.Area= 1,176 sf Storage= 2,430 cf

Plug-Flow detention time= 174.0 min calculated for 3,471 cf (71% of inflow)

Center-of-Mass det. time= 109.5 min (868.0 - 758.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,027 cf	16.00'W x 73.50'L x 3.54'H Field A
			4,165 cf Overall - 1,598 cf Embedded = 2,567 cf x 40.0% Voids
#2A	0.50'	1,598 cf	Cultec R-330XLHD x 30 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 3 rows

2,625 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	2.410 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.10 cfs @ 13.81 hrs HW=3.13' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.10 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 357

Pond 19P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 3 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

10 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 71.50' Row Length +12.0" End Stone x 2 = 73.50' Base Length

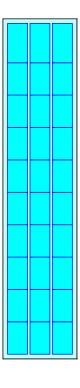
3 Rows x 52.0" Wide + 6.0" Spacing x 2 + 12.0" Side Stone x 2 = 16.00' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

30 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 3 Rows = 1,598.2 cf Chamber Storage

4,165.0 cf Field - 1,598.2 cf Chambers = 2,566.8 cf Stone x 40.0% Voids = 1,026.7 cf Stone Storage

Chamber Storage + Stone Storage = 2,624.9 cf = 0.060 af Overall Storage Efficiency = 63.0% Overall System Size = 73.50' x 16.00' x 3.54'

30 Chambers 154.3 cy Field 95.1 cy Stone

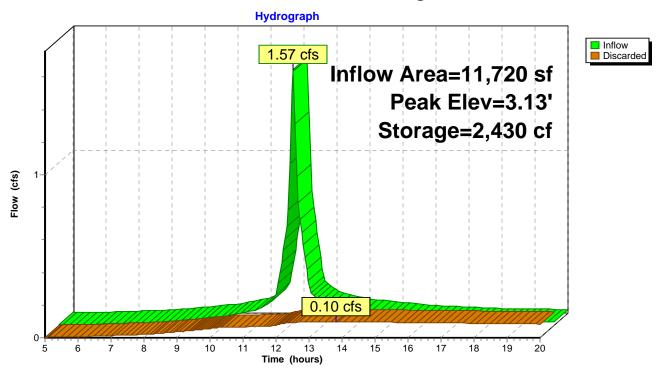




Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 358

Pond 19P: Roof Recharge



Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC Printed 3/9/2020

Page 359

Summary for Pond 21P: BASIN B

Inflow Area = 167,695 sf, 37.24% Impervious, Inflow Depth > 2.86" for 100-Year event Inflow = 10.67 cfs @ 12.17 hrs, Volume= 39,906 cf

Outflow = 4.98 cfs @ 12.51 hrs, Volume= 25,812 cf, Atten= 53%, Lag= 20.1 min Discarded = 0.40 cfs @ 12.51 hrs, Volume= 13,015 cf

Primary = 4.58 cfs @ 12.51 hrs, Volume= 12,797 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 228.07' @ 12.51 hrs Surf.Area= 7,611 sf Storage= 16,003 cf

Plug-Flow detention time= 132.5 min calculated for 25,812 cf (65% of inflow) Center-of-Mass det. time= 58.8 min (863.6 - 804.8)

Volume	Inver	t Avail	.Storage	Storage Description	on		
#1	225.50)' 2	23,381 cf	Custom Stage Da	ata (Irregular)Liste	ed below (Recalc)	
Elevatio (fee		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
225.5	0	5,039	294.3	0	0	5,039	
226.0	0	5,488	303.7	2,631	2,631	5,511	
227.0	0	6,427	322.5	5,951	8,582	6,498	
228.0	0	7,423	328.3	6,919	15,501	6,941	
228.9	0	10,160	388.5	7,880	23,381	10,390	
Device #1 #2	Routing Discarded Primary		50' 2.41	et Devices 0 in/hr Exfiltration ' long x 21.0' brea		a ed Rectangular Weir	
=			Head	d (feet) 0.20 0.40	0.60 0.80 1.00		

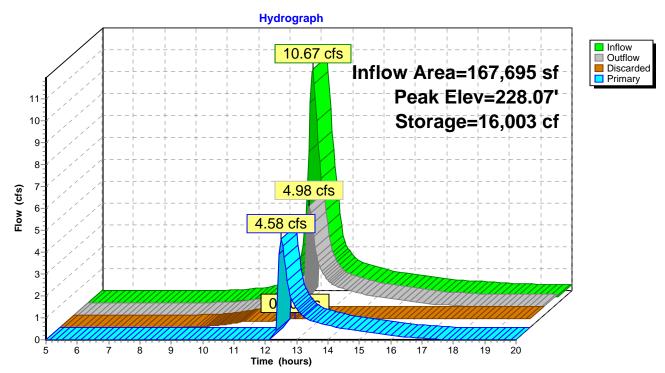
Discarded OutFlow Max=0.40 cfs @ 12.51 hrs HW=228.07' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.40 cfs)

Primary OutFlow Max=4.49 cfs @ 12.51 hrs HW=228.07' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 4.49 cfs @ 1.09 fps)

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 360

Pond 21P: BASIN B



Type III 24-hr 100-Year Rainfall=6.70"

Prepared by HP Printed 3/9/2020

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 361

Summary for Pond 22P: Roof Recharge

Inflow Area = 11,373 sf, 43.34% Impervious, Inflow Depth > 3.94" for 100-Year event

Inflow = 1.26 cfs @ 12.09 hrs, Volume= 3,734 cf

Outflow = 0.04 cfs @ 15.86 hrs, Volume= 1,549 cf, Atten= 96%, Lag= 226.0 min

Discarded = 0.04 cfs @ 15.86 hrs, Volume= 1,549 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 3.35' @ 15.86 hrs Surf.Area= 1,133 sf Storage= 2,407 cf

Plug-Flow detention time= 203.9 min calculated for 1,549 cf (41% of inflow)

Center-of-Mass det. time= 114.6 min (895.6 - 781.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	0.00'	1,013 cf	11.17'W x 101.50'L x 3.54'H Field A
			4,014 cf Overall - 1,483 cf Embedded = 2,531 cf x 40.0% Voids
#2A	0.50'	1,483 cf	Cultec R-330XLHD x 28 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

2,495 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	1.020 in/hr Exfiltration over Wetted area

Discarded OutFlow Max=0.04 cfs @ 15.86 hrs HW=3.35' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.04 cfs)

Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 362

Pond 22P: Roof Recharge - Chamber Wizard Field A

Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 2 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

14 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 99.50' Row Length +12.0" End Stone x 2 = 101.50' Base Length

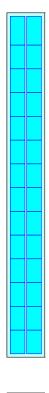
2 Rows x 52.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 11.17' Base Width 6.0" Base + 30.5" Chamber Height + 6.0" Cover = 3.54' Field Height

28 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 2 Rows = 1,482.7 cf Chamber Storage

4,014.2 cf Field - 1,482.7 cf Chambers = 2,531.4 cf Stone x 40.0% Voids = 1,012.6 cf Stone Storage

Chamber Storage + Stone Storage = 2,495.3 cf = 0.057 af Overall Storage Efficiency = 62.2% Overall System Size = 101.50' x 11.17' x 3.54'

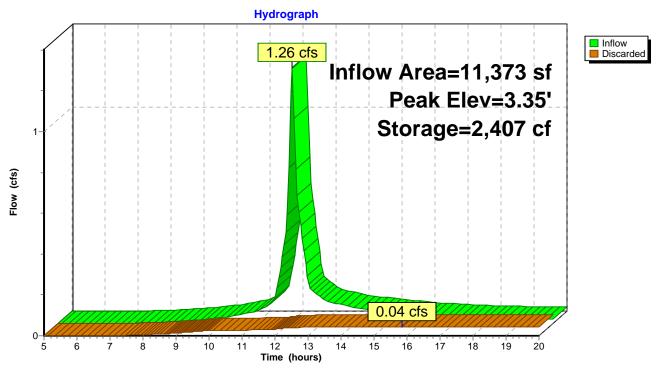
28 Chambers 148.7 cy Field 93.8 cy Stone



Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 363

Pond 22P: Roof Recharge



Prepared by HP

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 364

Summary for Pond 30P: BASIN A

[82] Warning: Early inflow requires earlier time span

Inflow Area = 18,459 sf, 78.24% Impervious, Inflow Depth > 5.60" for 100-Year event Inflow 3.08 cfs @ 12.00 hrs, Volume= 8,616 cf 0.30 cfs @ 12.65 hrs, Volume= 7,549 cf, Atten= 90%, Lag= 39.1 min Outflow 0.22 cfs @ 12.65 hrs, Volume= Discarded = 7,462 cf 0.07 cfs @ 12.65 hrs, Volume= Primary 87 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 240.01' @ 12.65 hrs Surf.Area= 3,012 sf Storage= 3,942 cf

Plug-Flow detention time= 161.1 min calculated for 7,522 cf (87% of inflow)

Center-of-Mass det. time= 121.4 min (859.4 - 738.0)

Volume	Invert	Avail.	Storage	Storage Description	on						
#1	238.20'		7,401 cf	Custom Stage Data (Irregular)Listed below (Recalc)							
Elevation (fee		urf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)					
238.2	20	1,411	262.1	0	0	1,411					
239.0	00	2,083	297.8	1,389	1,389	3,017					
240.0	00	3,005	316.6	2,530	3,919	3,986					
241.0	00	3,983	335.5	3,483	7,401	5,020					
Device	Routing	Inv	ert Outle	et Devices							
#1	Discarded	238.2	20' 2.41	0 in/hr Exfiltration	over Wetted are	a					
#2 Primary 240.00' 25.0' long x 21.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.63											

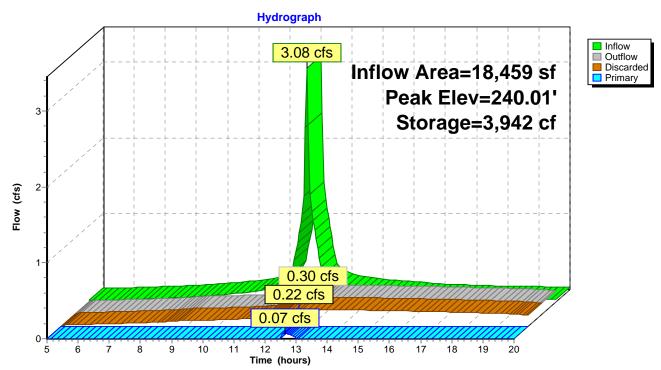
Discarded OutFlow Max=0.22 cfs @ 12.65 hrs HW=240.01' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.22 cfs)

Primary OutFlow Max=0.05 cfs @ 12.65 hrs HW=240.01' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 0.05 cfs @ 0.24 fps)

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 365

Pond 30P: BASIN A



Prepared by HP

Printed 3/9/2020

HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 366

Summary for Pond 31P: BASIN D

Inflow Area = 90,014 sf, 45.84% Impervious, Inflow Depth > 4.35" for 100-Year event 10.80 cfs @ 12.09 hrs, Volume= 32,604 cf
Outflow = 2.13 cfs @ 12.54 hrs, Volume= 26,961 cf, Atten= 80%, Lag= 27.0 min 24,595 cf
Primary = 1.38 cfs @ 12.54 hrs, Volume= 2,366 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 240.98' @ 12.54 hrs Surf.Area= 9,195 sf Storage= 14,812 cf

Plug-Flow detention time= 165.6 min calculated for 26,871 cf (82% of inflow) Center-of-Mass det. time= 117.6 min (890.6 - 772.9)

Volume	Inver	t Avail.	Storage	Storage Description	on		
#1	239.00	' 24	4,822 cf	Custom Stage Da	ata (Irregular)List	ed below (Recalc)	
Elevatio	_	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
239.0	00	5,028	276.6	0	0	5,028	
240.0	00	7,979	406.4	6,447	6,447	12,091	
241.0	00	9,227	425.3	8,595	15,042	13,409	
241.9	90	12,593	472.4	9,780	24,822	16,798	
Device	Routing	Inve	ert Outle	et Devices			
#1	Discarded	239.0	00' 2.41	0 in/hr Exfiltration	over Wetted are	a	
#2	Primary	240.9	Head	d (feet) 0.20 0.40	0.60 0.80 1.00	ed Rectangular Wei 1.20 1.40 1.60 63 2.64 2.64 2.63	r

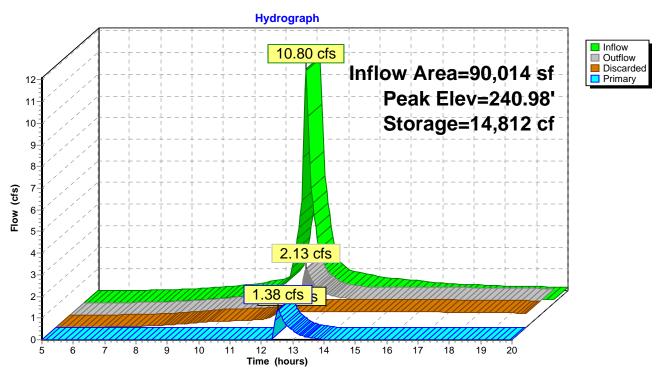
Discarded OutFlow Max=0.75 cfs @ 12.54 hrs HW=240.97' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.75 cfs)

Primary OutFlow Max=1.36 cfs @ 12.54 hrs HW=240.97' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 1.36 cfs @ 0.73 fps)

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 367

Pond 31P: BASIN D



Printed 3/9/2020 Prepared by HP HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 368

Summary for Pond 32P: BASIN C

Inflow Area = 188,274 sf, 48.56% Impervious, Inflow Depth > 3.61" for 100-Year event Inflow 16.60 cfs @ 12.14 hrs. Volume= 56.653 cf 5.91 cfs @ 12.50 hrs, Volume= Outflow 50,852 cf, Atten= 64%, Lag= 21.7 min 1.72 cfs @ 12.50 hrs, Volume= 44,037 cf Discarded = 6,815 cf Primary = 4.19 cfs @ 12.50 hrs, Volume=

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 226.06' @ 12.50 hrs Surf.Area= 15,920 sf Storage= 23,166 cf

Plug-Flow detention time= 133.0 min calculated for 50,683 cf (89% of inflow) Center-of-Mass det. time= 100.3 min (890.0 - 789.7)

<u>Volume</u>	Inver	t Avail.:	Storage	Storage Description	on				
#1	224.00	' 38	3,476 cf	Custom Stage Da	ata (Irregular)Liste	ed below (Recalc)			
Elevatio	_	urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area			
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)			
224.0	00	4,753	268.4	0	0	4,753			
225.0	00	12,400	506.3	8,277	8,277	19,424			
226.0	00	15,628	628.5	13,983	22,260	30,474			
226.9	90	20,519	675.7	16,216	38,476	35,408			
Device	Routing	Inve	ert Outle	et Devices					
#1	Discarded	224.0	0' 2.41	0 in/hr Exfiltration	over Wetted are	a			
#2 Primary 225.90' 25.0' long x 21.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.63									

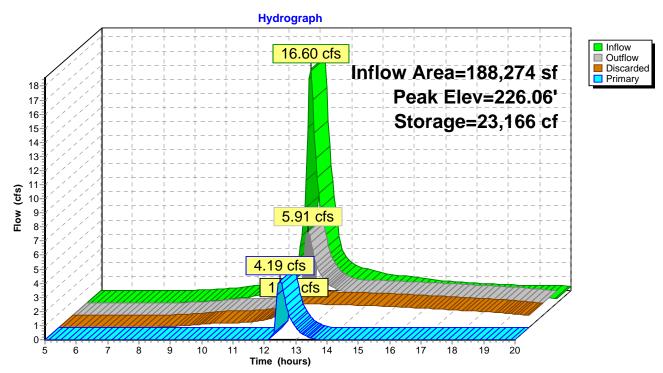
Discarded OutFlow Max=1.72 cfs @ 12.50 hrs HW=226.06' (Free Discharge) 1=Exfiltration (Exfiltration Controls 1.72 cfs)

Primary OutFlow Max=4.17 cfs @ 12.50 hrs HW=226.06' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 4.17 cfs @ 1.06 fps)

Prepared by HP
HydroCAD® 10.00-24 s/n 09450 © 2018 HydroCAD Software Solutions LLC

Page 369

Pond 32P: BASIN C



APPENDIX B: HYDRAULIC ANALYSIS

PIPE SIZING CALCULATIONS



220

100

Project No.: 2016-002 Location: Joanna Hills Estates, Avon, MA Calculated By: Justin Williams Calculated On: 10/27/2020

DESIGN STORM	<u>K</u>	<u>b</u>	Steel Formula: I=K/(tc+b)
2	70	13	Region 4 Design Storm 25 K= 170
10	111	16	b= 27
25	170	27	
50	187	24	

28

COVER	С
WOODED	0.20
LANDSCAPED	0.40
IMPERVIOUS	0.85

									Jo	anna	a Hills	s Estates - 2	25 YE	AR S	TORI	M										
			WATERS	HED CH	ARACTE	RISTICS										PIPE CHA	RACTERIS	TICS					FLOW CHARA		CTERISTIC	S
	LOCATION			-	AND US			W TIME		FLO			1	1	1			_	vetted perin			\/C				Тс
Description	Cover	Increm. (ACRE)	Total_A (ACRE)	С	CA	Total CA	To Inlet (MIN)	In Pipe (MIN)	Tc (MIN)	(IPH)	Q (CFS)	Structure	Invert	Pipe	Size (IN)	Length (FT)	Area (SF)	R (FT)	Slope	n	Qf (CFS)	Vf (FT/S)	Q/Qf	V/Vf	V (FT/S)	L/V (MIN)
WS CB-1	LANDSCAPED IMPERVIOUS	0.296 0.338	0.634	0.400 0.850 0.640	0.406		6.00	NONE	6.00	5.15		rom: CB-1	Out: In:	ADS N-12	12	23	0.79	0.250	0.005	0.012	2.73	3.47	0.77	0.97	3.37	0.11
WS CB-2	LANDSCAPED IMPERVIOUS	0.297 0.256	0.554	0.400 0.850 0.608	0.337		6.00	NONE	6.00	5.15		rom: CB-2	Out: In:	ADS N-12	12	15	0.79	0.250	0.005	0.012	2.73	3.47	0.64	0.92	3.19	0.08
DMH-1	TO WQU-1		0.334	0.000	0.557	0.743	6.00			5.13	3.81	rom: DMH-1 fo: WQU-1	Out:	ADS N-12	15	19	1.23	0.313	0.005	0.012	4.95	4.03	0.77	0.97	3.91	0.08
WQU-1	TO FLARED END-1					0.743	6.11	0.08	6.19	5.12	3.80	rom: WQU-1	Out: 1 In:	ADS N-12	15	162	1.23	0.3125	0.005	0.012	4.95	4.03	0.77	0.97	3.91	0.69
WS CB-3	LANDSCAPED IMPERVIOUS	0.000 0.138	0.138	0.400 0.850 0.850	0.117		6.00	NONE	6.00	5.15		rom: CB-3	Out: In:	HDPE	12	25	0.79	0.250	0.005	0.013	2.52	3.21	0.24	0.69	2.22	0.19
WS CB-4	LANDSCAPED IMPERVIOUS	0.000 0.061	0.061	0.400 0.850 0.850	0.052		6.00	NONE	6.00	5.15		rom: CB-4	Out: In:	HDPE	12	18	0.79	0.250	0.005	0.013	2.52	3.21	0.11	0.55	1.75	0.17
WS CB-5	LANDSCAPED IMPERVIOUS	0.000 0.041		0.400 0.850 0.850	0.035		6.00	NONE	6.00	5.15		rom: CB-5	Out: In:	HDPE	12	26	0.79	0.250	0.005	0.013	2.52	3.21	0.07	0.49	1.56	0.28
DMH-2	TO WQU-2					0.204	6.00	0.28	6.28	5.11	1.04	rom: DMH-2 o: WQU-2	Out: In:	HDPE	12	28	0.79	0.250	0.008	0.013	3.19	4.06	0.33	0.76	3.07	0.15
WQU-2	TO FLARED END-2					0.204	6.28	0.15	6.43	5.09	1.04	rom: WQU-2	Out: 2 In:	HDPE	12	36	0.79	0.250	0.005	0.013	2.52	3.21	0.41	0.81	2.60	0.23

Project No.: 2016-002 Location: Joanna Hills Estates, Avon, MA Calculated By: Justin Williams Calculated On: 10/27/2020

	Joanna Hills Estates - 25 YEAR STORM WATERSHED CHARACTERISTICS PIPE CHARACTERISTICS																									
			WATERSI								PIPE CHARACTERISTICS										FLOW CHARACTERISTICS			S		
Description	LOCATION Cover	Increm. T	otal_A	C	CA	Total CA	To Inlet (MIN)	OW TIME In Pipe (MIN)	Tc (MIN)	T	Q (CFS)	Structure	Invert	Pipe	Size (IN)	R = hy	Area (SF)	us = area/v R (FT)	Slope	neter n	Qf (CFS)	Vf (FT/S)	Q/Qf	V/Vf	V (FT/S)	Tc L/V (MIN)
WS CB-6	LANDSCAPED IMPERVIOUS	0.383 0.314	AOIL)	0.400 0.850			(MINA)	(MINV)	(WIIIV)	<u> </u>		n: CB-6	Out:	HDPE	12	12	0.79	0.250	0.005	0.013	2.52	3.21	0.86	1.00	3.21	0.06
			0.697	0.603	0.420		6.00	NONE	6.00	5.15		DMH-3	ln:	TIDI L	12	12	0.73	0.200	0.000	0.010	2.02	0.21	0.00	1.00	0.21	0.00
WS CB-7	LANDSCAPED IMPERVIOUS	0.099 0.183	0.282	0.400 0.850 0.692	0.195		6.00	NONE	6.00) 5.15		n: CB-7 DMH-3	Out: In:	HDPE	12	12	0.79	0.250	0.005	0.013	2.52	3.21	0.40	0.80	2.57	0.08
DMH-3	TO DMH-4					0.615	6.00	0.08	6.08	3 5.14	Fror 3.16	n: DMH-3	Out: In:	HDPE	12	210	0.79	0.250	0.020	0.013	5.04	6.42	0.63	0.91	5.87	0.60
DMH-4	TO WQU-3					0.615	6.08	0.60	6.67	' 5.05	Fror 3.10	n: DMH-4 WQU-3	Out:	HDPE	12	58	0.79	0.250	0.016	0.013	4.51	5.74	0.69	0.94	5.39	0.18
WS CB-8	LANDSCAPED IMPERVIOUS	0.456 0.443	0.000	0.400 0.850	0.550		6.00	NONE	6.00) F 15		n: CB-8	Out:	ADS N-12	15	13	1.23	0.313	0.005	0.012	4.95	4.03	0.58	0.90	3.61	0.06
WS CB-9	LANDSCAPED IMPERVIOUS	0.450 0.323	0.899	0.400 0.850			6.00	NONE		5.15	Fror	DMH-5 n: CB-9	In: Out:	ADS N-12	12	13	0.79	0.250	0.005	0.012	2.73	3.47	0.86	1.00	3.48	0.06
DMH-5	TO WQU-3		0.773	0.588	0.455	1.014	6.00	NONE 0.06		5.15		DMH-5 n: DMH-5 WQU-3	In: Out: In:	ADS N-12	18	26	1.77	0.375	0.005	0.012	8.05	4.55	0.65	0.92	4.20	0.10
WQU-3	TO FLARED END-3					1.629	6.67	0.18	6.85	5 5.02	8.18	n: WQU-3 FLARED END-	Out: -3 In:	RCP	21	46	2.41	0.4375	0.005	0.011	13.24	5.51	0.62	0.91	5.01	0.15
WS CB-10	LANDSCAPED IMPERVIOUS	0.044 0.238	0.282	0.400 0.850 0.780	0.220		6.00	NONE	6.00) 5.15	Fror 1.13 To:	n: CB-10 DMH-6	Out: In:	HDPE	12	11	0.79	0.250	0.025	0.013	5.63	7.17	0.20	0.66	4.72	0.04
WS CB-11	LANDSCAPED IMPERVIOUS	0.025 0.047	0.072	0.400 0.850 0.694	0.050		6.00	NONE	6.00) 5.15		n: CB-11 DMH-6	Out: In:	HDPE	12	14	0.79	0.250	0.025	0.013	5.63	7.17	0.05	0.43	3.07	0.08
DMH-6	TO DMH-7					0.270	6.00	0.08	6.08	3 5.14	1.39 From	n: DMH-6 DMH-7	Out: In:	HDPE	12	282	0.79	0.250	0.025	0.013	5.63	7.17	0.25	0.70	5.00	0.94
WS CB-12	LANDSCAPED IMPERVIOUS	0.060 0.158	0.218	0.400 0.850 0.727	0.158		6.00	NONE	6 00) 5 15		n: CB-12 DMH-7	Out:	HDPE	12	20	0.79	0.250	0.005	0.013	2.52	3.21	0.32	0.76	2.42	0.14
DMH-7	TO DMH-8		5.210	0.121	3.100	0.428	6.08				Fror 2.14	n: DMH-7 DMH-8	Out:	HDPE	12	34	0.79	0.250	0.020	0.013	5.04	6.42	0.42	0.82	5.24	0.11

Project No.: 2016-002 Location: Joanna Hills Estates, Avon, MA Calculated By: Justin Williams Calculated On: 10/27/2020

									JO	anna	а піі	15 ESta	ates - 25	ILA	N 31	OKI	/1										
			WATERS															RACTERIST						FLO	OW CHARAC	CTERISTIC	
	LOCATION				AND US			W TIME		FL	OW					<u>. </u>				etted perim	eter		\/C			.,	Тс
Description	Cover	Increm. (ACRE)	Total_A (ACRE)	С	CA	Total CA	To Inlet (MIN)	In Pipe (MIN)	Tc (MIN)	(IPH)	Q (CFS)		ructure	Invert	Pipe	(IN)	Length (FT)	Area (SF)	R (FT)	Slope	n	Qf (CFS)	Vf (FT/S)	Q/Qf	V/Vf	V (FT/S)	L/V (MIN)
WS CB-13	LANDSCAPED IMPERVIOUS	0.137 0.362		0.400 0.850								From: CB	3-13 C	Out:	HDPE	12	4	0.79	0.250	0.020	0.013	5.04	6.42	0.37	0.79	5.04	0.01
WS CB-14	LANDSCAPED	0.087	0.499	0.727	0.363		6.00	NONE	6.00	5.15	1.87	To: DM From: CB	MH-8 Ir	n: Out:													
WO 0B-14	IMPERVIOUS	0.163	0.250	0.850	0.173		6.00	NONE	6.00	5.15	0.89		л-1 -1 С ЛН-8 Ir		HDPE	12	15	0.79	0.250	0.010	0.013	3.56	4.54	0.25	0.70	3.18	0.08
DMH-8	TO DMH-9					0.964	7.02	0.11	7.12	4.98	4.80	From: DM	ЛН-8 C	Out:	HDPE	12	56	0.79	0.250	0.025	0.013	5.63	7.17	0.85	1.00	7.17	0.13
												To: DM	1H-9 Ir														
DMH-9	TO DMH-10					0.964	7.12	0.13	7.25	4.96	4.79	From: DM			HDPE	15	127	1.23	0.3125	0.015	0.013	7.91	6.45	0.60	0.90	5.83	0.36
												To: DN	/IH-10 Ir	า:													
WS CB-15	LANDSCAPED IMPERVIOUS	0.245 0.067	0.040	0.400 0.850	0.455		40.40	NONE				From: CB			RCP	12	25	0.79	0.250	0.005	0.011	2.98	3.79	0.24	0.69	2.61	0.16
			0.312	0.497	0.155		10.40	NONE	10.40	4.55	0.70	TO: DN	/IH-10 Ir	1: 													
DMH-10	TO DMH-11					1.119	10.40	0.36	10.76	4.50	5.04	From: DM To: DM	ИН-10 С ИН-11 Ir		RCP	15	62	1.23	0.3125	0.012	0.011	8.36	6.81	0.60	0.90	6.16	0.17
WS CB-16	LANDSCAPED IMPERVIOUS	0.694 0.206		0.400 0.850								From: CB			RCP	12	91	0.79	0.250	0.005	0.011	2.98	3.79	0.78	0.98	3.70	0.41
WS CB-17	LANDSCAPED	0.582	0.901	0.400	0.453		6.00	NONE	6.00	5.15	2.33	To: DM From: CB		Out:													
	IMPERVIOUS	0.498	1.081	0.850 0.607	0.656		10.40	NONE	10.40	4.55	2.98	To: DM	/IH-11 Ir		RCP	15	22	1.23	0.313	0.005	0.011	5.40	4.40	0.55	0.88	3.88	0.09
DMH-11	TO WQU-4					2.229	10.76	0.41	11.17	4.45	9.93	From: DM			RCP	21	18	2.41	0.438	0.005	0.011	13.24	5.51	0.75	0.96	5.30	0.06
WQU-4	TO FLARED END-4					2.229	11.17	0.06	5 11.23	4.45	9.91	From: W(QU-4 C		RCP	21	31	2.41	0.438	0.005	0.011	13.24	5.51	0.75	0.96	5.30	0.10
WS CB-18	LANDSCAPED IMPERVIOUS	0.852 0.497	1.349	0.400 0.850 0.566	0.764	_	6.00	NONE	6.00	5.15	3.93	From: CB			ADS N-12	15	10	1.23	0.313	0.005	0.012	4.95	4.03	0.79	0.98	3.95	0.04
WQU-5	TO FLARED END-5					0.764	6.00	0.04	6.04	5.14	3.93		QU-5 C		ADS N-12	15	126	1.23	0.313	0.005	0.012	4.95	4.03	0.79	0.98	3.95	0.53

APPENDIX C: GROUNDWATER RECHARGE CALCULATIONS & 72-HR DRAWDOWN CALCULATIONS

Groundwater Recharge Calculations - Basin A

Project No. 2016-002

Project: Joanna Hills Estates **Location:** Joanna Road, Avon, MA

Date: March 6, 2020

Calculate the Required Recharge Volume

NRCS Hydrologic Soil Group	Volume to Recharge (inches)	Impervious Area (square feet)	Required Recharge Volume (cubic feet)
Α	0.60	7566	378
В	0.35		0
С	0.25		0
D	0.10	2893	24
		Total Required Recharge Volume	402

Capture Area Adjustment

A minimum of 65% of the total site impervious area must be directed to a recharge BMP:

· R	ous Area Directed to echarge BMP (square feet)	Total Impervious Area (square feet)	% of Total Directed to Recharge BMP
	10459	10459	100%

Calculate the Adjustment Factor:

Impervious Area Directed to Recharge BMP (square feet)	Total Impervious Area (square feet)	Ratio of Total Impervious Area to Impervious Area Directed to BMP		
10459	10459	1.00		

Required Recharge Volume (cubic feet)	Ratio of Total Impervious Area to Impervious Area Directed to BMP	Adjusted Required Recharge Volume (cubic feet)
402	1.00	402
	Total Required Recharge Volume	402
	Total Provided Recharge Volume (EL. 238.47 in Basin)	403

Groundwater Recharge Calculations - Basin B

Project No. 2016-002

Project: Joanna Hills Estates **Location:** Joanna Road, Avon, MA

Date: March 6, 2020

Calculate the Required Recharge Volume

NRCS Hydrologic Soil Group	Volume to Recharge (inches)	Impervious Area (square feet)	Required Recharge Volume (cubic feet)
Α	0.60	45015	2251
В	0.35		0
С	0.25		0
D	0.10	10020	84
		Total Required Recharge Volume	2334

Capture Area Adjustment

A minimum of 65% of the total site impervious area must be directed to a recharge BMP:

Impervious Area Directed to Recharge BMP (square feet)	Total Impervious Area (square feet)	% of Total Directed to Recharge BMP
55035	55035	100%

Calculate the Adjustment Factor:

Impervious Area Directed to Recharge BMP (square feet)	Total Impervious Area (square feet)	Ratio of Total Impervious Area to Impervious Area Directed to BMP		
55035	55035	1.00		

Required Recharge Volume (cubic feet)	Ratio of Total Impervious Area to Impervious Area Directed to BMP	Adjusted Required Recharge Volume (cubic feet)
2334	1.00	2334
	Total Required Recharge Volume	2334
	Total Provided Recharge Volume (EL. 225.95 in Basin)	-23/10

Groundwater Recharge Calculations - Basin C

Project No. 2016-002

Project: Joanna Hills Estates **Location:** Joanna Road, Avon, MA

Date: March 6, 2020

Calculate the Required Recharge Volume

NRCS Hydrologic Soil Group	Volume to Recharge (inches)	Impervious Area (square feet)	Required Recharge Volume (cubic feet)
Α	0.60	38690	1935
В	0.35	34912	1018
С	0.25		0
D	0.10	2200	18
		Total Required Recharge Volume	2971

Capture Area Adjustment

A minimum of 65% of the total site impervious area must be directed to a recharge BMP:

Impervious Area Directed to Recharge BMP (square feet)	Total Impervious Area (square feet)	% of Total Directed to Recharge BMP		
74130	75803	98%		

Calculate the Adjustment Factor:

Impervious Area Directed to Recharge BMP (square feet)	Total Impervious Area (square feet)	Ratio of Total Impervious Area to Impervious Area Directed to BMP		
74130	75803	1.02		

Required Recharge Volume (cubic feet)	Ratio of Total Impervious Area to Impervious Area Directed to BMP	Adjusted Required Recharge Volume (cubic feet)
2971	1.02	3038
	Total Required Recharge Volume	3038
	Total Provided Recharge Volume (EL.224.49 in Basin)	3065

Groundwater Recharge Calculations - Basin D

Project No. 2016-002

Project: Joanna Hills Estates **Location:** Joanna Road, Avon, MA

Date: March 6, 2020

Calculate the Required Recharge Volume

NRCS Hydrologic Soil Group	Volume to Recharge (inches)	Impervious Area (square feet)	Required Recharge Volume (cubic feet)
Α	0.60	7532	377
В	0.35		0
С	0.25	6345	132
D	0.10	13151	110
		Total Required Recharge Volume	618

Capture Area Adjustment

A minimum of 65% of the total site impervious area must be directed to a recharge BMP:

Impervious Area Directed to Recharge BMP (square feet)	Total Impervious Area (square feet)	% of Total Directed to Recharge BMP
27028	27028	100%

Calculate the Adjustment Factor:

Impervious Area Directed to Recharge BMP (square feet)	Total Impervious Area (square feet)	Ratio of Total Impervious Area to Impervious Area Directed to BMP	
27028	27028	1.00	

Required Recharge Volume (cubic feet)	Ratio of Total Impervious Area to Impervious Area Directed to BMP	Adjusted Required Recharge Volume (cubic feet)
618	1.00	618
	Total Required Recharge Volume	618
	Total Provided Recharge Volume (EL. 239.12 in Basin)	626

Groundwater Recharge Calculations - Basin E

Project No. 2016-002

Project: Joanna Hills Estates **Location:** Joanna Road, Avon, MA

Date: March 6, 2020

Calculate the Required Recharge Volume

NRCS Hydrologic Soil Group	Volume to Recharge (inches)	Impervious Area (square feet)	Required Recharge Volume (cubic feet)
Α	0.60	22353	1118
В	0.35		0
С	0.25	1116	23
D	0.10		0
		Total Required Recharge Volume	1141

Capture Area Adjustment

A minimum of 65% of the total site impervious area must be directed to a recharge BMP:

Impervious Area Directed to Recharge BMP (square feet)	Total Site Impervious Area (square feet)	% of Total Site Directed to Recharge BMP
23469	23469	100%

Calculate the Adjustment Factor:

Impervious Area Directed to Recharge BMP (square feet)	Total Site Impervious Area (square feet)	Ratio of Total Impervious Area to Impervious Area Directed to BMP
23469	23469	1.00

Required Recharge Volume (cubic feet)	Ratio of Total Impervious Area to Impervious Area Directed to BMP	Adjusted Required Recharge Volume (cubic feet)
1141	1.00	1141
	Total Required Recharge Volume	1141
	Total Provided Recharge Volume (EL. 238.10)	1141

72 Hour Drawdown Calculations

Project No. 2016-002

Project: Joanna Hills Estates **Location:** Joanna Road, Avon, MA

Date: March 6, 2020

According to the Massachusetts Stormwater Management Handbook, recharge BMP's must be designed to drain within 72 hours. Below is the drawdown calculation used:

Time $_{(Drawdown)} = R_v / (K*A)$

Where:

R_v: Required Recharge Volume (ft³)

K: Recharge Rate (Rawl's Rate) used to size the infiltration BMP (in/hour)

A: Bottom Area of the Infiltration Facility (ft²)

Location	R_{v}	K (in/hour)	K (ft/hr)	Α	Drawdown Time (hrs)	Drawdown Time Less than 72 Hours?
Basin A	402	2.41	0.20	1411	1.42	YES
Basin B	2334	2.41	0.20	5039	2.31	YES
Basin C	3038	2.41	0.20	4753	3.18	YES
Basin D	618	2.41	0.20	5028	0.61	YES
Basin E	1141	2.41	0.20	3709	1.53	YES

Drawdown Conforms to the Stormwater Management Standards

APPENDIX D: REQUIRED WATER QUALITY VOLUME & TSS REMOVAL CALCULATIONS

Water Quality Calculations

Project No. 2016-002

Project: Joanna Hills Estates
Location: Joanna Road, Avon, MA

Date: March 6, 2020

Calculate the Required Water Quality Volume

Basin A

Depth of Runoff (inches)	Discharge To:	Total Impervious Area (square feet)	Required Water Quality Volume (cubic feet)
0.5	Not Discharging to a Critical Area		0
1	Discharging to a Critical Area		0
1	Soils with Rapid Infiltration Rate >2.41 "/hr	10459	872
		Required Water Quality Volume	872
		Provided Water Quality Volume (EL: 238.74)	875

Basin B

Depth of Runoff (inches)	Discharge To:	Total Impervious Area (square feet)	Required Water Quality Volume (cubic feet)
0.5	Not Discharging to a Critical Area		0
1	Discharging to a Critical Area		0
1	Soils with Rapid Infiltration Rate >2.41 "/hr	55035	4586
		Required Water Quality Volume	4586
		Provided Water Quality Volume (EL: 226.35)	4633

Basin C

Depth of Runoff (inches)	Discharge To:	Total Impervious Area (square feet)	Required Water Quality Volume (cubic feet)
0.5	Not Discharging to a Critical Area		0
1	Discharging to a Critical Area		0
1	Soils with Rapid Infiltration Rate >2.41 "/hr	75803	6317
		Required Water Quality Volume	6317
		Provided Water Quality Volume (EL: 224.83)	6345

Water Quality Calculations

Project No. 2016-002

Project: Joanna Hills Estates
Location: Joanna Road, Avon, MA

Date: March 6, 2020

Calculate the Required Water Quality Volume

Basin D

Depth of Runoff (inches)	Discharge To:	Total Impervious Area (square feet)	Required Water Quality Volume (cubic feet)
0.5	Not Discharging to a Critical Area		0
1	Discharging to a Critical Area		0
1	Soils with Rapid Infiltration Rate >2.41 "/hr	27028	2252
		Required Water Quality Volume	2252
		Provided Water Quality Volume (EL: 239.41)	2262

Basin E

Basin E			
Depth of Runoff (inches)	Discharge To:	Total Impervious Area (square feet)	Required Water Quality Volume (cubic feet)
0.5	Not Discharging to a Critical Area		0
1	Discharging to a Critical Area		0
1	Soils with Rapid Infiltration Rate >2.41 "/hr	23469	1956
		Required Water Quality Volume	1956
		Provided Water Quality Volume (EL: 238.30)	1966

TSS Removal Spreadsheet

Project No. 2016-002

Project: Joanna Hills Estates
Location: Joanna Road, Avon, MA

Date: March 6, 2020

Treatment Train: To Basin A

ВМР	TSS Removal Rate	Starting TSS Load	Amount Removed	Remaining Load	TSS Removed
Deep Sump Hooded Catch Basins	25%	100%	75%	75%	25%
Water Quality Unit - WQU-2	95%	75%	71%	4%	96%
Infiltration Basin A	80%	4%	3%	1%	99%

Since the project is within soils with a rapid infiltration rate (greater than 2.4 inches per hour), 44% TSS removal pretreatment is required prior to infiltration.

96% > 44% so the pretreatment requirement is met

Treatment Train: To Basin B

ВМР	TSS Removal Rate	Starting TSS Load	Amount Removed	Remaining Load	TSS Removed
Deep Sump Hooded Catch Basins	25%	100%	75%	75%	25%
Water Quality Unit - WQU-3	87%	75%	65%	10%	90%
Infiltration Basin B	80%	10%	8%	2%	98%

Since the project is within soils with a rapid infiltration rate (greater than 2.4 inches per hour), 44% TSS removal pretreatment is required prior to infiltration.

90% > 44% so the pretreatment requirement is met

TSS Removal Spreadsheet

Project No. 2016-002

Project: Joanna Hills Estates **Location:** Joanna Road, Avon, MA

Date: March 6, 2020

Treatment Train: To Basin C

ВМР	TSS Removal Rate	Starting TSS Load	Amount Removed	Remaining Load	TSS Removed
Deep Sump Hooded Catch Basins	25%	100%	75%	75%	25%
Water Quality Unit - WQU-4	84%	75%	63%	12%	88%
Infiltration Basin C	80%	12%	10%	2%	98%

Since the project is within soils with a rapid infiltration rate (greater than 2.4 inches per hour), 44% TSS removal pretreatment is required prior to infiltration.

88% > 44% so the pretreatment requirement is met

Treatment Train: To Basin D

ВМР	TSS Removal Rate	Starting TSS Load	Amount Removed	Remaining Load	TSS Removed
Deep Sump Hooded Catch Basins	25%	100%	75%	75%	25%
Water Quality Unit - WQU-1	91%	75%	68%	7%	93%
Infiltration Basin D	80%	7%	5%	1%	99%

Since the project is within soils with a rapid infiltration rate (greater than 2.4 inches per hour), 44% TSS removal pretreatment is required prior to infiltration.

93% > 44% so the pretreatment requirement is met

TSS Removal Spreadsheet

Project No. 2016-002

Project: Joanna Hills Estates **Location:** Joanna Road, Avon, MA

Date: March 6, 2020

Treatment Train: To Basin E

ВМР	TSS Removal Rate	Starting TSS Load	Amount Removed	Remaining Load	TSS Removed
Deep Sump Hooded Catch Basins	25%	100%	75%	75%	25%
Water Quality Unit - WQU-5	88%	75%	66%	9%	91%
Infiltration Basin E	80%	9%	7%	2%	98%

Since the project is within soils with a rapid infiltration rate (greater than 2.4 inches per hour), 44% TSS removal pretreatment is required prior to infiltration.

91% > 44% so the pretreatment requirement is met





Brief Stormceptor Sizing Report - WQU-1

	Project Information & Location					
Project Name	Project Name Joanna Hills Estates		2016-002			
City	Avon	State/ Province	Massachusetts			
Country	Country United States of America		8/8/2019			
Designer Information	n	EOR Information	(optional)			
Name	Name JUSTIN WILLIAMS					
Company MBL LAND DEVELOPMENT & PERMITTING		Company				
Phone # 781-706-7433		Phone #				
Email Justin@MBLLandDevelopment.com		Email				

Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

Site Name	WQU-1
Target TSS Removal (%)	90
TSS Removal (%) Provided	91
Recommended Stormceptor Model	STC 2400

The recommended Stormceptor Model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary				
Stormceptor Model	% TSS Removal Provided			
STC 450i	82			
STC 900	88			
STC 1200	88			
STC 1800	88			
STC 2400	91			
STC 3600	91			
STC 4800	93			
STC 6000	93			
STC 7200	95			
STC 11000	96			
STC 13000	96			
STC 16000	97			





Sizing Details					
Drainage	Area	Water Quality Objective			
Total Area (acres)	1.18	TSS Removal	(%)	90.0	
Imperviousness %	56.0	Runoff Volume Cap	oture (%)		
Rainfa	Rainfall		Oil Spill Capture Volume (Gal)		
Station Name	BLUE HILL	Peak Conveyed Flow Rate (CFS)		6.10	
State/Province	Massachusetts	Water Quality Flow F	Rate (CFS)	1.21	
Station ID #	0736	Up Stro	eam Storage		
Years of Records	58	Storage (ac-ft)	Discha	rge (cfs)	
Latitude	42°12'44"N	0.000 0.000		000	
Longitude	71°6′53"W	Up Stream Flow Diversion			
		Max. Flow to Stormc	eptor (cfs)		

Particle Size Distribution (PSD) The selected PSD defines TSS removal Fine Distribution		
Particle Diameter (microns)	Distribution %	Specific Gravity
20.0	20.0	1.30
60.0	20.0	1.80
150.0	20.0	2.20
400.0	20.0	2.65
2000.0	20.0	2.65

Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules.
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed.
- For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

For Stormceptor Specifications and Drawings Please Visit: https://www.conteches.com/technical-guides/search?filter=1WBC0O5EYX





Brief Stormceptor Sizing Report - WQU-2

Project Information & Location			
Project Name	roject Name Joanna Hills Estates Project Number 2016-002		2016-002
City	Avon	State/ Province	Massachusetts
Country	United States of America	ates of America Date 8/8/2019	
Designer Information		EOR Information	(optional)
Name	JUSTIN WILLIAMS	Name	
Company	MBL LAND DEVELOPMENT & PERMITTING	NG Company	
Phone #	781-706-7433	Phone #	
Email	Justin@MBLLandDevelopment.com	Email	

Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

Site Name	WQU-2
Target TSS Removal (%)	95
TSS Removal (%) Provided	95
Recommended Stormceptor Model	STC 2400

The recommended Stormceptor Model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary		
Stormceptor Model	% TSS Removal Provided	
STC 450i	89	
STC 900	94	
STC 1200	94	
STC 1800	94	
STC 2400	95	
STC 3600	96	
STC 4800	97	
STC 6000	97	
STC 7200	98	
STC 11000	98	
STC 13000	98	
STC 16000	99	





Sizing Details				
Drainage	Area	Water Quality Objective		
Total Area (acres)	0.24	TSS Removal (%) 95.0		95.0
Imperviousness %	100.0	Runoff Volume Capture (%)		
Rainfa	Rainfall		Oil Spill Capture Volume (Gal)	
Station Name	BLUE HILL	Peak Conveyed Flow Rate (CFS) 1.8		1.81
State/Province	Massachusetts	Water Quality Flow Rate (CFS)		0.94
Station ID #	0736	Up Stream Storage		
Years of Records	58	Storage (ac-ft) Discharge (cfs)		rge (cfs)
Latitude	42°12'44"N	0.000 0.000		000
Longitude	71°6'53"W	Up Stream Flow Diversion		on
		Max. Flow to Stormceptor (cfs)		

Particle Size Distribution (PSD) The selected PSD defines TSS removal Fine Distribution			
Particle Diameter Distribution Specific Gravity (microns) %			
20.0	20.0	1.30	
60.0	20.0	1.80	
150.0	20.0	2.20	
400.0	20.0	2.65	
2000.0	20.0	2.65	

Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules.
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed.
- For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

For Stormceptor Specifications and Drawings Please Visit: https://www.conteches.com/technical-guides/search?filter=1WBC0O5EYX





Brief Stormceptor Sizing Report - WQU-3

Project Information & Location			
Project Name	roject Name Joanna Hills Estates Project Number 2016-002		2016-002
City	Avon	State/ Province	Massachusetts
Country	United States of America	ates of America Date 8/8/2019	
Designer Information		EOR Information	(optional)
Name	JUSTIN WILLIAMS	Name	
Company	MBL LAND DEVELOPMENT & PERMITTING	NG Company	
Phone #	781-706-7433	Phone #	
Email	Justin@MBLLandDevelopment.com	Email	

Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

Site Name	WQU-3
Target TSS Removal (%)	85
TSS Removal (%) Provided	87
Recommended Stormceptor Model	STC 2400

The recommended Stormceptor Model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary		
Stormceptor Model	% TSS Removal Provided	
STC 450i	75	
STC 900	83	
STC 1200	83	
STC 1800	83	
STC 2400	87	
STC 3600	87	
STC 4800	90	
STC 6000	90	
STC 7200	92	
STC 11000	94	
STC 13000	94	
STC 16000	95	





Sizing Details				
Drainage	Area	Water Quality Objective		
Total Area (acres)	3.47	TSS Removal (%) 85.0		85.0
Imperviousness %	36.3	Runoff Volume Capture (%)		
Rainfa	Rainfall		ume (Gal)	
Station Name	BLUE HILL	Peak Conveyed Flow Rate (CFS) 9.4		9.42
State/Province	Massachusetts	Water Quality Flow Rate (CFS)		2.22
Station ID #	0736	Up Stream Storage		
Years of Records	58	Storage (ac-ft) Discharge (cfs)		rge (cfs)
Latitude	42°12'44"N	0.000 0.000		000
Longitude	71°6'53"W	Up Stream Flow Diversion		on
		Max. Flow to Stormceptor (cfs)		

Particle Size Distribution (PSD) The selected PSD defines TSS removal Fine Distribution			
Particle Diameter Distribution Specific Gravity (microns) %			
20.0	20.0	1.30	
60.0	20.0	1.80	
150.0	20.0	2.20	
400.0	20.0	2.65	
2000.0	20.0	2.65	

Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules.
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed.
- For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

For Stormceptor Specifications and Drawings Please Visit: https://www.conteches.com/technical-guides/search?filter=1WBC0O5EYX





Brief Stormceptor Sizing Report - WQU-4

Project Information & Location			
Project Name	roject Name Joanna Hills Estates Project Number 2016-002		2016-002
City	Avon	State/ Province	Massachusetts
Country	United States of America	ates of America Date 8/8/2019	
Designer Information		EOR Information	(optional)
Name	JUSTIN WILLIAMS	Name	
Company	MBL LAND DEVELOPMENT & PERMITTING	NG Company	
Phone #	781-706-7433	Phone #	
Email	Justin@MBLLandDevelopment.com	Email	

Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

Site Name	WQU-4
Target TSS Removal (%)	84
TSS Removal (%) Provided	84
Recommended Stormceptor Model	STC 2400

The recommended Stormceptor Model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Siz	ing Summary
Stormceptor Model	% TSS Removal Provided
STC 450i	71
STC 900	80
STC 1200	80
STC 1800	80
STC 2400	84
STC 3600	84
STC 4800	87
STC 6000	88
STC 7200	90
STC 11000	92
STC 13000	93
STC 16000	94





Sizing Details				
Drainage	Area	Water Quality Objective		
Total Area (acres)	3.60	TSS Removal	(%)	84.0
Imperviousness %	48.3	Runoff Volume Cap	oture (%)	
Rainfa	all	Oil Spill Capture Vol	lume (Gal)	
Station Name	BLUE HILL	Peak Conveyed Flow Rate (CFS)		13.64
State/Province	Massachusetts	Water Quality Flow Rate (CFS)		3.91
Station ID #	Station ID # 0736		eam Storage	
Years of Records	58	Storage (ac-ft)	Discha	rge (cfs)
Latitude	42°12'44"N	0.000	0.000	
Longitude	71°6'53"W	Up Stream Flow Diversion		on
		Max. Flow to Stormc	eptor (cfs)	

Particle Size Distribution (PSD) The selected PSD defines TSS removal Fine Distribution				
Particle Diameter (microns)	Distribution %	Specific Gravity		
20.0	20.0	1.30		
60.0	20.0	1.80		
150.0	20.0	2.20		
400.0	20.0	2.65		
2000.0	20.0	2.65		

Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules.
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed.
- For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

For Stormceptor Specifications and Drawings Please Visit: https://www.conteches.com/technical-guides/search?filter=1WBC0O5EYX





Brief Stormceptor Sizing Report - WQU-5

Project Information & Location						
Project Name	Project Name Joanna Hills Estates		2016-002			
City	Avon	State/ Province	Massachusetts			
Country United States of America		Date	8/8/2019			
Designer Information	n	EOR Information (optional)				
Name	JUSTIN WILLIAMS	Name				
Company MBL LAND DEVELOPMENT & PERMITTING		Company				
Phone # 781-706-7433		Phone #				
Email Justin@MBLLandDevelopment.com		Email				

Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

Site Name	WQU-5
Target TSS Removal (%)	88
TSS Removal (%) Provided	88
Recommended Stormceptor Model	STC 2400

The recommended Stormceptor Model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Siz	ing Summary
Stormceptor Model	% TSS Removal Provided
STC 450i	76
STC 900	84
STC 1200	84
STC 1800	84
STC 2400	88
STC 3600	88
STC 4800	91
STC 6000	91
STC 7200	93
STC 11000	95
STC 13000	95
STC 16000	96





Sizing Details				
Drainage	Area	Water Quality Objective		
Total Area (acres)	3.10	TSS Removal (%)		88.0
Imperviousness %	32.9	Runoff Volume Cap	oture (%)	
Rainfa	ıll	Oil Spill Capture Vol	ume (Gal)	
Station Name	BLUE HILL	Peak Conveyed Flow Rate (CFS)		13.49
State/Province	Massachusetts	Water Quality Flow Rate (CFS)		1.73
Station ID #	Station ID # 0736		eam Storage	
Years of Records	58	Storage (ac-ft)	Discharge (cfs)	
Latitude	42°12'44"N	0.000	0.000	
Longitude 71°6′53"W		Up Stream	Flow Diversion	on
		Max. Flow to Stormo	eptor (cfs)	

Particle Size Distribution (PSD) The selected PSD defines TSS removal Fine Distribution				
Particle Diameter (microns)	Distribution %	Specific Gravity		
20.0	20.0	1.30		
60.0	20.0	1.80		
150.0	20.0	2.20		
400.0	20.0	2.65		
2000.0	20.0	2.65		

Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules.
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed.
- For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

For Stormceptor Specifications and Drawings Please Visit: https://www.conteches.com/technical-guides/search?filter=1WBC0O5EYX

APPENDIX E: CONSTRUCTION PERIOD POLLUTION PREVENTION PLAN

This construction period pollution prevention plan has been prepared in accordance with the Stormwater Management Policy issued by the Department of Environmental Protection (DEP), for the proposed site development at:

Joanna Hills Estates, Joanna Road, Avon, MA

SECTION I: POTENTIAL SOURCES OF POLLUTION

The following potential sources of pollution should be monitored during construction.

WASTE MATERIALS

All waste materials will be collected and stored in a securely lidded dumpster located more than 100 feet from any resource area as is reasonably practical. The dumpster will meet all local and State solid waste management regulations. All trash and construction debris from the site will be deposited in the dumpster at the end of each work day. No construction waste materials will be buried onsite. All personnel will be instructed regarding the correct procedure for waste disposal. Notices stating these practices will be posted in the office trailer, and the individual who manages day-to-day site operations will be responsible for seeing that these practices are followed.

HAZARDOUS WASTE

All hazardous waste materials will be disposed of in the manner specified by local or State regulation or by the manufacturer. Site personnel will be instructed in these practices and the individual, whom manages day-to-day site operations, will be responsible for seeing that these practices are followed.

SANITARY WASTE

All sanitary waste will be collected from the portable units a minimum of once per week by a licensed sanitary waste management contractor, as required by the local or State regulation.

NON-STORM WATER DISCHARGES

During construction activities at the site, some water from the site will be suitable for discharge. Uncontaminated groundwater from de-watering activities will be directed to recharge groundwater on-site. The construction de-watering and all non-stormwater discharges will be directed through a silt bag, dewatering or sedimentation basin prior to discharge to the wetlands. The general contractor will comply with the EPA.'s Final General Permit for Construction De-watering Discharges and the Stormwater Pollution Prevention Plan.

CONCRETE TRUCK WASHOUT AREAS

Concrete trucks will be directed to a washout area located outside of the 100-foot Wetland Buffer. Washout areas shall consist of a layer of polyurethane sheeting draped over a rectangular area built out of straw bales.

PROPER EQUIPMENT/ VEHICLE FUELING AND MAINTENANCE PRACTICES

Petroleum products related to the operation of said equipment will be stored and tightly sealed containers, which will be clearly labeled. Spray guns will be cleaned on a disposable tarp. Vehicles will not be allowed to refuel on-site.

SPILL PREVENTION AND CONTROL PLAN

Materials and equipment necessary for spill cleanup will be kept on-site. Equipment will include but not be limited to brooms, dustpans, mops, rags, gloves, goggles, kitty litter, sand, saw dust and plastic and metal trash containers. All spills will be cleaned up immediately upon discovery. Spills large enough to reach the stormwater management system shall be reported to the Massachusetts DEP or National Response Center at 1-800-424-8802.

SECTION II: BEST MANAGEMENT PRACTICES

An Erosion Control and Sedimentation Control program will be implemented to prevent indirect impact to the existing wetland, existing roadways, and surrounding sites during the construction. The program incorporates Best Management Practices (BMP's) as specified in the guidelines developed by DEP and the Environmental Protection Agency and complies with the requirements of the NPDES General Permit for Storm Water Discharges for Construction Activities. These measures include the installation of temporary erosion and sedimentation controls and construction sequencing. Areas of exposed soil will be kept to a minimum and/or phased during construction and a permanent vegetative cover or other forms of stabilization will be established as soon as practicable.

Proper implementation of the erosion and sedimentation control program will:

- Minimize exposed soils through temporary mulching or seeding or by sequencing so that the amount of exposed soil is kept to a minimum.
- Place erosion controls structures to manage erosion and site runoff.
- Managing the control structures through the life of the construction activities and repairing all damaged structures as well as removing trapped silt as soon as recommended.
- Establish a permanent vegetative cover or other forms of stabilization as soon as practicable.

The following erosion and sedimentation control BMP's are presented in the sequence to which they will be implemented at the site. The measures will be inspected on a weekly basis or immediately before and or after storm event greater than 0.5". The controls will be routinely maintained throughout the duration of the project. Any damaged controls will be repaired and or replaced immediately. The locations of the specified sedimentation and erosion control measures are depicted on the proposed design drawings.

EROSION CONTROL BARRIERS

Erosion control barriers will be installed and inspected by the appropriate authority at the down gradient limit of work prior to any construction. The barriers will consist of filter mitts and will be entrenched into the ground to prevent under flow. When necessary, additional erosion controls will be installed immediately down gradient of the erosion prone areas, such as the base of steep exposed slopes, around material stockpile areas, throughout the construction phase of the project. Erosion control barriers shall be inspected continuously and maintained and adjusted throughout construction. A sufficient supply of material shall be kept on site to facilitate the repair or replacement of the proposed barriers.

STABILIZED CONSTRUCTION ENTRANCE

The stabilized construction entrance shall be installed after site clearing but before any earth moving activities. The entrance should be maintained in a condition that will prevent tracking or flowing of sediment onto public rights-of-way. This may require periodic topdressing with additional stone. Remove mud and sediment tracked or washed onto public road immediately. Reshape pad as needed for drainage and runoff control. Repair any broken road pavement immediately. All temporary erosion and sediment control measures shall be removed within 30 days after final site stabilization is achieved or after the temporary practices are no longer needed. Trapped sediment shall be removed or stabilized on site. Inspect the pad and sediment disposal area weekly and after heavy rains or heavy use.

DUST CONTROL

Dust shall be controlled during construction by applying water to exposed surfaces during dry conditions.

STREET SWEEPING

The surfaces of Joanna Road, Lothrop Road, and the section of Page Street nearest Joanna Road to be swept by street sweeper a minimum of once per week and whenever sediment accumulates on the pavement surfaces.

TEMPORARY SURFACE AND SLOPE STABILIZATION

Any area of exposed soil that will remain unstabilized for a period of more than twenty days will be covered with a layer of straw or mulch until the time of final loam and seeding.

TEMPORARY SEEDING

A temporary vegetative cover of fast growing indigenous grasses will be established on areas of exposed soils that remain unstable for a period of twenty-one days. Depending on the slope, the seeded surfaces will be covered with a layer of mulch.

PERMANENT SEEDING

Upon completion of the final grading, any area not covered by pavement, other forms of stabilization, or other landscaped methods will be loamed and seeded with New England Erosion Control/Restoration Mix (for dry sites) produced by New England Wetland Plants, Inc. (or approved equivalent). This mix includes grasses and broad leaf herbaceous plants that are indigenous to the northeastern Massachusetts. Depending on slope the seeded area will be covered with mulch or erosion control blanket. The seed mix will be applied at a rate of 25lbs/acre.

INFILTRATION PROTECTION

The following practices should be implemented by the contractor to protect the in-situ soils in the location of the infiltration basin

- Never allow heavy construction equipment to drive across areas;
- Limit smearing and compacting of soils in infiltration areas;
- Rotary till or disc harrow to a depth of 12" to restore infiltration rates after final grading.

SECTION III: INSPECTIONS

Construction Inspections: Construction inspections shall be performed by personnel from the site contractor and/or the Engineer of Record, as appointed by the owner. Inspection forms shall be executed for each corresponding inspection.

- Perimeter Sediment Controls: FilterMitt will be laid in advance of construction along the
 perimeter of the project site in locations shown on the Erosion & Sedimentation Control
 Plan. Such barriers shall be inspected within 12 hours of a storm event in excess of 0.5"
 and weekly. Sediment deposits must be removed when the level of deposition reaches
 approximately one-half the height of the barrier.
- Construction Entrance: The temporary construction entrance should be maintained in a condition that will prevent tracking or flowing of sediment into the street. This may require periodic topdressing with additional stone.
 - The entrance should be inspected weekly and within 12 hours of a storm event in excess of 0.5". Mud and soil particles will eventually clog the voids in the gravel and the effectiveness of the gravel pad. When this occurs, the pad should be top dressed with new stone. Complete replacement of the pad may be necessary when the pad becomes completely clogged.
- Catch Basin Inlet Protection: Silt sacks are to be installed in all proposed catch basins to prevent sediment from entering the municipal drainage system prior to permanent stabilization. Silt sacks should be inspected after rainstorm in excess of 0.5" and weekly. In addition to the silt sacks a perimeter of hay bales shall be installed as an additional filter around all catch basins. Sediment should be disposed of in a suitable area and protected from erosion by either structural or vegetative means. Catch basin inlet protection should be removed and the area repaired as soon as the contributing drainage area to the inlet has been completely stabilized.

APPENDIX F: LONG TERM POLLUTION PREVENTION PLAN

To keep the stormwater management system functioning properly, a Long Term Pollution Prevention Plan is required. Adherence to this Long Term Pollution Prevention Plan will be the responsibility of the following:

Joanna Hills, LLC

LONG TERM POLLUTION PREVENTION PLAN TRAINING

Annual stormwater pollution prevention plan training shall be conducted. Training records shall be kept on file.

GOOD HOUSEKEEPING PRACTICES

All areas exposed to the weather shall be kept clean.

SOLID WASTE MANAGEMENT

Solid waste shall be kept in the covered dumpster and collected at a minimum of once per week and disposed of in a legal manner, at a state licensed recycling center or landfill.

REQUIREMENTS FOR STORAGE AND USE OF FERTILIZERS, HERBICIDES AND PESTICIDES

Fertilizers shall not be used within 100 feet of the wetland resource areas. Excess fertilizers shall be swept up from all impervious surfaces and not be allowed to run into the stormwater management system.

All fertilizers, herbicides and pesticides and any other hazardous substances shall be stored at least 100 feet from the wetlands resource areas and be kept in a wrapped or sealed container and under cover.

SNOW DISPOSAL AND PLOWING PLANS

Snow shall not be stored within 100 feet of wetland resource areas. If not possible to store the snow on-site, it shall be trucked away and disposed of in the same manner described above.

WINTER ROAD SALT/ SAND USE AND STORAGE RESTRICTIONS

Road salt shall not be used on the site.

STREET SWEEPING SCHEDULE

Street sweeping shall be performed on paved surfaces no less than four times per year.

APPENDIX G: OPERATION & MAINTENANCE PLAN

To keep the stormwater management system functioning properly and to ensure that the Total Suspended Solids (TSS) are reduced, periodic inspections and maintenance of the system is required. The operation and maintenance of all components of the proposed stormwater management system will be the responsibility of the following:

Joanna Hills, LLC

The following is a guideline of the specific maintenance schedules and tasks on a component by component basis that is required to keep the stormwater management system functioning properly. A log of the maintenance performed is to be perpetually maintained and a copy submitted to the Avon Conservation Commission annually.

DEEP SUMP CATCH BASINS

<u>Unscheduled Maintenance</u>: At the end of foliage and snow-removal seasons, inspect or clean the basin. Remove any branches, trash or other large debris that could interfere with the proper operation of the stormwater management system. Whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the basin, remove any accumulated sediment with clamshell buckets or vacuum trucks.

<u>Quarterly Maintenance</u>: Inspect or clean the basin. Remove any branches, trash or other large debris that could interfere with the proper operation of the stormwater management system. Remove any accumulated sediment with clamshell buckets or vacuum trucks.

WATER QUALITY UNITS

See attached on the following pages, the operation and maintenance requirements and owner's manual for the Stormceptor Water Quality Units.

PAVED AREAS

<u>Quarterly Maintenance</u>: Sweep, vacuum, or clean paved areas to reduce the amount of sediment entering the stormwater management system.

INFILTRATION BASINS WITH RIPRAP OVERFLOW SPILLWAYS

<u>Unscheduled Maintenance</u>: After rain events in excess of two inches, or after any snow or rain event accompanied by high winds, inspect the basin for debris. Remove any branches, trash, or other large debris that could interfere with the proper operation of the stormwater management system.

<u>General Maintenance</u>: Maintain the grassed side slopes of the basin through regular mowing. Keep the grass between three to six inches in length. Remove grass clippings to prevent them from impeding the flow of stormwater. During the spring and fall, remove any accumulated leaves from the basin including the rip rap overflow spillway. Reset any displaced rip rap.

Quarterly Maintenance: Inspect the basin for debris. Remove any branches, trash or other large debris that could interfere with the proper operation of the stormwater management system. Remove any accumulated sediment by the use of hand tools (rakes, shovels, wheelbarrows, etc.) when it exceeds three inches.

<u>Annual Maintenance</u>: Inspect the basin for debris. Remove any branches, trash or other large debris that could interfere with the proper operation of the stormwater management system. Remove any accumulated sediment by the use of hand tools (rakes, shovels, wheelbarrows, etc.) annually. Reset any displaced rip rap from the overflow spillway.

CULTEC 330XLHD INFILTRATION CHAMBERS (ROOF RECHARGE)

See attached on the following pages, the operation and maintenance requirements

<u>Semi Annual Maintenance (Spring And Fall)</u>: Check inlets and outlets for clogging and remove any debris that could interfere with the proper operation of the system.

PAVED AREAS:

QUARTERLY MAINTENANCE: SWEEP, VACUUM, OR CLEAN PAVED AREAS TO REDUCE THE AMOUNT OF SEDIMENT ENTERING THE STORMWATER MANAGEMENT SYSTEM.

STORMWATER OPERATION + MAINTENANCE (O+M) PLAN

DEEP SUMP CATCH BASINS:

UNSCHEDULED MAINTENANCE: AT THE END OF FOLIAGE AND SNOW-REMOVAL SEASONS, INSPECT OR CLEAN THE BASIN. REMOVE ANY BRANCHES, TRASH OR OTHER LARGE DEBRIS THAT COULD INTERFERE WITH THE PROPER OPERATION OF THE STORMWATER MANAGEMENT SYSTEM. WHENEVER THE DEPTH OF DEPOSITS IS GREATER THAN OR EQUAL TO ONE HALF THE DEPTH FROM THE BOTTOM OF THE INVERT OF THE LOWEST PIPE IN THE BASIN, REMOVE ANY ACCUMULATED SEDIMENT WITH CLAMSHELL BUCKETS OR VACUUM TRUCKS.

QUARTERLY MAINTENANCE: INSPECT OR CLEAN THE BASIN. REMOVE ANY BRANCHES, TRASH OR OTHER LARGE DEBRIS THAT COULD INTERFERE WITH THE PROPER OPERATION OF THE STORMWATER MANAGEMENT SYSTEM. REMOVE ANY ACCUMULATED SEDIMENT WITH CLAMSHELL BUCKETS OR VACUUM TRUCKS.

INFILTRATION BASIN WITH OVERFLOW SPILLWAY:

UNSCHEDULED MAINTENANCE: AFTER RAIN EVENTS IN EXCESS OF TWO INCHES, OR AFTER ANY SNOW OR RAIN EVENT ACCOMPANIED BY HIGH WINDS, INSPECT THE BASIN FOR DEBRIS. REMOVE ANY BRANCHES, TRASH, OR OTHER LARGE DEBRIS THAT COULD INTERFERE WITH THE PROPER OPERATION OF THE STORMWATER MANAGEMENT SYSTEM.

GENERAL MAINTENANCE: MAINTAIN THE GRASSED SIDE SLOPES OF THE BASIN THROUGH REGULAR MOWING. KEEP THE GRASS BETWEEN THREE TO SIX INCHES IN LENGTH. REMOVE GRASS CLIPPINGS TO PREVENT THEM FROM IMPEDING THE FLOW OF STORMWATER. DURING THE SPRING AND FALL, REMOVE ANY ACCUMULATED LEAVES FROM THE BASIN INCLUDING THE RIP RAP OVERFLOW SPILLWAY. RESET ANY DISPLACED RIP RAP.

QUARTERLY MAINTENANCE: INSPECT THE BASIN FOR DEBRIS. REMOVE ANY BRANCHES, TRASH OR OTHER LARGE DEBRIS THAT COULD INTERFERE WITH THE PROPER OPERATION OF THE STORMWATER MANAGEMENT SYSTEM. REMOVE ANY ACCUMULATED SEDIMENT BY THE USE OF HAND TOOLS (RAKES, SHOVELS, WHEELBARROWS, ETC.) WHEN IT EXCEEDS THREE INCHES.

ANNUAL MAINTENANCE: INSPECT THE BASIN FOR DEBRIS. REMOVE ANY BRANCHES, TRASH OR OTHER LARGE DEBRIS THAT COULD INTERFERE WITH THE PROPER OPERATION OF THE STORMWATER MANAGEMENT SYSTEM. REMOVE ANY ACCUMULATED SEDIMENT BY THE USE OF

CULTEC (330XLHD) INFILTRATION CHAMBERS (ROOF RECHARGE):

SEE FULL O&M REQUIREMENTS ATTACHED IN DRAINAGE REPORT.

<u>SEMI ANNUAL MAINTENANCE (SPRING AND FALL)</u>: CHECK INLETS AND OUTLETS FOR CLOGGING AND REMOVE ANY DEBRIS THAT COULD INTERFERE WITH THE PROPER OPERATION OF THE

STORMCEPTOR (STC 2400) WATER QUALITY UNITS:

SEE FULL O&M REQUIREMENTS ATTACHED IN DRAINAGE REPORT.

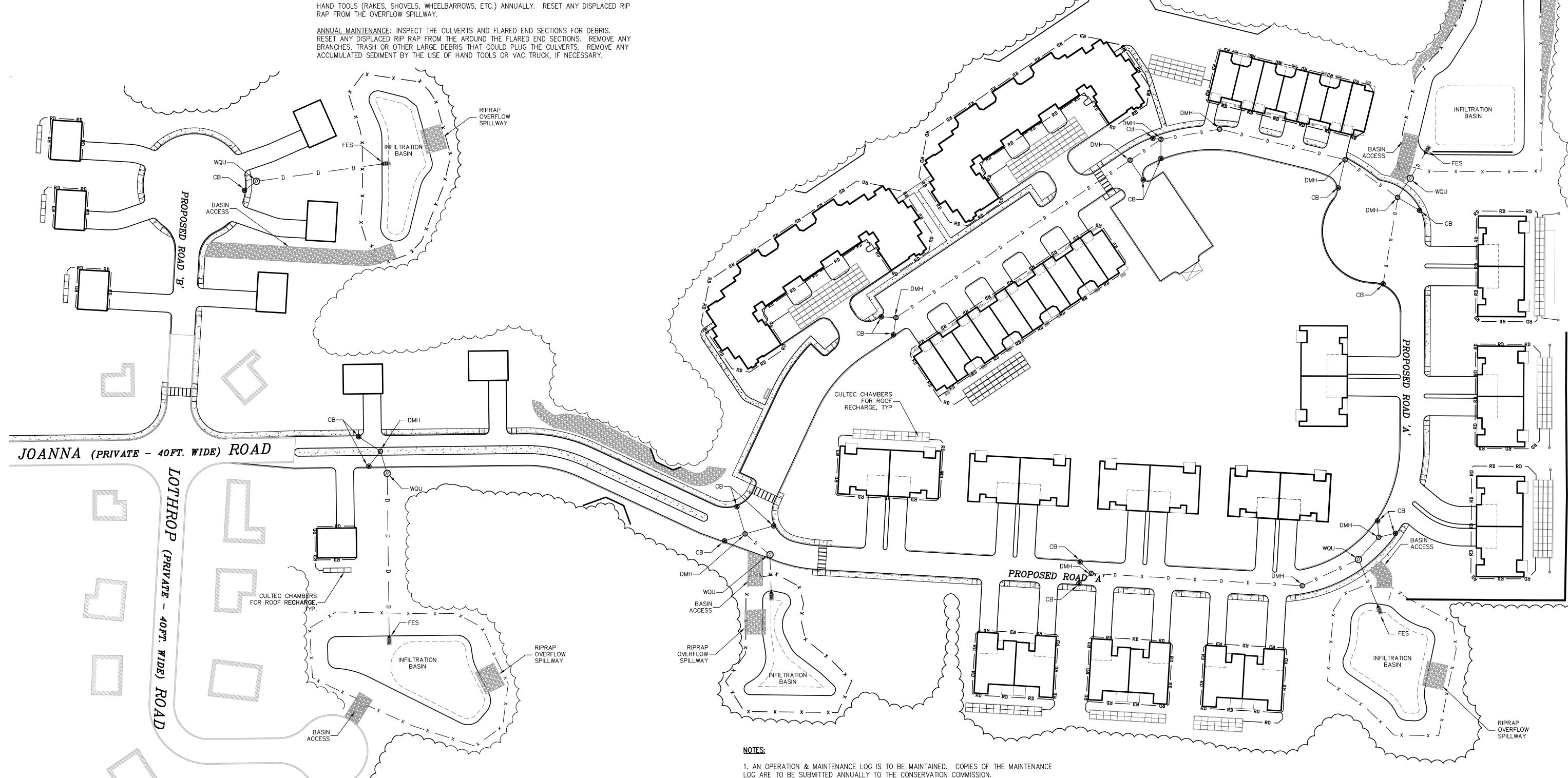
SEMI ANNUAL MAINTENANCE (SPRING AND FALL): CHECK INLETS, SEPARATION SCREENS AND OUTLETS FOR CLOGGING AND REMOVE ANY DEBRIS THAT COULD INTERFERE WITH THE PROPER OPERATION OF THE SYSTEM. REMOVE ANY ACCUMULATED SEDIMENT WITH VACUUM TRUCKS WHEN IT REACHES 50% OF THE CAPACITY IN THE SUMP.

LEGEND

WATER QUAILTY UNIT (WQU) FLARED-END SECTION WITH RIPRAP (FES)

CATCHBASIN (CB)

DRAIN MANHOLE (DMH)



	3/6/2020 DATE	REVISIONS PER TOWN & CONSULTANT COMMENTS DESCRIPTION	TLD BY	PROJ. MANAGER: CHIEF DESIGNER: REVIEWED BY:	MBL MBL DATE	SEAL	SEAL	PR JOAN 32 NOF
-		REVISIONS	1 2.					SOUTH EASTON

PREPARED FOR NNA HILLS LLC ORFOLK AVENUE

VERT. DATUM: HORZ.: VERT. GRAPHIC SCALE MASSACHUSETTS

HORZ.: 1"=50'

SCALE:

LAND DEVELOPMENT & PERMITTING, CORPLAND DEVELOPMENT, TRANSPORTATION AND ENVIRONMENTAL SOLUT 770 BROADWAY SUITE 6 RAYNHAM, MA. 02767 P.508.297.2746 F.508.297.2756 EMAIL:info@MBLLandDevelopment.com

SITE PLANS OPERATION & MAINTENANCE BMP MAP CHAPTER 40B - JOANNA HILLS ESTATES ASSESSORS MAP C7, BLOCK 3 & PLOT 15

OVERFLOW-SPILLWAY

O&M

PROJ. No.: 2016-002

DATE: MARCH 6, 2020

AVON MASSACHUSETTS



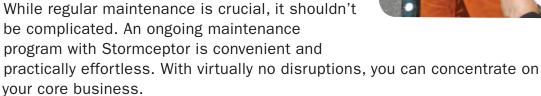
Inspection and Maintenance. Easy. Convenient.

When it rains, oils, sediment and other contaminants are captured and contained by over 40,000 Stormceptor units operating worldwide. While Stormceptor's patented scour prevention technology ensures captured pollutants remain in the unit during all rainfall events, the accumulated pollutants must eventually be removed as part of a regular maintenance program.

If neglected, oil and sediment gradually build up and diminish any BMP's efficiency, harming the environment and leaving owners and operators vulnerable to fines, surcharges and bad publicity.

Maintenance is a must

Ease, frequency and cost of maintenance are often overlooked by specifiers when considering the merits of a stormwater treatment system. In reality, maintenance is fundamental to the long-term performance of any stormwater quality treatment device.





Inspections are easily carried out above ground from any standard surface access cover through a visual inspection of the orifice and drop tee components. A sludge judge and oil dip-stick are all that are needed for sediment and oil depth measurements.

Easy unit access

Maintenance is typically conducted from the same surface access cover, eliminating the need for confined space entry into the unit. Your site remains undisturbed, saving you time and money.







No muss, no fuss and fast

Maintenance is performed quickly and inexpensively with a standard vacuum truck. Servicing usually takes less than two hours, with no disruption to your site.

A complete stormwater management plan for Stormceptor extends beyond installation and performance to regular maintenance. It's the smart, cost-effective way to ensure your unit continues to remove more pollutants than any other separator for decades to come.



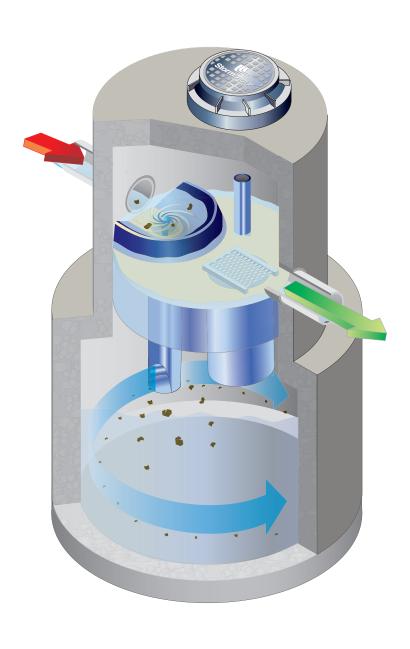
Stormceptor maintenance recommendations

- Units should be inspected post-construction, prior to being put into service.
- Inspect every six months for the first year of operation to determine the oil and sediment accumulation rate.
- In subsequent years, inspections can be based on first-year observations or local requirements.
- Cleaning is recommended once the sediment depth reaches 15% of storage capacity, (generally taking one year or longer). Local regulations for maintenance frequency may vary.
- · Inspect the unit immediately after an oil, fuel or chemical spill.
- A licensed waste management company should remove captured petroleum waste products from any oil, chemical or fuel spills and dispose responsibly.

With over 40,000 units operating worldwide, Stormceptor performs and protects every day, in every storm.



Stormceptor®Owner's Manual



Stormceptor is protected by one or more of the following patents:

Canadian Patent No. 2,137,942

Canadian Patent No. 2,175,277

Canadian Patent No. 2,180,305

Canadian Patent No. 2,180,338

Canadian Patent No. 2,206,338

Canadian Patent No. 2,327,768

U.S. Patent No. 5,753,115

U.S. Patent No. 5,849,181

U.S. Patent No. 6,068,765

U.S. Patent No. 6,371,690

U.S. Patent No. 7,582,216

U.S. Patent No. 7,666,303

Australia Patent No. 693.164

Australia Patent No. 707,133

Australia Patent No. 729,096

Australia Patent No. 779,401

Australia Patent No. 2008,279,378

Australia Patent No. 2008,288,900

Indonesia Patent No. 0007058

Japan Patent No. 3581233

Japan Patent No. 9-11476

Korean Patent No. 0519212

Malaysia Patent No. 118987

New Zealand Patent No. 314,646

New Zealand Patent No. 583,008

New Zealand Patent No. 583,583

South African Patent No. 2010/00682

South African Patent No. 2010/01796

Other Patents Pending

Table of Contents

- 1 Stormceptor Overview
- 2 Stormceptor Operation & Components
- 3 Stormceptor Identification
- 4 Stormceptor Inspection & Maintenance
 Recommended Stormceptor Inspection Procedure
 Recommended Stormceptor Maintenance Procedure
- 5 Contact Information (Stormceptor Licensees)

Congratulations!

Your selection of a Stormceptor® means that you have chosen the most recognized and efficient stormwater oil/sediment separator available for protecting the environment. Stormceptor is a pollution control device often referred to as a "Hydrodynamic Separator (HDS)" or an "Oil Grit Separator (OGS)", engineered to remove and retain pollutants from stormwater runoff to protect our lakes, rivers and streams from the harmful effects of non-point source pollution.

1 – Stormceptor Overview

Stormceptor is a patented stormwater quality structure most often utilized as a treatment component of the underground storm drain network for stormwater pollution prevention. Stormceptor is designed to remove sediment, total suspended solids (TSS), other pollutants attached to sediment, hydrocarbons and free oil from stormwater runoff. Collectively the Stormceptor provides spill protection and prevents non-point source pollution from entering downstream waterways.

Key benefits of Stormceptor include:

- Removes sediment, suspended solids, debris, nutrients, heavy metals, and hydrocarbons (oil and grease) from runoff and snowmelt.
- Will not scour or re-suspend trapped pollutants.
- Provides sediment and oil storage.
- Provides spill control for accidents, commercial and industrial developments.
- Easy to inspect and maintain (vacuum truck).
- "STORMCEPTOR" is *clearly* marked on the access cover (excluding inlet designs).
- Relatively small footprint.
- 3rd Party tested and independently verified.
- Dedicated team of experts available to provide support.

Model Types:

- STC (Standard)
- STF (Fiberglass)
- EOS (Extended Oil Storage)
- OSR (Oil and Sand Removal)
- MAX (Custom designed unit, specific to site)

Configuration Types:

- Inlet unit (accommodates inlet flow entry, and multi-pipe entry)
- In-Line (accommodates multi-pipe entry)
- Submerged Unit (accommodates the site's tailwater conditions)
- Series Unit (combines treatment in two systems)

Please Maintain Your Stormceptor

To ensure long-term environmental protection through continued performance as originally designed for your site, **Stormceptor must be maintained**, as any stormwater treatment practice does. The need for maintenance is determined through inspection of the Stormceptor. Procedures for inspection are provided within this document. Maintenance of the Stormceptor is performed from the surface via vacuum truck.

If you require information about Stormceptor, or assistance in finding resources to facilitate inspections or maintenance of your Stormceptor please call your local Stormceptor Licensee or Imbrium® Systems.

2 - Stormceptor Operation & Components

Stormceptor is a flexibly designed underground stormwater quality treatment device that is unparalleled in its effectiveness for pollutant capture and retention using patented flow separation technology.

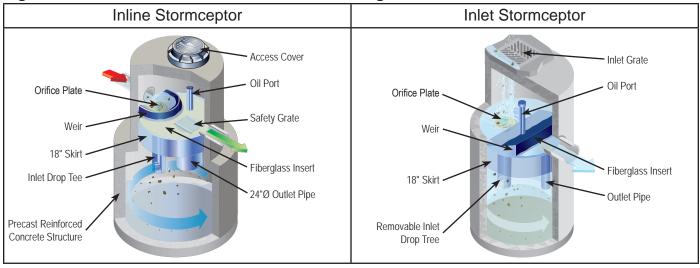
Stormceptor creates a non-turbulent treatment environment below the insert platform within the system. The insert diverts water into the lower chamber, allowing free oils and debris to rise, and sediment to settle under relatively low velocity conditions. These pollutants are trapped and stored below the insert and protected from large runoff events for later removal during the maintenance procedure.

With thousands of units operating worldwide, Stormceptor delivers reliable protection every day, in every storm. The patented Stormceptor design prohibits the scour and release of captured pollutants, ensuring superior water quality treatment and protection during even the most extreme storm events. Stormceptor's proven performance is backed by the longest record of lab and field verification in the industry.

Stormceptor Schematic and Component Functions

Below are schematics of two common Stormceptor configurations with key components identified and their functions briefly described.

Figure 1. Figure 2.



- Manhole access cover provides access to the subsurface components
- Precast reinforced concrete structure provides the vessel's watertight structural support
- Fiberglass insert separates vessel into upper and lower chambers
- Weir directs incoming stormwater and oil spills into the lower chamber
- Orifice plate prevents scour of accumulated pollutants
- **Inlet drop tee** conveys stormwater into the lower chamber
- Fiberglass skirt provides double-wall containment of hydrocarbons
- Outlet riser pipe conveys treated water to the upper chamber; primary vacuum line access port for sediment removal
- Oil inspection port primary access for measuring oil depth and oil removal
- Safety grate safety measure to cover riser pipe in the event of manned entry into vessel

3 - Stormceptor Identification

Stormceptor is available in both precast concrete and fiberglass vessels, with precast concrete often being the dominant material of construction.

In the Stormceptor, a patented, engineered fiberglass insert separates the structure into an upper chamber and lower chamber. The lower chamber will remain full of water, as this is where the pollutants are sequestered for later removal. Multiple Stormceptor model (STC, OSR, EOS, MAX and STF) configurations exist, each to be inspected and maintained in a similar fashion.

Each unit is easily identifiable as a Stormceptor by the trade name "Stormceptor" embossed on each access cover at the surface. To determine the location of "inlet" Stormceptor units with horizontal catch basin inlet, look down into the grate as the Stormceptor insert will be visible. The name "Stormceptor" is not embossed on inlet models due to the variability of inlet grates used/approved across North America.

Once the location of the Stormceptor is determined, the model number may be identified by comparing the measured depth from the fiberglass insert level at the outlet pipe's invert (water level) to the bottom of the tank using **Table 1**.

In addition, starting in 1996 a metal serial number tag containing the model number has been affixed to the inside of the unit, on the fiberglass insert. If the unit does not have a serial number, or if there is any uncertainty regarding the size of the unit using depth measurements, please contact your local Stormceptor Representative for assistance.

Sizes/Models

Typical general dimensions and capacities of the standard precast STC, EOS & OSR Stormceptor models in both USA and Canada/International (excluding South East Asia and Australia) are provided in **Tables 1 and 2**. Typical rim to invert measurements are provided later in this document. The total depth for cleaning will be the sum of the depth from outlet pipe invert (generally the water level) to rim (grade) and the depth from outlet pipe invert to the precast bottom of the unit. Note that depths and capacities may vary slightly between regions.

Table 1A. (US) Stormceptor Dimensions – Insert to Base of Structure

STC Model	Insert to Base (in.)
450	60
900	55
1200	71
1800	105
2400	94
3600	134
4800	128
6000	150
7200	134
11000*	128
13000*	150
16000*	134

EOS Model	Insert to Base (in.)
4-175	60
9-365	55
12-590	71
18-1000	105
24-1400	94
36-1700	134
48-2000	128
60-2500	150
72-3400	134
110-5000*	128
130-6000*	150
160-7800*	134

OSR Model	Insert to Base (in.)
65	60
140	55
250	94
390	128
560	134
780*	128
1125*	134

Typical STF m (in.)	
1.5 (60)	_
1.5 (61)	
1.8 (73)	
2.9 (115)	
2.3 (89)	
3.2 (127)	
2.9 (113)	
3.5 (138)	
3.3 (128)	
	_

Notes:

^{1.} Depth Below Pipe Inlet Invert to the Bottom of Base Slab can vary slightly by manufacturing facility, and can be modified to accommodate specific site designs, pollutant loads or site conditions. Contact your local representative for assistance.

^{*}Consist of two chamber structures in series.

Table 1B. (CA & Int'l) Stormceptor Dimensions - Insert to Base of Structure

STC Model	Insert to Base (m)
300	1.5
750	1.5
1000	1.8
1500	2.8
2000	2.8
3000	3.7
4000	3.4
5000	4.0
6000	3.7
9000*	3.4
11000*	4.0
14000*	3.7

EOS Model	Insert to Base (m)
300	1.5
750	1.5
1000	1.8
2000	2.8
3000	3.7
4000	3.4
5000	4.0
6000	3.7
9000*	3.4
10000*	4.0
14000*	3.7

OSR Model	Insert to Base (m)
300	1.7
750	1.6
2000	2.6
4000	3.6
6000	3.7
9000*	3.6
14000*	3.7

Typical STF m (in.)
1.5 (60)
1.5 (61)
1.8 (73)
2.9 (115)
2.3 (89)
3.2 (127)
2.9 (113)
3.5 (138)
3.3 (128)

Notes:

Table 2A. (US) Storage Capacities

STC Model	Hydrocarbon Storage Capacity	Sediment Capacity	Hydrocarbon Storage Capacity				Hydrocarbon Storage Capacity	Sediment Capacity
	gal	ft³		gal		gal	ft ³	
450	86	46	4-175	175	065	115	46	
900	251	89	9-365	365	140	233	58	
1200	251	127	12-590	591				
1800	251	207	18-1000	1198				
2400	840	205	24-1400	1457	250	792	156	
3600	840	373	36-1700	1773				
4800	909	543	48-2000	2005	390	1233	465	
6000	909	687	60-2500	2514				
7200	1059	839	72-3400	3418	560	1384	690	
11000*	2797	1089	110-5000*	5023	780*	2430	930	
13000*	2797	1374	130-6000*	6041				
16000*	3055	1677	160-7800*	7850	1125*	2689	1378	

Notes:

^{1.} Depth Below Pipe Inlet Invert to the Bottom of Base Slab can vary slightly by manufacturing facility, and can be modified to accommodate specific site designs, pollutant loads or site conditions. Contact your local representative for assistance.

^{*}Consist of two chamber structures in series.

^{1.} Hydrocarbon & Sediment capacities can be modified to accommodate specific site design requirements, contact your local representative for assistance.

^{*}Consist of two chamber structures in series.

Table 2B. (CA & Int'l) Storage Capacities

STC Model	Hydrocarbon Storage Capacity L	Sediment Capacity L	EOS Model	Hydrocarbon Storage Capacity L	OSR Model	Hydrocarbon Storage Capacity L	Sediment Capacity L
300	300	1450	300	662	300	300	1500
750	915	3000	750	1380	750	900	3000
1000	915	3800	1000	2235			
1500	915	6205					
2000	2890	7700	2000	5515	2000	2790	7700
3000	2890	11965	3000	6710			
4000	3360	16490	4000	7585	4000	4700	22200
5000	3360	20940	5000	9515			
6000	3930	26945	6000	12940	6000	5200	26900
9000*	10555	32980	9000*	19010	9000*	9300	33000
11000*	10555	37415	10000*	22865			
14000*	11700	53890	14000*	29715	14000*	10500	53900

Notes:

4 - Stormceptor Inspection & Maintenance

Regular inspection and maintenance is a proven, cost-effective way to maximize water resource protection for all stormwater pollution control practices, and is required to insure proper functioning of the Stormceptor. Both inspection and maintenance of the Stormceptor is easily performed from the surface. Stormceptor's patented technology has no moving parts, simplifying the inspection and maintenance process.

Please refer to the following information and guidelines before conducting inspection and maintenance activities.

When is inspection needed?

- Post-construction inspection is required prior to putting the Stormceptor into service.
- Routine inspections are recommended during the first year of operation to accurately assess the sediment accumulation.
- Inspection frequency in subsequent years is based on the maintenance plan developed in the first year.
- Inspections should also be performed immediately after oil, fuel, or other chemical spills.

When is maintenance cleaning needed?

For optimum performance, the unit should be cleaned out once the sediment depth reaches
the recommended maintenance sediment depth, which is approximately 15% of the unit's
total storage capacity (see **Table 2**). The frequency should be adjusted based on historical
inspection results due to variable site pollutant loading.

^{1.} Hydrocarbon & Sediment capacities can be modified to accommodate specific site design requirements, contact your local representative for assistance.

^{*}Consist of two chamber structures in series.

- Sediment removal is easier when removed on a regular basis at or prior to the recommended maintenance sediment depths, as sediment build-up can compact making removal more difficult.
- The unit should be cleaned out immediately after an oil, fuel or chemical spill.

What conditions can compromise Stormceptor performance?

- If construction sediment and debris is not removed prior to activating the Stormceptor unit, maintenance frequency may be reduced.
- If the system is not maintained regularly and fills with sediment and debris beyond the capacity as indicated in **Table 2**, pollutant removal efficiency may be reduced.
- If an oil spill(s) exceeds the oil capacity of the system, subsequent spills may not be captured.
- If debris clogs the inlet of the system, removal efficiency of sediment and hydrocarbons may be reduced.
- If a downstream blockage occurs, a backwater condition may occur for the Stormceptor and removal efficiency of sediment and hydrocarbons may be reduced.

What training is required?

The Stormceptor is to be inspected and maintained by professional vacuum cleaning service providers with experience in the maintenance of underground tanks, sewers and catch basins. For typical inspection and maintenance activities, no specific supplemental training is required for the Stormceptor. Information provided within this Manual (provided to the site owner) contains sufficient guidance to maintain the system properly.

In unusual circumstances, such as if a damaged component needs replacement or some other condition requires manned entry into the vessel, confined space entry procedures must be followed. Only professional maintenance service providers trained in these procedures should enter the vessel. Service provider companies typically have personnel who are trained and certified in confined space entry procedures according to local, state, and federal standards.

What equipment is typically required for inspection?

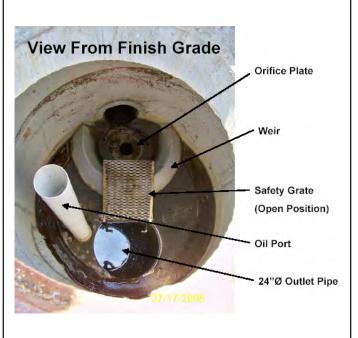
- · Manhole access cover lifting tool
- Oil dipstick / Sediment probe with ball valve (typically ¾-inch to 1-inch diameter)
- Flashlight
- Camera
- Data log / Inspection Report
- · Safety cones and caution tape
- · Hard hat, safety shoes, safety glasses, and chemical-resistant gloves

Recommended Stormceptor Inspection Procedure:

- Stormceptor is to be inspected from grade through a standard surface manhole access cover.
- Sediment and oil depth inspections are performed with a sediment probe and oil dipstick.
- Oil depth is measured through the oil inspection port, either a 4-inch (100 mm) or 6-inch (150 mm) diameter port.
- Sediment depth can be measured through the oil inspection port or the 24-inch (610 mm) diameter outlet riser pipe.
- Inspections also involve a visual inspection of the internal components of the system.

Figure 3. Figure 4.





What equipment is typically required for maintenance?

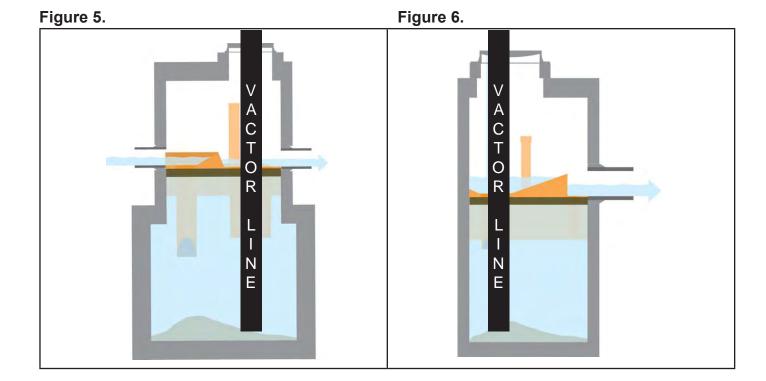
- Vacuum truck equipped with water hose and jet nozzle
- Small pump and tubing for oil removal
- · Manhole access cover lifting tool
- Oil dipstick / Sediment probe with ball valve (typically ¾-inch to 1-inch diameter)
- Flashlight
- Camera
- Data log / Inspection Report
- Safety cones
- Hard hats, safety shoes, safety glasses, chemical-resistant gloves, and hearing protection for service providers
- Gas analyzer, respiratory gear, and safety harness for specially trained personnel if confined space entry is required

Recommended Stormceptor Maintenance Procedure

Maintenance of Stormceptor is performed using a vacuum truck.

No entry into the unit is required for maintenance. *DO NOT ENTER THE STORMCEPTOR CHAMBER* unless you have the proper personal safety equipment, have been trained and are qualified to enter a confined space, as identified by local Occupational Safety and Health Regulations (e.g. 29 CFR 1910.146 or Canada Occupational Safety and Health Regulations – SOR/86-304). Without the proper equipment, training and permit, entry into confined spaces can result in serious bodily harm and potentially death. Consult local, provincial, and/or state regulations to determine the requirements for confined space entry. Be aware, and take precaution that the Stormceptor fiberglass insert may be slippery. In addition, be aware that some units do not have a safety grate to cover the outlet riser pipe that leads to the submerged, lower chamber.

- Ideally maintenance should be conducted during dry weather conditions when no flow is entering the unit.
- Stormceptor is to be maintained through a standard surface manhole access cover.
- Insert the oil dipstick into the oil inspection port. If oil is present, pump off the oil layer into separate containment using a small pump and tubing.
- Maintenance cleaning of accumulated sediment is performed with a vacuum truck.
 - For 6-ft (1800 mm) diameter models and larger, the vacuum hose is inserted into the lower chamber via the 24-inch (610 mm) outlet riser pipe.
 - For 4-ft (1200 mm) diameter model, the removable drop tee is lifted out, and the vacuum hose is inserted into the lower chamber via the 12-inch (305 mm) drop tee hole.



- Using the vacuum hose, decant the water from the lower chamber into a separate containment tank or to the sanitary sewer, if permitted by the local regulating authority.
- Remove the sediment sludge from the bottom of the unit using the vacuum hose. For large Stormceptor units, a flexible hose is often connected to the primary vacuum line for ease of movement in the lower chamber.
- Units that have not been maintained regularly, have surpassed the maximum recommended sediment capacity, or contain damaged components may require manned entry by trained personnel using safe and proper confined space entry procedures.

Figure 7.



Figure 8.



A maintenance worker stationed at the above ground surface uses a vacuum hose to evacuate water, sediment, and debris from the system.

What is required for proper disposal?

The requirements for the disposal of material removed from Stormceptor units are similar to that of any other stormwater treatment Best Management Practices (BMP). Local guidelines should be consulted prior to disposal of the separator contents. In most areas the sediment, once dewatered, can be disposed of in a sanitary landfill. It is not anticipated that the sediment would be classified as hazardous waste. This could be site and pollutant dependent. In some cases, approval from the disposal facility operator/agency may be required.

What about oil spills?

Stormceptor is often implemented in areas where there is high potential for oil, fuel or other hydrocarbon or chemical spills. Stormceptor units should be cleaned immediately after a spill occurs by a licensed liquid waste hauler. You should also notify the appropriate regulatory agencies as required in the event of a spill.

What if I see an oil rainbow or sheen at the Stormceptor outlet?

With a steady influx of water with high concentrations of oil, a sheen may be noticeable at the Stormceptor outlet. This may occur because a hydrocarbon rainbow or sheen can be seen at

very small oil concentrations (< 10 ppm). Stormceptor is effective at removing 95% of free oil, and the appearance of a sheen at the outlet with high influent oil concentrations does not mean that the unit is not working to this level of removal. In addition, if the influent oil is emulsified, the Stormceptor will not be able to remove it. The Stormceptor is designed for free oil removal and not emulsified or dissolved oil conditions.

What factors affect the costs involved with inspection/maintenance?

The Vacuum Service Industry for stormwater drainage and sewer systems is a well-established sector of the service industry that cleans underground tanks, sewers and catch basins. Costs to clean Stormceptor units will vary. Inspection and maintenance costs are most often based on unit size, the number of units on a site, sediment/oil/hazardous material loads, transportation distances, tipping fees, disposal requirements and other local regulations.

What factors predict maintenance frequency?

Maintenance frequency will vary with the amount of pollution on your site (number of hydrocarbon spills, amount of sediment, site activity and use, etc.). It is recommended that the frequency of maintenance be increased or reduced based on local conditions. If the sediment load is high from an unstable site or sediment loads transported from upstream catchments, maintenance may be required semi-annually. Conversely once a site has stabilized, maintenance may be required less frequently (for example: two to seven year, site and situation dependent). Maintenance should be performed immediately after an oil spill or once the sediment depth in Stormceptor reaches the value specified in **Table 3** based on the unit size.

Table 3A. (US) Recommended Sediment Depths Indicating Maintenance

STC Model	Maintenance Sediment depth (in)	EOS Model	Maintenance Sediment depth (in)	Oil Storage Depth (in)	OSR Model	Maintenance Sediment depth (in)
450	8	4-175	9	24	065	8
900	8	9-365	9	24	140	8
1200	10	12-590	11	39		
1800	15					
2400	12	24-1400	14	68	250	12
3600	17	36-1700	19	79		
4800	15	48-2000	16	68	390	17
6000	18	60-2500	20	79		
7200	15	72-3400	17	79	560	17
11000*	17	110-5000*	16	68	780*	17
13000*	20	130-6000*	20	79		
16000*	17	160-7800*	17	79	1125*	17

Note:

^{1.} The values above are for typical standard units.

^{*}Per structure.

Table 3B. (CA & Int'l) Recommended Sediment Depths Indicating Maintenance

STC Model	Maintenance Sediment depth (mm)	EOS Model	Maintenance Sediment depth (mm)	Oil Storage Depth (mm)	OSR Model	Maintenance Sediment depth (mm)
300	225	300	225	610	300	200
750	230	750	230	610	750	200
1000	275	1000	275	990		
1500	400					
2000	350	2000	350	1727	2000	300
3000	475	3000	475	2006		
4000	400	4000	400	1727	4000	375
5000	500	5000	500	2006		
6000	425	6000	425	2006	6000	375
9000*	400	9000*	400	1727	9000*	425
11000*	500	10000*	500	2006		
14000*	425	14000*	425	2006	14000*	425

Note:

Replacement parts

Since there are no moving parts during operation in a Stormceptor, broken, damaged, or worn parts are not typically encountered. Therefore, inspection and maintenance activities are generally focused on pollutant removal. However, if replacements parts are necessary, they may be purchased by contacting your local Stormceptor Representative, or Imbrium Systems.

The benefits of regular inspection and maintenance are many – from ensuring maximum operation efficiency, to keeping maintenance costs low, to the continued protection of natural waterways – and provide the key to Stormceptor's long and effective service life.

Stormceptor Inspection and Maintenance Log

tormceptor Model No:
llowable Sediment Depth:
erial Number:
stallation Date:
ocation Description of Unit:
ther Comments:

^{1.} The values above are for typical standard units.

^{*}Per structure.

Contact Information

Questions regarding the Stormceptor can be addressed by contacting your area Stormceptor Licensee, Imbrium Systems, or visit our website at www.stormceptor.com.

Stormceptor Licensees:

CANADA

Lafarge Canada Inc. www.lafargepipe.com

403-292-9502 / 1-888-422-4022 Calgary, AB 780-468-5910 Edmonton, AB

204-958-6348 Winnipeg, MB, NW. ON, SK

Langley Concrete Group

www.langleyconcretegroup.com

604-502-5236 BC

Hanson Pipe & Precast Inc. www.hansonpipeandprecast.com

519-622-7574 / 1-888-888-3222 ON

Lécuyer et Fils Ltée. www.lecuyerbeton.com

450-454-3928 / 1-800-561-0970 QC

Strescon Limited www.strescon.com

902-494-7400 NS, NF 506-633-8877 NB, PE

UNITED STATES

Rinker Materials www.rinkerstormceptor.com 1-800-909-7763

AUSTRALIA & SOUTHEAST ASIA, including New Zealand & Japan

Humes Water Solutions www.humes.com.au +61 7 3364 2894

Imbrium Systems Inc. & Imbrium Systems LLC

 Canada
 1-416-960-9900 / 1-800-565-4801

 United States
 1-301-279-8827 / 1-888-279-8826

 International
 +1-416-960-9900 / +1-301-279-8827

Email info@imbriumsystems.com

www.imbriumsystems.com www.stormceptor.com

Stormceptor® Owner's Manual STC_OM_05/14

Contactor® & Recharger® **Stormwater Chambers**



Operation and Maintenance Guidelines

for **CULTEC Stormwater Management Systems**





Operations and Maintenance Guidelines

Published by
CULTEC, Inc.
P.O. Box 280
878 Federal Road
Brookfield, Connecticut 06804 USA
www.cultec.com

Copyright Notice

© 2017 CULTEC, Inc. All rights reserved. Printed in the USA.

This document and any accompanying CULTEC products are copyrighted by CULTEC, Inc. Any reproduction and/or distribution without prior written consent from CULTEC, Inc. is strictly prohibited.

Disclaimers:

The drawings, photographs and illustrations shown in this document are for illustrative purposes only and are not necessarily to scale. Actual designs may vary.

CULTEC reserves the right to make design and/or specification changes at any time without notice at CULTEC's sole discretion.

CULTEC is not responsible for typographical errors.

Protected by one or more of the following patents:

Protected by one or more of the following patents:

U.S. Patents 6,129,482; 6,322,288; 6,854,925; 7,226,241; 7,806,627; 8,366,346; 8,425,148; and others; U.S. Designs D613819; D638,095; D668,318 and others; Canadian Patent 2,591,255 and others; Community Designs 1092191; 1745209; and others.

CULTEC, the CULTEC logo, RECHARGER, CONTACTOR, HVLV, PAC, STORMFILTER, STORMGENIE and The Chamber with The Stripe are registered trademarks of CULTEC, Inc.

Chamber of Choice, 902, HD, 100, 125, 150, 150XL, 180, 280, 330, 330XL, V8, 902, Field Drain Panel, C-1, C-2, C-3, C-4, EZ-24, Landscape Series are trademarks of CULTEC, Inc. All rights reserved.

Contact Information:

For general information on our other products and services, please contact our offices within the United States at (800)428-5832, (203)775-4416 ext. 202, or e-mail us at custservice@cultec.com.

For technical support, please call (203)775-4416 ext. 203 or e-mail tech@cultec.com.

Visit www.cultec.com/downloads.html for Product Downloads and CAD details.

Doc ID: CULG008 05-17

May 2017

These instructions are for single-layer traffic applications only. For multi-layer applications, contact CULTEC.

All illustrations and photos shown herein are examples of typical situations. Be sure to follow the engineer's drawings.

Actual designs may vary.



This manual contains guidelines recommended by CULTEC, Inc. and may be used in conjunction with, but not to supersede, local regulations or regulatory authorities. OSHA Guidelines must be followed when inspecting or cleaning any structure.

Introduction

The CULTEC Subsurface Stormwater Management System is a high-density polyethylene (HDPE) chamber system arranged in parallel rows surrounded by washed stone. The CULTEC chambers create arch-shaped voids within the washed stone to provide stormwater detention, retention, infiltration, and reclamation. Filter fabric is placed between the native soil and stone interface to prevent the intrusion of fines into the system. In order to minimize the amount of sediment which may enter the CULTEC system, a sediment collection device (stormwater pretreatment device) is recommended upstream from the CULTEC chamber system. Examples of pretreatment devices include, but are not limited to, an appropriately sized catch basin with sump, pretreatment catchment device, oil grit separator, or baffled distribution box. Manufactured pretreatment devices may also be used in accordance with CULTEC chambers. Installation, operation, and maintenance of these devices shall be in accordance with manufacturer's recommendations. Almost all of the sediment entering the stormwater management system will be collected within the pretreatment device.

Best Management Practices allow for the maintenance of the preliminary collection systems prior to feeding the CULTEC chambers. The pretreatment structures shall be inspected for any debris that will restrict inlet flow rates. Outfall structures, if any, such as outlet control must also be inspected for any obstructions that would restrict outlet flow rates. OSHA Guidelines must be followed when inspecting or cleaning any structure.

Operation and Maintenance Requirements

I. Operation

CULTEC stormwater management systems shall be operated to receive only stormwater run-off in accordance with applicable local regulations. CULTEC subsurface stormwater management chambers operate at peak performance when installed in series with pretreatment. Pretreatment of suspended solids is superior to treatment of solids once they have been introduced into the system. The use of pretreatment is adequate as long as the structure is maintained and the site remains stable with finished impervious surfaces such as parking lots, walkways, and pervious areas are properly maintained. If there is to be an unstable condition, such as improvements to buildings or parking areas, all proper silt control measures shall be implemented according to local regulations.

II. Inspection and Maintenance Options

- A. The CULTEC system may be equipped with an inspection port located on the inlet row. The inspection port is a circular cast box placed in a rectangular concrete collar. When the lid is removed, a 6-inch (150 mm) pipe with a screw-in plug will be exposed. Remove the plug. This will provide access to the CULTEC Chamber row below. From the surface, through this access, the sediment may be measured at this location. A stadia rod may be used to measure the depth of sediment if any in this row. If the depth of sediment is in excess of 3 inches (76 mm), then this row should be cleaned with high pressure water through a culvert cleaning nozzle. This would be carried out through an upstream manhole or through the CULTEC StormFilter Unit (or other pretreatment device). CCTV inspection of this row can be deployed through this access port to deter mine if any sediment has accumulated in the inlet row.
- **B.** If the CULTEC bed is not equipped with an inspection port, then access to the inlet row will be through an upstream manhole or the CULTEC StormFilter.

1. Manhole Access

This inspection should only be carried out by persons trained in confined space entry and sewer inspection services. After the manhole cover has been removed a gas detector must be lowered into the manhole to ensure that there are not high concentrations of toxic gases present. The inspector should be lowered into the manhole with the proper safety equipment as per OSHA requirements. The inspector may be able to observe sediment from this location. If this is not possible, the inspector will need to deploy a CCTV robot to permit viewing of the sediment.

Operations and Maintenance Guidelines



2. StormFilter Access

Remove the manhole cover to allow access to the unit. Typically a 30-inch (750 mm) pipe is used as a riser from the StormFilter to the surface. As in the case with manhole access, this access point requires a technician trained in confined space entry with proper gas detection equipment. This individual must be equipped with the proper safety equipment for entry into the StormFilter. The technician will be lowered onto the StormFilter unit. The hatch on the unit must be removed. Inside the unit are two filters which may be removed according to StormFilter maintenance guidelines. Once these filters are removed the inspector can enter the StormFilter unit to launch the CCTV camera robot.

C. The inlet row of the CULTEC system is placed on a polyethylene liner to prevent scouring of the washed stone beneath this row. This also facilitates the flushing of this row with high pressure water through a culvert cleaning nozzle. The nozzle is deployed through a manhole or the StormFilter and extended to the end of the row. The water is turned on and the inlet row is back-flushed into the manhole or StormFilter. This water is to be removed from the manhole or StormFilter using a vacuum truck.

III. Maintenance Guidelines

The following guidelines shall be adhered to for the operation and maintenance of the CULTEC stormwater management system:

- **A**. The owner shall keep a maintenance log which shall include details of any events which would have an effect on the system's operational capacity.
- **B.** The operation and maintenance procedure shall be reviewed periodically and changed to meet site conditions.
- C. Maintenance of the stormwater management system shall be performed by qualified workers and shall follow applicable occupational health and safety requirements.
- **D.** Debris removed from the stormwater management system shall be disposed of in accordance with applicable laws and regulations.

IV. Suggested Maintenance Schedules

A. Minor Maintenance

The following suggested schedule shall be followed for routine maintenance during the regular operation of the stormwater system:

Frequency	Action
Monthly in first year	Check inlets and outlets for clogging and remove any debris, as required.
Spring and Fall	Check inlets and outlets for clogging and remove any debris, as required.
One year after commissioning and every third year following	Check inlets and outlets for clogging and remove any debris, as required.

B. Major Maintenance

The following suggested maintenance schedule shall be followed to maintain the performance of the CULTEC stormwater management chambers. Additional work may be necessary due to insufficient performance and other issues that might be found during the inspection of the stormwater management chambers. (See table on next page)



	Frequency	Action
Inlets and Outlets	Every 3 years	Obtain documentation that the inlets, outlets and vents have been cleaned and will function as intended.
	Spring and Fall	Check inlet and outlets for clogging and remove any debris as required.
CULTEC Stormwater Chambers	2 years after commissioning	Inspect the interior of the stormwater management chambers through inspection port for deficiencies using CCTV or comparable technique.
		Obtain documentation that the stormwater management chambers and feed connectors will function as anticipated.
	9 years after commis- sioning every 9 years following	Clean stormwater management chambers and feed connectors of any debris.
	Tonorming	Inspect the interior of the stormwater management structures for deficiencies using CCTV or comparable technique.
		Obtain documentation that the stormwater management chambers and feed connectors have been cleaned and will function as intended.
	45 years after com- missioning	Clean stormwater management chambers and feed connectors of any debris.
		Determine the remaining life expectancy of the stormwater management chambers and recommended schedule and actions to rehabilitate the stormwater management chambers as required.
		Inspect the interior of the stormwater management chambers for deficiencies using CCTV or comparable technique.
		Replace or restore the stormwater management chambers in accordance with the schedule determined at the 45-year inspection.
		Attain the appropriate approvals as required.
		Establish a new operation and maintenance schedule.
Surrounding Site	Monthly in 1 st year	Check for depressions in areas over and surrounding the stormwater management system.
	Spring and Fall	Check for depressions in areas over and surrounding the stormwater management system.
	Yearly	Confirm that no unauthorized modifications have been performed to the site.

For additional information concerning the maintenance of CULTEC Subsurface Stormwater Management Chambers, please contact CULTEC, Inc. at 1-800-428-5832.



WQMP Operation & Maintenance (O&M) Plan

Project Name:
Prepared for:
Project Name:
Address:
City, State Zip:
Prepared on:
Date:



This O&M Plan describes the designated responsible party for implementation of this WQMP, including: operation and maintenance of all the structural BMP(s), conducting the training/educational program and duties, and any other necessary activities. The O&M Plan includes detailed inspection and maintenance requirements for all structural BMPs, including copies of any maintenance contract agreements, manufacturer's maintenance requirements, permits, etc.

8.1.1 Project Information

Project name	
Address	
City, State Zip	
Site size	
List of structural BMPs, number of each	
Other notes	

8.1.2 Responsible Party

The responsible party for implementation of this WQMP is:

Name of Person or HOA Property Manager	
Address	
City, State Zip	
Phone number	
24-Hour Emergency Contact number	
Email	

8.1.3 Record Keeping

Parties responsible for the O&M plan shall retain records for at least 5 years.

All training and educational activities and BMP operation and maintenance shall be documented to verify compliance with this O&M Plan. A sample Training Log and Inspection and Maintenance Log are included in this document.

8.1.4 Electronic Data Submittal

This document along with the Site Plan and Attachments shall be provided in PDF format. AutoCAD files and/or GIS coordinates of BMPs shall also be submitted to the City.





Appendix ____

BMP SITE PLAN

Site plan is preferred on minimum 11" by 17" colored sheets, as long as legible.



BMP OPERATION & MAINTENANCE LOG

Brief Description of Implementation, Maintenance, and Inspection Activity Performed





Minor Maintenance

Frequency		Action
Monthly in first year		Check inlets and outlets for clogging and remove any debris, as required.
		Notes
□ Month 1	Date:	
□ Month 2	Date:	
□ Month 3	Date:	
□ Month 4	Date	
□ Month 5	Date:	
□ Month 6	Date:	
□ Month 7	Date:	
□ Month 8	Date:	
□ Month 9	Date:	
□ Month 10	Date:	
□ Month 11	Date:	
□ Month 12	Date:	
Spring and	Fall	Check inlets and outlets for clogging and remove any debris, as required.
		Notes
□ Spring	Date:	
□ Fall	Date:	
□ Spring	Date:	
□ Fall	Date:	
□ Spring	Date:	
□ Fall	Date:	
□ Spring	Date:	
□ Fall	Date:	
□ Spring	Date:	
□ Fall	Date:	
□ Spring	Date:	
□ Fall	Date:	
	fter commissioning	Check inlets and outlets for clogging and remove any debris, as required.
	hird year following	Notes
□ Year 1	Date:	
□ Year 4	Date:	
□ Year 7	Date:	
□ Year 10	Date:	
□ Year 13	Date:	
□ Year 16	Date:	
□ Year 19	Date:	
□ Year 22	Date:	



Major Maintenance

	Frequency		Action
	Every 3 years		Obtain documentation that the inlets, outlets and vents have been cleaned and will function as intended.
	□ Year 1	Date:	Notes
	□ Year 4	Date:	
	□ Year 7	Date:	+
	□ Year 10	Date:	
	□ Year 13	Date:	
	□ Year 16	Date:	
its	□ Year 19	Date:	
T He	□ Year 22	Date:	
Inlets and Outlets	Spring and Fall	1	Check inlet and outlets for clogging and remove any debris, as required.
l et et		T	Notes
<u>=</u>	□ Spring	Date:	
	□ Fall	Date:	
	□ Spring	Date:	
	□ Fall	Date:	
	□ Spring	Date:	
	□ Fall	Date:	
	□ Spring	Date:	
	□ Fall	Date:	
	□ Spring	Date:	
	□ Fall	Date:	
	□ Spring	Date:	
	□ Fall	Date:	
nbers	2 years after cor	nmissioning	□ Inspect the interior of the stormwater management chambers through inspection port for deficiencies using CCTV or comparable technique.
Chan			□ Obtain documentation that the stormwater management chambers and feed connectors will function as anticipated.
ate		_	Notes
CULTEC Stormwater Chambers	□ Year 2	Date:	



Major Maintenance

	Frequency		Action
	9 years after cor every 9 years fol		Clean stormwater management chambers and feed connectors of any debris.
			$\hfill \square$ Inspect the interior of the stormwater management structures for deficiencies using CCTV or comparable technique.
			☐ Obtain documentation that the stormwater management chambers and feed connectors have been cleaned and will function as intended.
			Notes
	□ Year 9	Date:	
	□ Year 18	Date:	
	□ Year 27	Date:	
oers	□ Year 36	Date:	
Chaml	45 years after co	ommissioning	Clean stormwater management chambers and feed connectors of any debris.
CULTEC Stormwater Chambers			□ Determine the remaining life expectancy of the stormwater management chambers and recommended schedule and actions to rehabilitate the stormwater management chambers as required.
EC Stori			□ Inspect the interior of the stormwater management chambers for deficiencies using CCTV or comparable technique.
CULT			$\hfill \square$ Replace or restore the stormwater management chambers in accordance with the schedule determined at the 45-year inspection.
			□ Attain the appropriate approvals as required.
			Establish a new operation and maintenance schedule.
			Notes
	□ Year 45	Date:	



Major Maintenance

	Frequency		Action
	Monthly in 1st ye	ear	□ Check for depressions in areas over and surrounding the stormwater management system.
			Notes
	□ Month 1	Date:	
	□ Month 2	Date:	
	□ Month 3	Date:	
	□ Month 4	Date:	
	□ Month 5	Date:	
	□ Month 6	Date:	
	□ Month 7	Date:	
	□ Month 8	Date:	
	□ Month 9	Date:	
	□ Month 10	Date:	
	□ Month 11	Date:	
	□ Month 12	Date:	
	Spring and Fall	<u>'</u>	☐ Check for depressions in areas over and surrounding the stormwater management system.
ite			Notes
Surrounding Site	□ Spring	Date:	
l gi	□ Fall	Date:	
pur	□ Spring	Date:	
l ror	□ Fall	Date:	
l Sur	□ Spring	Date:	
,	□ Fall	Date:	
	□ Spring	Date:	
	□ Fall	Date:	
	□ Spring	Date:	
	□ Fall	Date:	
	□ Spring	Date:	
	□ Fall	Date:	
	Yearly		Confirm that no unauthorized modifications have been performed to the site.
		<u> </u>	Notes
	□ Year 1	Date:	
	□ Year 2	Date:	
	□ Year 3	Date:	
	□ Year 4	Date:	
	□ Year 5	Date:	
	□ Year 6	Date:	
	□ Year 7	Date:	





Stormwater Management System Annual Cost of Operation & Maintenance

Project No. 2016-002

Project: Joanna Hills Estates
Location: Joanna Road, Avon, MA

Date: March 6, 2020

Annual O&M Budget

Facility	Activity	Frequency	Units	Quantity	Unit Price	Cost	Cost/ Year
All	Routine Inspections	Quarterly	Hrs	16	\$25	\$400	\$1,600
All	Landscaping (basin mowing)	As needed	LS	1	\$500	\$500	\$2,000
Recharge Chambers	Recharge Chamber CCTV Inspection	2 Years	Each	15	\$300	\$4,500	\$2,250
Recharge Chambers	Recharge Chamber Cleaning	3 Years	Each	15	\$50	\$750	\$250
Deep Sump Hooded Catch Basins	Catch Basin Cleaning	Quarterly	Each	17	\$50	\$850	\$3,400
Water Quality Units	Cleaning	Semi- Annually	Each	5	\$50	\$250	\$500
Culverts & Flared End Sections	Debris Removal	Quarterly	Each	5	\$25	\$125	\$500
	Sediment Removal - Excavation	5 Years	Hrs	16	\$125	\$2,000	\$400
Infiltration Basins	Sediment Removal - Disposal	5 Years	CY	25	\$50	\$1,250	\$250
	Refurbishment - Till & Seed	5 Years	SF	53646	0.25	\$13,412	\$2,682
Overflow Spillways	Debris Removal/ Replace Rip Rap	Annual	Each	5	\$100	\$500	\$500
						Total	\$14,332

APPENDIX H: ILLICIT DISCHARGE COMPLIANCE STATEMENT

To the best of my knowledge, no detectable illicit discharges exist on-site. The site plans included with this report detail the stormwater management system that manages stormwater on the site and demonstrate that the system does not include the entry of an illicit discharge. As the owner, I will ultimately be responsible for implementing the Long Term Pollution Prevention Plan which includes measures to prevent illicit discharges.

Signature:

Owner

APPENDIX I: GROUNDWATER MOUNDING ANALYSIS

Mounding Analysis Calculations

Project No. 2016-002

Project: Joanna Hills Estates
Location: Joanna Road, Avon, MA

Date: March 6, 2020

Infiltration Facility Name	Recharge Rate (feet/day)	1/2 Length (ft)	1/2 Width (ft)	Duration	Specific Yield	Hydraulic Conductivity (ft/day)	Saturated Thickness (ft)	Mound Height (ft)	Separation to Groundwater (ft)
Basin A	4.82	50	8	1 Day	0.20	60	28	1.37	2 (Min.)
Basin B	4.82	32.5	35	1 Day	0.20	60	28	3.50	3.76
Basin C	4.82	38	29	1 Day	0.20	60	28	3.45	3.48
Basin D	4.82	33	20	1 Day	0.20	60	28	2.33	2.34
Basin E	4.82	45	13	1 Day	0.20	60	28	1.99	2 (Min.)

Per MA DEP Stormwater Management Regulations, groundwater mounding calculations beneath infiltration systems, within areas subject to jursidiction under the Wetlands Protection Act, are required when the bottom of an infiltration facility is within 4 feet of season high groundwater. The table above summarizes the calculated mound heights using the Hantoush Equation as provided separation to groundwater before mounding.

Length of Application: The length of the bottom of the proposed infiltration facility.

Width of Application: The width of the bottom of the proposed infiltration facility.

Recharge Rate: Based on soil texture from test pits and Soils Mapping - Loamy Sand (2.41 in/hr = 4.82 ft/day), Sandy Loam (1.02 in/hr=2.41 ft/day)

Duration of Application: The duration is 1 day to match the 100-year, 24-hour storm event

Specific Yeild: This is a value based on the soil classification of the soils found at the location of the infiltration facility (Clays=0.02, Sandy Clay=0.07. Silt=0.18, Fine Sand=0.21, Medium Sand=0.26, Coarse Sand=0.27, Gravelly Sand=0.25, Fine Gravel=0.25, Medium, Gravel=0.23, Coarse Gravel=0.22) Source: Johnson, 1967

Hydraulic Conductivity: This is a value that represents the ease with which water can move through the soil (Sandy Loams=10 ft/day, Loamy Sands=60 ft/day & Course Sands/ Gravels=200 ft/day)

<u>Initial Saturated Thickness</u>: This value represents the depth to the highest natural restrictive layer (clay or bedrock). When bedrock is not encountered during on-site test pits, this value was estimated from a Well Completion Report from the MassDEP Search Well Database. For this project the well was located at 352 Page St and the initial saturated thickness is the depth to bedrock in the well report (28 ft).

BASIN A

This spreadsheet will calculate the height of a groundwater mound beneath a stormwater infiltration basin. More information can be found in the U.S. Geological Survey Scientific Investigations Report 2010-5102 "Simulation of groundwater mounding beneath hypothetical stormwater infiltration basins".

The user must specify infiltration rate (R), specific yield (Sy), horizontal hydraulic conductivity (Kh), basin dimensions (x, y), duration of infiltration period (t), and the initial thickness of the saturated zone (hi(0), height of the water table if the bottom of the aquifer is the datum). For a square basin the half width equals the half length (x = y). For a rectangular basin, if the user wants the water-table changes perpendicular to the long side, specify x as the short dimension and y as the long dimension. Conversely, if the user wants the values perpendicular to the short side, specify y as the short dimension, x as the long dimension. All distances are from the center of the basin. Users can change the distances from the center of the basin at which water-table aquifer thickness are calculated.

Cells highlighted in yellow are values that can be changed by the user. Cells highlighted in red are output values based on user-specified inputs. The user MUST click the blue "Re-Calculate Now" button each time ANY of the user-specified inputs are changed otherwise necessary iterations to converge on the correct solution will not be done and values shown will be incorrect. Use consistent units for all input values (for example, feet and days)

		use consistent units (e.g. feet & days or inches & hours)	Conversi	on Table		
Input Values	_		inch/hou	r feet/d	lay	
4.8200	R	Recharge (infiltration) rate (feet/day)	C	.67	1.33	l e e e e e e e e e e e e e e e e e e e
0.200	Sy	Specific yield, Sy (dimensionless, between 0 and 1)				
60.00	K	Horizontal hydraulic conductivity, Kh (feet/day)*	2	.00	4.00	In the report accompanying this spreadsheet
50.000	х	1/2 length of basin (x direction, in feet)				(USGS SIR 2010-5102), vertical soil permeability
8.000	у	1/2 width of basin (y direction, in feet)	hours	days		(ft/d) is assumed to be one-tenth horizontal
1.000	t	duration of infiltration period (days)		36	1.50	hydraulic conductivity (ft/d).
28.000	hi(0)	initial thickness of saturated zone (feet)				
water Mounding, in	center of basin					
eet	•					
eet 1.365	feet					
	feet 0	Re-Calculate Now				
1.365	feet 0 20	Re-Calculate Now				
1.365 1.311	feet 0 20 40					
1.365 1.311 1.126	0 20 40 50	Re-Calculate Now Groundwater Mo	unding, ir	feet		
1.365 1.311 1.126 0.946	0 20 40 50		unding, ir	ı feet		
1.311 1.126 0.946 0.759	feet 0 20 40 50 60 70 80	Groundwater Mo	unding, ir	feet		

140

Disclaimer

1.00

120

This spreadsheet solving the Hantush (1967) equation for ground-water mounding beneath an infiltration basin is made available to the general public as a convenience for those wishing to replicate values documented in the USGS Scientific Investigations Report 2010-5102 "Groundwater mounding beneath hypothetical stormwater infiltration basins" or to calculate values based on user-specified site conditions. Any changes made to the spreadsheet (other than values identified as user-specified) after transmission from the USGS could have unintended, undesirable consequences. These consequences could include, but may not be limited to: erroneous output, numerical instabilities, and violations of underlying assumptions that are inherent in results presented in the accompanying USGS published report. The USGS assumes no responsibility for the consequences of any changes made to the spreadsheet. If changes are made to the spreadsheet, the user is responsible for documenting the changes and justifying the results and conclusions.

1 000

0.800 0.600 0.400 0.200 0.000

BASIN B

This spreadsheet will calculate the height of a groundwater mound beneath a stormwater infiltration basin. More information can be found in the U.S. Geological Survey Scientific Investigations Report 2010-5102 "Simulation of groundwater mounding beneath hypothetical stormwater infiltration basins".

The user must specify infiltration rate (R), specific yield (Sy), horizontal hydraulic conductivity (Kh), basin dimensions (x, y), duration of infiltration period (t), and the initial thickness of the saturated zone (hi(0), height of the water table if the bottom of the aquifer is the datum). For a square basin the half width equals the half length (x = y). For a rectangular basin, if the user wants the water-table changes perpendicular to the long side, specify x as the short dimension and y as the long dimension. Conversely, if the user wants the values perpendicular to the short side, specify y as the short dimension, x as the long dimension. All distances are from the center of the basin. Users can change the distances from the center of the basin at which water-table aquifer thickness are calculated.

Cells highlighted in yellow are values that can be changed by the user. Cells highlighted in red are output values based on user-specified inputs. The user MUST click the blue "Re-Calculate Now" button each time ANY of the user-specified inputs are changed otherwise necessary iterations to converge on the correct solution will not be done and values shown will be incorrect. Use consistent units for all input values (for example, feet and days)

use consistent units (e.g. feet & days or inches & hours)

Input Values inch/hour feet/day 4.8200 R Recharge (infiltration) rate (feet/day) 0.67 0.210 Specific yield, Sy (dimensionless, between 0 and 1) Sv Horizontal hydraulic conductivity, Kh (feet/day)* Κ 2.00 In the report accompanying this spreadsheet 32,500 X 1/2 length of basin (x direction, in feet) (USGS SIR 2010-5102), vertical soil 35.000 1/2 width of basin (y direction, in feet) davs hours permeability (ft/d) is assumed to be one-1.000 duration of infiltration period (days) 36 1.50 tenth horizontal hydraulic conductivity (ft/d). 28.000 hi(0) initial thickness of saturated zone (feet) maximum thickness of saturated zone (beneath center of basin at end of infiltration period) 31.497 h(max) Δh(max) maximum groundwater mounding (beneath center of basin at end of infiltration period) Ground-Distance from

Conversion Table

water center of basin
Mounding, in in x direction, in feet

3.497 0
3.231 20
2.445 40
2.070 50

60 70

80

90

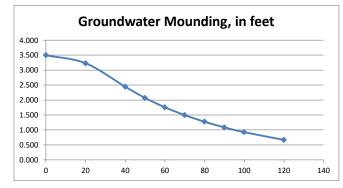
100

120

1.758

1.086





Disclaimer

This spreadsheet solving the Hantush (1967) equation for ground-water mounding beneath an infiltration basin is made available to the general public as a convenience for those wishing to replicate values documented in the USGS Scientific Investigations Report 2010-5102 "Groundwater mounding beneath hypothetical stormwater infiltration basins" or to calculate values based on user-specified site conditions. Any changes made to the spreadsheet (other than values identified as user-specified) after transmission from the USGS could have unintended, undesirable consequences. These consequences could include, but may not be limited to: erroneous output, numerical instabilities, and violations of underlying assumptions that are inherent in results presented in the accompanying USGS published report. The USGS assumes no responsibility for the consequences of any changes made to the spreadsheet. If changes are made to the spreadsheet, the user is responsible for documenting the changes and justifying the results and conclusions.

BASIN C

This spreadsheet will calculate the height of a groundwater mound beneath a stormwater infiltration basin. More information can be found in the U.S. Geological Survey Scientific Investigations Report 2010-5102 "Simulation of groundwater mounding beneath hypothetical stormwater infiltration basins".

The user must specify infiltration rate (R), specific yield (Sy), horizontal hydraulic conductivity (Kh), basin dimensions (x, y), duration of infiltration period (t), and the initial thickness of the saturated zone (hi(0), height of the water table if the bottom of the aquifer is the datum). For a square basin the half width equals the half length (x = y). For a rectangular basin, if the user wants the water-table changes perpendicular to the long side, specify x as the short dimension and y as the long dimension. Conversely, if the user wants the values perpendicular to the short side, specify y as the short dimension, x as the long dimension. All distances are from the center of the basin. Users can change the distances from the center of the basin at which water-table aquifer thickness are calculated.

Cells highlighted in yellow are values that can be changed by the user. Cells highlighted in red are output values based on user-specified inputs. The user MUST click the blue "Re-Calculate Now" button each time ANY of the user-specified inputs are changed otherwise necessary iterations to converge on the correct solution will not be done and values shown will be incorrect. Use consistent units for all input values (for example, feet and days)

		use consisten	t units (e.g. fe	et & days or	r inches & h	ours)	Conver	sion 1	Гable		
Input Values							inch/h	our	feet/da	у	
4.8200	\boldsymbol{R}	Recharge (ir	filtration) ra	te (feet/da	ay)			0.67	,	1.33	
0.200	Sy	Specific yiel	d, Sy (dimens	ionless, be	etween 0 a	nd 1)					
60.00	K	Horizontal h	ydraulic cond	ductivity, K	(h (feet/d	ay)*		2.00)	4.00	In the report accompanying this spreadsheet
38.000	x	1/2 length o	f basin (x dir	ection, in f	eet)						(USGS SIR 2010-5102), vertical soil permeability
29.000	у	1/2 width o	f basin (y dire	ction, in fe	eet)		hours		days		(ft/d) is assumed to be one-tenth horizontal
1.000	t		infiltration pe		•			36	j	1.50	hydraulic conductivity (ft/d).
28.000	hi(0)	initial thickr	ess of satura	ted zone (feet)						
	h(max) Δh(max) Distance from center of basin		nickness of sa roundwater r		•						•
Mounding, in i											
	eet										
3.447	0	Re-C	alculate	Now							
3.237	20	Inc C	aicaiace								
2.548 2.144	40 50										
1.814	60			Grou	ındwat	er Moun	nding,	in f	eet		
1.542	70		4.000								
1.314	80		3.500								
1.121	90			1							
0.957	100		3.000								
0.696	120		2.500								
			2.000		-	_					—
			1.500				_				
			1.000					-			
			0.500								
			0.000								
Disalaina			0	20	40	60	80	100	12)	140

Disclaimer

This spreadsheet solving the Hantush (1967) equation for ground-water mounding beneath an infiltration basin is made available to the general public as a convenience for those wishing to replicate values documented in the USGS Scientific Investigations Report 2010-5102 "Groundwater mounding beneath hypothetical stormwater infiltration basins" or to calculate values based on user-specified site conditions. Any changes made to the spreadsheet (other than values identified as user-specified) after transmission from the USGS could have unintended, undesirable consequences. These consequences could include, but may not be limited to: erroneous output, numerical instabilities, and violations of underlying assumptions that are inherent in results presented in the accompanying USGS published report. The USGS assumes no responsibility for the consequences of any changes made to the spreadsheet. If changes are made to the spreadsheet, the user is responsible for documenting the changes and justifying the results and conclusions.

BASIN D

This spreadsheet will calculate the height of a groundwater mound beneath a stormwater infiltration basin. More information can be found in the U.S. Geological Survey Scientific Investigations Report 2010-5102 "Simulation of groundwater mounding beneath hypothetical stormwater infiltration basins".

The user must specify infiltration rate (R), specific yield (Sy), horizontal hydraulic conductivity (Kh), basin dimensions (x, y), duration of infiltration period (t), and the initial thickness of the saturated zone (hi(0), height of the water table if the bottom of the aquifer is the datum). For a square basin the half width equals the half length (x = y). For a rectangular basin, if the user wants the water-table changes perpendicular to the long side, specify x as the short dimension and y as the long dimension. Conversely, if the user wants the values perpendicular to the short side, specify y as the short dimension, x as the long dimension. All distances are from the center of the basin. Users can change the distances from the center of the basin at which water-table aquifer thickness are calculated.

Cells highlighted in yellow are values that can be changed by the user. Cells highlighted in red are output values based on user-specified inputs. The user MUST click the blue "Re-Calculate Now" button each time ANY of the user-specified inputs are changed otherwise necessary iterations to converge on the correct solution will not be done and values shown will be incorrect. Use consistent units for all input values (for example, feet and days)

Input Values		use consistent units (e.g. feet & days or inches & hours) Conversion Table inch/hour feet/day
4.8200	R	Recharge (infiltration) rate (feet/day) 0.67 1.33
0.210	Sy	Specific yield, Sy (dimensionless, between 0 and 1)
60.00	K	Horizontal hydraulic conductivity. Kh. /feet/day)* 2.00 4.00
33.000	x	1/2 length of hosin (v. direction in feet)
20.000	y	1/2 length of basin (x direction, in feet) 1/2 width of basin (y direction, in feet) hours loss SIR 2010-5102), vertical soil permeability (ft/d) is assumed to be one-
1.000	-	duration of infiltration period (days) 36 1.50 tenth horizontal hydraulic conductivity (ft/d).
28.000	hi(0)	initial thickness of saturated zone (feet)
	,	
30.329	h(max)	maximum thickness of saturated zone (beneath center of basin at end of infiltration period)
2.329	Δh(max)	maximum groundwater mounding (beneath center of basin at end of infiltration period)
Ground-	Distance from	
water	center of basin	
Mounding, in	in x direction, in	
feet	feet	
2.329	0	Be Calculate News
2.143	20	Re-Calculate Now
1.554	40	
1.287	50	
1.078	60	Groundwater Mounding, in feet
0.909	70	2.500
0.769	80	
0.653	90	2.000
0.554	100	
0.399	120	1.500
		1.000
		0.500
		0.300

Disclaimer

This spreadsheet solving the Hantush (1967) equation for ground-water mounding beneath an infiltration basin is made available to the general public as a convenience for those wishing to replicate values documented in the USGS Scientific Investigations Report 2010-5102 "Groundwater mounding beneath hypothetical stormwater infiltration basins" or to calculate values based on user-specified site conditions. Any changes made to the spreadsheet (other than values identified as user-specified) after transmission from the USGS could have unintended, undesirable consequences. These consequences could include, but may not be limited to: erroneous output, numerical instabilities, and violations of underlying assumptions that are inherent in results presented in the accompanying USGS published report. The USGS assumes no responsibility for the consequences of any changes made to the spreadsheet. If changes are made to the spreadsheet, the user is responsible for documenting the changes and justifying the results and conclusions.

20

40

60

80

100

120

140

0.000 0

BASIN E

This spreadsheet will calculate the height of a groundwater mound beneath a stormwater infiltration basin. More information can be found in the U.S. Geological Survey Scientific Investigations Report 2010-5102 "Simulation of groundwater mounding beneath hypothetical stormwater infiltration basins".

The user must specify infiltration rate (R), specific yield (Sy), horizontal hydraulic conductivity (Kh), basin dimensions (x, y), duration of infiltration period (t), and the initial thickness of the saturated zone (hi(0), height of the water table if the bottom of the aquifer is the datum). For a square basin the half width equals the half length (x = y). For a rectangular basin, if the user wants the water-table changes perpendicular to the long side, specify x as the short dimension and y as the long dimension. Conversely, if the user wants the values perpendicular to the short side, specify y as the short dimension, x as the long dimension. All distances are from the center of the basin. Users can change the distances from the center of the basin at which water-table aquifer thickness are calculated.

Cells highlighted in yellow are values that can be changed by the user. Cells highlighted in red are output values based on user-specified inputs. The user MUST click the blue "Re-Calculate Now" button each time ANY of the user-specified inputs are changed otherwise necessary iterations to converge on the correct solution will not be done and values shown will be incorrect. Use consistent units for all input values (for example, feet and days)

		use consisten	t units (e.g. fe	et & days o ı	r inches & h	ours)	Conve	rsion T	able		
Input Values							inch/h	our	feet/da	у	
4.8200	R	Recharge (in	filtration) ra	te (feet/da	ay)			0.67		1.33	
0.200	Sy	Specific yield	d, Sy (dimens	sionless, be	etween 0 a	nd 1)					
60.00	K	Horizontal h	ydraulic con	ductivity, k	Kh (feet/da	ay)*		2.00		4.00	In the report accompanying this spreadsheet
45.000	x	1/2 length o	f basin (x dir	ection, in f	feet)						(USGS SIR 2010-5102), vertical soil permeability
13.000	У	1/2 width of	basin (y dire	ection, in f	eet)		hours		days		(ft/d) is assumed to be one-tenth horizontal
1.000	t	duration of i	infiltration po	eriod (days	s)			36		1.50	hydraulic conductivity (ft/d).
28.000	hi(0)	initial thickn	ess of satura	ted zone (feet)						
29.992 1.992	h(max) Δh(max)	maximum th maximum gi			•						•
Ground- D	istance from										
water co	enter of basin										
Mounding, in in											
	eet										
1.992	0	Re-Ca	alculate	Now							
1.898	20	1,10	inculate	11011							
1.565	40										
1.283	50			Grou	ındwat	er Mour	nding.	in fe	eet		
1.054	60			0.00							
0.881	70		2.500								_
0.743 0.630	80		2.000								
0.536	90 100		2.000	7							
0.388			1.500								
0.366	120				/						
			1.000			1					
			0.500				**	-			
			0.000 +	20	40	60	80	100	12	0	140
			Ü			-	-	100	12	-	

Disclaimer

This spreadsheet solving the Hantush (1967) equation for ground-water mounding beneath an infiltration basin is made available to the general public as a convenience for those wishing to replicate values documented in the USGS Scientific Investigations Report 2010-5102 "Groundwater mounding beneath hypothetical stormwater infiltration basins" or to calculate values based on user-specified site conditions. Any changes made to the spreadsheet (other than values identified as user-specified) after transmission from the USGS could have unintended, undesirable consequences. These consequences could include, but may not be limited to: erroneous output, numerical instabilities, and violations of underlying assumptions that are inherent in results presented in the accompanying USGS published report. The USGS assumes no responsibility for the consequences of any changes made to the spreadsheet. If changes are made to the spreadsheet, the user is responsible for documenting the changes and justifying the results and conclusions.

MassDEP

Well Completion Report

Assessors Map:

Date Issued:

WELL LOCATION

GPS North: GPS West:

Address: 352 Page Street Assessors Lot:

Sub Division: Permit Number:

Board Of Health Permit Obtained: NR

Work Performed <u>Well Type</u> <u>Drilling Method Overburden</u> <u>Drilling Method Bedrock</u>

New Well Irrigation

ADDITIONAL WELL INFORMATION

Developed: No **Disinfected:** No

City/Town: AVON

Total Well Depth: 400.00 **Fracture Enhancement:** No

Well Seal Type:

Depth to Bedrock: 28.00

PERMANENT PUMP (IF AVAILABLE)

Pump Description:

Type:

Nominal Pump Capacity:

Intake Depth:
Horsepower:
Comments:

From(ft)

<u>CASING</u> <u>SCREEN</u>

From(ft)	To(ft)	<u>Type</u>	<u>Thickness</u>	<u>Diameter</u>
0.00				
2.00 (Above Ground)	40.00	Steel		6

To(ft) Type slotsize Diameter

STATIC WATER LEVEL(ALL WELLS)

WELL SEAL / FILTER PACK / ABANDONMENT MATERIAL

Material Description Burness Date Measured Depth Polow Ground Su

From(ft)	To(ft)	Material Description	<u>Purpose</u>	Date Measured	<u>Depth Below Ground Surface</u>
				07/17/2003	17.00

WELL TEST DATA (ALL SECTIONS MANDATORY FOR PRODUCTION WELLS)

<u>Date</u>	<u>Method</u>	Yield(GPM)	Time Pumped (hrs & min)	Pumping Level (Ft. BGS)	Time To Recoover (Hrs & min)	Recovery
07/17/2003	Air Lift	4.00	04:00:00	400	00:30:00	17

OVER BURDEN

[F (ft)	- (6)	1.41		Commont		Loss / Add Drill Stem	D. 111 D. 4
	<u>From(tt)</u>	<u>To(ft)</u>	<u>Lithology</u>	Color	Comment	Water Zone	of Fluid Drop	<u>Drill Rate</u>

BEDROCK

	From(ft)	To(ft)	<u>Lithology</u>	Comment	Water Zone L	<u>Drop</u>	<u>Large</u>	Drill Rate	Rust Stain	Of Fluid	Per Ft
_											

APPENDIX J: SOIL LOGS & SIEVE ANALYSIS RESULTS

TEST PIT # 1 GRD. EL. 239.61 TEST BY: BRIAN M. DUNN TEST PIT # 13 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 17 GRD. EL. 224.24 TEST BY: BRIAN M. DUNN TEST PIT # 13 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 17 GRD. EL. 224.24 TEST BY: BRIAN M. DUNN TEST PIT # 17 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 236.17 TES GW. EL. _____232.61 WITNESSED BY: _____ DATE: 4/13/2016 MOTTLING. EL. 236.61 CERTIFIED BY: ______ DATE: 4/13/2016 MOTTLING. EL. N/A CERTIFIED BY: _____ DATE: 4/14/2016 MOTTLING. EL. N/A CERTIFIED BY: ______ DATE: 4/14/2016 MOTTLING. EL. N/A CERTIFIED BY: _____ ELEV. SURFACE SOIL SOIL SOIL COLOR 238.61 0"-12" A ORGANIC 231.61 12"-108" FILL FILL MATERIAL MOTTLES=84" PERC DEPTH____N/A___INCHES TEST PIT # 2 GRD. EL. 238.80 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 231.70 TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 231.70 TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223.52 TEST BY: BRIAN M. DUNN TEST PIT # 18 GRD. EL. 223 GW. EL. N/A WITNESSED BY: _____ DATE: 4/13/2016 MOTTLING. EL. N/A CERTIFIED BY: ______ DATE: 4/13/2016 MOTTLING. EL. 225.03 CERTIFIED BY: _____ DATE: 4/14/2016 MOTTLING. EL. 220.52 CERTIFIED BY: _____ ELEV. SURFACE SOIL SOIL SOIL DEPTH HORZ. TEXTURE COLOR 238.55 0"-3" | FILL | FILL MATERIAL 235.30 3"-42" A/B ORGANIC 233.30 42"-66" C1 | CLAY 231.80 66"-84" C2 BTM/NATURAL

WEEPING OBSERVED @ ____N/A ___ PERC RATE ___N/A __MIN/INCH STANDING OBSERVED @ ____108" PERC RATE ___8 ___MIN/INCH WEEPING OBSERVED @ ____18" PERC RATE ____8" N/A __MIN/INCH WEEPING OBSERVED @ ____18" PERC RATE ____8" N/A __MIN/INCH WEEPING OBSERVED @ ____18" PERC RATE ____8" N/A __MIN/INCH WEEPING OBSERVED @ ____18" PERC RATE ____10 ___MIN/INCH PERC DEPTH____N/A___INCHES TEST PIT # 3 GRD. EL. 239.54 TEST BY: BRIAN M. DUNN TEST PIT #7 GRD. EL. 229.31 TEST BY: BRIAN M. DUNN ELEV. SURFACE SOIL SOIL SOIL SOIL SOIL MOTTLING 239.04 0"-6" A ORGANIC 233.04 6"-78" B LOAMY SAND

PERC DEPTH___N/A__INCHES TEST PIT # 4 GRD. EL. 240.29 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.50 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 222.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.50 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 222.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.50 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST PIT # 20 GRD. EL. 223.83 TEST BY: BRIAN M. DUNN TEST BY: BRIA GW. EL. _____232.99 WITNESSED BY: _____

DATE: 4/13/2016 MOTTLING. EL. N/A CERTIFIED BY: _____ DATE: 4/13/2016 MOTTLING. EL. N/A CERTIFIED BY: _____ DATE: 4/14/2016 MOTTLING. EL. N/A CERTIFIED BY: _____ ELEV. SURFACE SOIL SOIL SOIL SOIL MOTTLING 239.79 0"-6" A ORGANIC 233.79 6"-90" B LOAMY SAND PERC DEPTH____N/A___INCHES

GW. EL. _____228.84 WITNESSED BY: _____

 ELEV.
 SURFACE
 SOIL
 SOIL
 SOIL
 SOIL
 SOIL
 MOTTLING

 235.67 0"-6" A ORGANIC 229.67 6"-88" B LOAMY SAND PERC DEPTH____N/A___INCHES

ELEV. SURFACE SOIL SOIL SOIL SOIL MOTTLING 233.76 0"-6" A ORGANIC 231.76 6"-30" B | SANDY LOAM 227.26 ₹30"-84" C1 LOAMY SAND 225,26 84"-108" C2 LOAMY SAND PERC DEPTH____35"____INCHES

GW. EL. _____233.04 WITNESSED BY: ______ GW. EL. ____224.31 WITNESSED BY: _____

ELEV. SURFACE SOIL SOIL SOIL SOIL SOIL MOTTLING 228.81 0"-6" A LOAM 226.81 6"-30" FILL FILL MATERIAL 217.81 24"-138" C1 SANDY

PERC DEPTH____N/A___INCHES GW. EL. 222.13 WITNESSED BY: _____

ELEV. SURFACE SOIL SOIL SOIL SOIL MOTTLING 229.63 0"-6" Ao ORGANIC 227.63 6"-30" B SANDY LOAM 223.13 30"-84" C1 LOAMY SAND 218.13 84"-144" C2 LOAMY SAND

ELEV. | SURFACE | SOIL | SOIL | SOIL | DEPTH | HORZ. | TEXTURE | COLOR 231.03 0"-12" A LOAM 230.03 12"-24" B LOAMY SAND 224.03 24"-96" C1 SANDY LOAM STANDING=96"

PERC DEPTH____48"____INCHES

GW. EL. _____225.26 WITNESSED BY: ______ GW. EL. ____219.06 WITNESSED BY: _____ GW. EL. ____225.03 WITNESSED BY: _____

| ELEV. | SURFACE | SOIL | SOIL | SOIL | SOIL | SOIL | MOTTLING 254.56 0"-6" A/Oi ORGANIC 252.56 6"-30" B | SANDY LOAM 248.06\30"-84" C1 | LOAMY SAND 243.06 84"-144" C2 LOAMY SAND STANDING=134" PERC DEPTH____60"___INCHES

GW. EL. _____217.23_ WITNESSED BY: _____

ELEV. SURFACE SOIL SOIL SOIL SOIL
DEPTH HORZ. TEXTURE COLOR MOTTLING 226.49 0"-6" A LOAM 223.99 6"-36" B LOAMY SAND 221.32 36"-68" C1 LOAMY SAND LARGE BOULDER WEEP=116" BOULDERS 216.15 68"-130" C2 LOAMY SAND STANDING=120"

PERC DEPTH 36" INCHES

GW. EL. 220.67 WITNESSED BY:

ELEV. SURFACE SOIL SOIL SOIL DEPTH HORZ. TEXTURE COLOR SOIL OTHER MOTTLING 229.19 0"-6" A/Oi ORGANIC 227.52 6"-26" B SANDY LOAM 15-20% DENSE 223.69 26"-72" C1 LOAMY SAND STANDING=126" COBBLES 218.85 72"-130" C2 LOAMY SAND

PERC DEPTH 48" INCHES

MASSACHUSETTS

GW. EL. <u>226.61</u> WITNESSED BY: _____ GW. EL. <u>227.56</u> WITNESSED BY: _____

ELEV. SURFACE SOIL SOIL SOIL SOIL SOIL MOTTLING 228.56 0"-12" A LOAM 227.56 12"-24" B LOAMY SAND

PERC DEPTH N/A INCHES

ELEV. SURFACE SOIL SOIL SOIL SOIL SOIL MOTTLING 231.03 0"-8" Ao ORGANIC 229.20 8"-30" B | SANDY LOAM 220.78 30"-131" C | LOAMY SAND WEEP=80" PERC DEPTH____N/A___INCHES

TEST PIT # 11 GRD. EL. 226.99 TEST BY: BRIAN M. DUNN TEST PIT # 15 GRD. EL. 226.38 TEST BY: BRIAN M. DUNN TEST PIT # 19 GRD. EL. 219.42 TEST BY: BRIAN M. DUNN GW. EL. _____218.13 WITNESSED BY: _____ DATE: 04/14/2016 MOTTLING. EL. N/A CERTIFIED BY: ______ DATE: 4/14/2016 MOTTLING. EL. N/A CERTIFIED BY: _____ DATE: 4/14/2016 MOTTLING. EL. N/A CERTIFIED BY: _____

ELEV. SURFACE SOIL SOIL SOIL SOIL SOIL MOTTLING 225.88 0"-6" A LOAM 224.88 6"-18" B LOAMY SAND 216.38 18"-120" C SANDY WEEP=99" SAND GRAVEL STANDING=120"

GW. EL. N/A WITNESSED BY: _____

ELEV. SURFACE SOIL SOIL SOIL COLOR 225.88 0"-8" Ao ORGANIC 224.88 8"-24" B SANDY LOAM 216.38 28"-96" C | SANDY LOAM

GW. EL. N/A WITNESSED BY: _____

ELEV. | SURFACE | SOIL | SOIL | SOIL | DEPTH | HORZ. | TEXTURE | COLOR 223.57 0"-8" A LOAM 222.24 8"-24" B LOAMY SAND BOULDERS FRACTURED LEDGE, 217.24 24"-84" C SANDY LOAM

PERC DEPTH____36"___INCHES

GW. EL. <u>220.52</u> WITNESSED BY: _____

223.02 0"-8" | Ao | ORGANIC 221.02 8"-24" B | SANDY LOAM 214.52 24"-108" C LOAMY SAND MOTTLES=36" STANDING=96"

GW. EL. <u>214.75</u> WITNESSED BY: _____

PERC DEPTH____56"___INCHES

ELEV. SURFACE SOIL SOIL SOIL SOIL MOTTLING 219.00 0"-5" A/OI LOAM 211.42 5"-96" B LOAMY SAND STANDING=96"

PERC DEPTH 48" INCHES

GW. EL. <u>218.33</u> WITNESSED BY: _____

ELEV. SURFACE SOIL SOIL SOIL COLOR 222.33 0"-6" | A/oi | ORGANIC 221.16 6"-20" B SANDY LOAM COBBLES, COARSE FINE SAND 214.83 20"-96" C LOAMY SAND STANDING=91"

PERC DEPTH____48"____INCHES

ROJ. MANAGER: MBL CHIEF DESIGNER: MBL REVIEWED BY: DATE 1 3/6/2020 REVISIONS PER TOWN & CONSULTANT COMMENTS TLD No. DATE DESCRIPTION REVISIONS

PERC DEPTH 56" INCHES

JOANNA HILLS LLC 32 NORFOLK AVENUE

SOUTH EASTON

HORZ: VERT.: DATUM: HORZ.: VERT.:

GRAPHIC SCALE

LAND DEVELOPMENT & PERMITTING, CORP. 770 BROADWAY SUITE 6 RAYNHAM, MA. 02767 P.508.297.2746 F.508.297.2756 EMAIL:info@MBLLandDevelopment.com WEB: www.MBLLandDevelopment.com

PERC DEPTH 56" INCHES

PERC DEPTH 48" INCHES

SITE PLANS TEST PIT LOGS CHAPTER 40B - JOANNA HILLS ESTATES ASSESSORS MAP C7, BLOCK 3 & PLOT 15

C - 7.0

PROJ. No.: 2016-002

DATE: AUGUST 8, 2019

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
247.61	2"-0"	0	ORGANIC			
246.78	0"-10"	Α	SANDY LOAM	10YR 3/2		ROOTS
245.61	10"-24"	В	SANDY LOAM	10YR 4/6		5% STONES, 10% C+G
239.78	24"-94"	С	M-C. LOAMY SAND	2.5YR 4/4		5% STONES, 10% C+G
REFUS.						BONY, TIGHT

WEEPING OBSERVED @ ____N/A PERC RATE ___8 MIN/INCH PERC DEPTH 24"-42" INCHES

TEST PIT # 22 GRD. EL. 236.21 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>N/A</u> WITNESSED BY: _____ DATE: 10/25/2018 MOTTLING. EL. N/A CERTIFIED BY: _____

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
236.04	2"-0"	0	ORGANIC			
235.21	0"-10"	Α	SANDY LOAM	10YR 3/2		ROOTS
234.21	10"-22"	В	SANDY LOAM	10YR 4/6		5% STONES, 10 % C+G
229.04	22"-84"	С	M-C. LOAMY SAND	2.5YR 4/4		5% STONES, 10 % C+G
REFUS.						BONY, TIGHT

WEEPING OBSERVED @ ____N/A ___ PERC RATE ____9 ___MIN/INCH PERC DEPTH 30"-48" INCHES

TEST PIT # 23 GRD. EL. 225.37 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>221.54</u> WITNESSED BY: _____

DATE: 10/25/2018 MOTTLING. EL. 221.54 CERTIFIED BY: ______

	ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
	224.37	0"-12"	Α	SANDY LOAM	10YR 4/2		ROOTS
	222.87	12"-30"	В	SANDY LOAM	10YR 4/6		5% STONES, 10% C+G
\otimes	217.87	30"-90"	С	MED. SANDY LOAM	2.5YR 4/4	MOTTLING @ 46"	5% STONES, 10% C+G
	REFUS.						BONY, TIGHT, MOD. STRUCTURE

WEEPING OBSERVED @ _____80" PERC RATE ____14 MIN/INCH PERC DEPTH 36"-54" INCHES

TEST PIT # 24 GRD. EL. 226.88 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>223.88</u> WITNESSED BY: _____ DATE: 10/25/2018 MOTTLING. EL. 223.88 CERTIFIED BY:

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
225.88	0"-12"	Α	SANDY LOAM	10YR 3/2		
224.54	12"-28"	В	MED. LOAMY SAND	10YR 4/4		10% C+G
215.88	28"-132"	С	M-C. LOAMY SAND	2.5YR 6/4	MOTTLING @ 36" 7.5YR 4/6	5% STONES, 10% C+G
WATER	IG OBSERVE	D @	94"	PFR	C RATE6	MIN/INCH

TEST PIT # 25 GRD. EL. 229.1 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>223.1</u> WITNESSED BY: _____

DATE: 10/25/2018 MOTTLING. EL. 223.1 CERTIFIED BY: ______

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
228.93	2"-0"	0	ORGANIC			
228.10	0"-10"	AB	SANDY LOAM	10YR 3/6		ROOTS
226.10	10"-34"	В	SANDY LOAM	10YR 4/6		10% STONES, 15% C+G
219.93	34"-108"	С	M-C. LOAMY SAND	2.5YR 4/4	MOTTLING @ 72"	10% STONES, 15% C+G, MOD. STRUCTURE
						LEDGE AT 72" ON SOUTH SIDE OF HOLE
WATFR						

TEST PIT # 26 GRD. EL. 237.85 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>N/A</u> WITNESSED BY: _____

DATE: 10/25/2018 MOTTLING. EL. N/A CERTIFIED BY:

WEEPING OBSERVED @ _____96" PERC RATE ____3 MIN/INCH

PERC DEPTH 36"-54" INCHES

PERC DEPTH 34"-52" INCHES

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
237.51	4"-0"	0	ORGANIC			
237.00	0"-6"	AB	SANDY LOAM	10YR 3/6		ROOTS
235.00	6"-32"	В	SANDY LOAM	10YR 4/6		15% STONES, 15% C+G
228.17	32"-114"	С	M-C. LOAMY SAND	2.5YR 4/4		15% STONES, 15% C+G

TEST PIT # 27 GRD. EL. 238.44 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>232.77</u> WITNESSED BY: <u>KATHLEEN WALDRON</u> DATE: <u>10/4/2019</u> MOTTLING. EL. <u>232.77</u> WADE SAUCIER

WEEPING OBSERVED @ ____N/A ___ PERC RATE ___4 ___MIN/INCH

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
237.77	0"-8"	Α	SANDY LOAM	10YR 2/2		ROOTS, ORGANICS
235.94	8"-30"	В	FINE LOAMY SAND	10YR 4/4		5-10% G+C
232.77	30"-68"	C1	FINE LOAMY SAND	10YR 4/6		5-10% G+C
228.44	68"-120"	C2	MEDIUM LOAMY SAND	2.5Y 4/2	MOTTLING @ 68"	5% G+C

TEST PIT # 28 GRD. EL. 238.13 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>235.80</u> WITNESSED BY: <u>KATHLEEN WALDRON</u> DATE: 10/4/2019 MOTTLING. EL. 235.80 WADE SAUCIER

WEEPING OBSERVED @ ____N/A ___ PERC RATE _____MIN/INCH

STANDING OBSERVED @ ____N/A ___ PERC DEPTH_____INCHES

ELEV. 237.30	SURFACE DEPTH 0"-10"	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR 10YR 2/2	SOIL MOTTLING	OTHER ROOTS,
207.00	0 10	ļ /\	2071111	10111 2/2		ORGANICS
235.88	10"-27"	В	LOAMY SAND	10YR 5/4		STONEY
233.30	27"–58"	C1	FINE SANDY LOAM	2.5Y 4/3	MOTTLING @ 28" 7.5Y 4/4	TIGHT, COMPACT MOTTLES THROUGHOUT
230.13	58"-96"	C2	LOAMY SAND	2.5Y 4/2		GRAVELLY, LOOSE

STANDING OBSERVED @ ____N/A ___ PERC DEPTH_____INCHES

TEST PIT # 29 GRD. EL. 237.83 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>235.50</u> WITNESSED BY: <u>KATHLEEN WALDRON</u> DATE: 10/4/2019 MOTTLING. EL. 235.50 WADE SAUCIER

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
236.66	0"-14"	Α	LOAM	10YR 2/2		ROOTS, ORGANICS
235.00	14"-34"	В	SANDY LOAM	10YR 5/4		ROOTS, STONEY
231.33	34"-78"	C1	FINE SANDY LOAM	2.5Y 4/3	MOTTLING @ 28" 7.5Y 4/4	TIGHT, COMPACT, MOTTLES THROUGHOUT
228.83	78"–108"	C2	LOAMY SAND	2.5Y 4/2		GRAVELLY, LOOSE
WATER						

TEST PIT # 30 GRD. EL. 239.99 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>236.66</u> WITNESSED BY: <u>KATHLEEN WALDRON</u> DATE: 10/4/2019 MOTTLING. EL. 236.66 WADE SAUCIER

WEEPING OBSERVED @ ____N/A ___ PERC RATE _____MIN/INCH

STANDING OBSERVED @ ____N/A ___ PERC DEPTH_____INCHES

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
239.07	0"-11"	Α	SANDY LOAM	10YR 3/3		ROOTS, ORGANICS
237.82	11"-26"	В	SANDY LOAM	10YR 4/4		ROOTS, STONEY
232.82	26"-86"	С	SAND	2.5Y 4/4	MOTTLING @ 40" 7.5Y 4/6	GRAVELLY, MANY STONES
WATER	G OBSERVE	.D @	66"	PFR	C RATE	MIN/INCI

TEST PIT # 31 GRD. EL. 238.82 TEST BY: TRACY L. DUARTE, PE GW. EL. 235.82 WITNESSED BY: KATHLEEN WALDRON DATE: <u>10/4/2019</u> MOTTLING. EL. <u>235.82</u> WADE SAUCIER

STANDING OBSERVED @ _____84" PERC DEPTH_____INCHES

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
238.07	0"-9"	Α	SANDY LOAM	10YR 3/3		ROOTS, ORGANICS
236.65	9"-26"	В	LOAMY SAND	10YR 5/6		
232.15	26"-80"	С	SAND	2.5Y 5/3	MOTTLING @ 36" 7.5Y 4/6	GRAVELLY, MANY STONES

TEST PIT # 32 GRD. EL. 238.88 TEST BY: TRACY L. DUARTE, PE GW. EL. 235.55 WITNESSED BY: KATHLEEN WALDRON DATE: 10/4/2019 MOTTLING. EL. 235.55 WADE SAUCIER

STANDING OBSERVED @ ____N/A PERC DEPTH_____INCHES

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
238.05	0"-10"	Α	SANDY LOAM	10YR 3/3		ROOTS, ORGANICS
237.38	10"-18"	В	LOAMY SAND	10YR 5/6		ROOTS, STONEY
235.55	18"-40"	C1	FINE SAND	2.5Y 5/3		
230.21	40"-104"	C2	SAND	2.5Y 5/3	MOTTLING @ 40" 7.5Y 4/6	STONEY
WATER WEEPIN(G OBSERVE		101"	PER	C RATE	MIN/INC
STANDIN	IG OBSERV	ED @ _	N/A	PER	C DEPTH	INCHES

MASSACHUSETTS

TEST PIT # 33 GRD. EL. 239.43 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>236.18</u> WITNESSED BY: <u>KATHLEEN WALDRON</u> DATE: 10/4/2019 MOTTLING. EL. 236.18 WADE SAUCIER

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
238.43	0"-12"	Α	LOAM	10YR 3/2		ROOTS, ORGANICS
237.10	12"-28"	В	SANDY LOAM	10YR 5/6		STONEY
230.93	28"-102"	С	LOAMY SAND	2.5Y 5/3	MOTTLING @ 39"	GRAVELLY, LOOSE
WATER WEEPING	G OBSERVE		N/A	PER	C RATE	MIN/INCH
STANDIN	IG OBSERV	ED @ _	N/A	PER	C DEPTH	INCHES

TEST PIT # 34 GRD. EL. 238.44 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>235.77</u> WITNESSED BY: <u>KATHLEEN WALDRON</u> DATE: 10/4/2019 MOTTLING. EL. 235.77 WADE SAUCIER

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
237.61	0"-10"	Α	LOAM	10YR 3/2		ROOTS, ORGANICS
236.11	10"-28"	В	SANDY LOAM	10YR 5/6		
234.61	28"-46"	C1	SANDY LOAM	2.5Y 5/4	MOTTLING @ 32"	
230.11	46"-100"	C2	LOAMY SAND	2.5Y 5/2		GRAVELLY, LOOSE
WATER WEEPIN(G OBSERVE	.D @	N/A	PER	C RATE	MIN/INCH

TEST PIT # 35 GRD. EL. 238.44 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>235.94</u> WITNESSED BY: <u>KATHLEEN WALDRON</u> WADE SAUCIER DATE: 10/4/2019 MOTTLING. EL. 235.94

STANDING OBSERVED @ ____N/A ___ PERC DEPTH_____

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
237.69	0"-9"	А	LOAM	10YR 3/2		ROOTS, ORGANICS
236.36	9"-25"	В	SANDY LOAM	10YR 5/6		
234.86	25"-43"	C1	SANDY LOAM	2.5Y 5/4	MOTTLING @ 30"	
230.44	43"-96"	C2	LOAMY SAND	2.5Y 5/2		GRAVELLY, LOOSE

_ MIN/INCH

TEST PIT # 36 GRD. EL. 223..69 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>220.52</u> WITNESSED BY: <u>KATHLEEN WALDRON</u> DATE: 10/4/2019 MOTTLING. EL. 220.52 WADE SAUCIER

WEEPING OBSERVED @ ____N/A PERC RATE _____

STANDING OBSERVED @ ____N/A PERC DEPTH_____

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
222.86	0"-10"	Α	SANDY LOAM	10YR 3/2		ROOTS
221.36	10"-28"	В	LOAMY SAND	10YR 5/6		STONEY
215.69	28"-96"	С	LOAMY SAND	2.5Y 5/3	MOTTLING @ 38" 10YR 5/4	GRAVELLY, LOOSE
WATER WEEPING	G OBSERVE	 [D @	N/A	PER	C RATE	MIN/INCH
STANDIN	IG OBSERV	ED @	N/A		C DEPTH	,

TEST PIT # 37 GRD. EL. 220.40 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>215.32</u> WITNESSED BY: <u>KATHLEEN WALDRON</u> DATE: 10/4/2019 MOTTLING. EL. 215.32 WADE SAUCIER

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
219.40	0"-12"	А	SANDY LOAM	10YR 3/2		ROOTS
218.90	12"-18"	В	LOAMY SAND	10YR 5/6		STONEY
212.98	18"-89"	С	LOAMY SAND	2.5Y 5/3	MOTTLING @ 61"	GRAVELLY, LOOSE
WATER WEEPIN(G OBSERVE	.D @ _	N/A	PER	C RATE	MIN/INCH

TEST PIT # 38 GRD. EL. 218.40 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>215.07</u> WITNESSED BY: <u>KATHLEEN WALDRON</u>

DATE: 10/4/2019 MOTTLING. EL. 215.07

STANDING OBSERVED @ ____N/A ___ PERC DEPTH_____INCHES

WADE SAUCIER

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
217.57	0"-10"	A	SANDY LOAM	10YR 3/2		ROOTS
216.23	10"-26"	В	SANDY LOAM	10YR 5/6		
211.57	26"-82"	С	LOAMY SAND	2.5Y 5/3	MOTTLING @ 40"	GRAVELLY, LOOSE

WEEPING OBSERVED @ ____N/A ___ PERC RATE _____MIN/INCH STANDING OBSERVED @ ____N/A ___ PERC DEPTH_____INCHES

TEST PIT # 39 GRD. EL. 226.04 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>221.04</u> WITNESSED BY: <u>KATHLEEN WALDRON</u> DATE: 10/9/2019 MOTTLING. EL. 221.04 WADE SAUCIER

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
225.04	0"-12"	Α	SANDY LOAM	10YR 3/2		ROOTS
223.87	12"-26"	В	LOAMY SAND	10YR 5/6		5% STONES, 10% C+G
223.37	26"-32"	ВС	COARSE LOAMY SAND	2.5Y 5/6	MOTTLING @ 60"	5% STONES, 10% C+G
216.21	32"-118"	С	COARSE LOAMY SAND	2.5Y 6/3		5% STONES, 10% C+G

WEEPING OBSERVED @ ____N/A ___ PERC RATE _____MIN/INCH STANDING OBSERVED @ ____N/A ___ PERC DEPTH_____INCHES

TEST PIT # 40 GRD. EL. 224.90 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>222.40</u> WITNESSED BY: <u>KATHLEEN WALDRON</u> DATE: 10/9/2019 MOTTLING. EL. 222.40 WADE SAUCIER

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
224.40	0"-6"	A	SANDY LOAM	10YR 3/2		ROOTS
223.4	6"-18"	В	SANDY LOAM	10YR 5/6		5% STONES, 10% C+G
216.73	18"-98"	С	SANDY LOAM	2.5Y 6/4	MOTTLING @ 30"	GRAVELLY, FIRM IN PLACE

WEEPING OBSERVED @ ____N/A PERC RAIE _____MIN/INCH STANDING OBSERVED @ ____N/A ___ PERC DEPTH_____INCHES

PROJ. MANAGER: MBL CHIEF DESIGNER: MBL REVIEWED BY: DATE 1 3/6/2020 REVISIONS PER TOWN & CONSULTANT COMMENTS TLD No. DATE DESCRIPTION REVISIONS

PERC DEPTH 34"-52" INCHES

PREPARED FOR JOANNA HILLS LLC

SOUTH EASTON

32 NORFOLK AVENUE



GRAPHIC SCALE

SITE PLANS TEST PIT LOGS CHAPTER 40B - JOANNA HILLS ESTATES ASSESSORS MAP C7, BLOCK 3 & PLOT 15

MASSACHUSETTS

C-7.1

PROJ. No.: 2016-002

DATE: AUGUST 8, 2019

INDICATES PERC

TEST

INDICATES OBSERVED
GROUNDWATER

TEST PIT # 41 GRD. EL. 222.41 TEST BY: TRACY L. DUARTE, PE GW. EL. 219.91 WITNESSED BY: KATHLEEN WALDRON DATE: 10/9/2019 MOTTLING. EL. 219.91 WADE SAUCIER

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
221.58	0"-10"	Α	SANDY LOAM	10YR 3/2		ROOTS, ORGANICS
220.74	10"-20"	В	LOAMY SAND	10YR 5/6		
219.41	20"-36"	BC	COARSE LOAMY SAND	2.5Y 6/6	MOTTLING @ 30"	
213.74	36"-104"	С	COARSE LOAMY SAND	2.5Y 5/2		GRAVELLY, FIRM IN PLACE

WEEPING OBSERVED @ ____N/A ___ PERC RATE _____MIN/INCH STANDING OBSERVED @ ____N/A ___ PERC DEPTH_____INCHES

TEST PIT # 42 GRD. EL. 224.41 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>221.74</u> WITNESSED BY: <u>KATHLEEN WALDRON</u> DATE: 10/9/2019 MOTTLING. EL. 221.74 WADE SAUCIER

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
223.74	0"-8"	Α	SANDY LOAM	10YR 3/2		
222.58	8"-22"	В	LOAMY SAND	10YR 5/6		
217.16	22"-87"	С	COARSE LOAMY SAND	2.5Y 6/2	MOTTLING @ 32"	GRAVELLY, FIRM IN PLACE

WEEPING OBSERVED @ ____N/A ___ PERC RATE _____MIN/INCH STANDING OBSERVED @ ____N/A ___ PERC DEPTH_____INCHES

TEST PIT # 43 GRD. EL. 221.93 TEST BY: TRACY L. DUARTE, PE GW. EL. <u>219.26</u> WITNESSED BY: <u>KATHLEEN WALDRON</u> DATE: 10/9/2019 MOTTLING. EL. 219.26 WADE SAUCIER

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
221.10	0"-10"	Α	SANDY LOAM	10YR 3/2		ROOTS, ORGANICS
220.26	10"-20"	В	LOAMY SAND	10YR 5/6		
219.26	20"-32"	ВС	LOAMY SAND	2.5Y 5/8		
214.68	32"-87"	С	COARSE LOAMY SAND	2.5Y 6/2	MOTTLING @ 32" 5YR 5/8	GRAVELLY, FIRM IN PLACE

WATER
WEEPING OBSERVED @ ____N/A PERC RATE _____ STANDING OBSERVED @ ____N/A PERC DEPTH_____

__MIN/INCH

TEST PIT # 44 GRD. EL. 222.41 TEST BY: TRACY L. DUARTE, PE
GW. EL. 220.33 WITNESSED BY: KATHLEEN WALDRON DATE: 10/9/2019 MOTTLING. EL. 220.33 WADE SAUCIER

ELEV.	SURFACE DEPTH	SOIL HORZ.	SOIL TEXTURE	SOIL COLOR	SOIL MOTTLING	OTHER
221.66	0"-9"	А	SANDY LOAM	10YR 3/2		
220.41	9"-24"	В	LOAMY SAND	10YR 5/6		
214.58	24"-94"	С	COARSE LOAMY SAND	2.5Y 6/2	MOTTLING @ 25"	GRAVELLY, FIRM IN PLACE

No. DATE

WEEPING OBSERVED @ ____N/A PERC RATE _____ STANDING OBSERVED @ ____N/A PERC DEPTH_____

REVISIONS

PROJ. MANAGER: MBL SEAL PREPARED FOR CHIEF DESIGNER: MBL REVIEWED BY: JOANNA HILLS LLC 32 NORFOLK AVENUE 1 3/6/2020 REVISIONS PER TOWN & CONSULTANT COMMENTS TLD DESCRIPTION

SOUTH EASTON

MASSACHUSETTS

HORZ.: LAND DEVELOPMENT & PERMITTING, CORP.

LAND DEVELOPMENT, TRANSPORTATION AND ENVIRONMENTAL SOLUTIONS

770 BROADWAY SUITE 6

RAYNHAM, MA. 02767

P.508.297.2746 F.508.297.2756

EMAIL:info@MBLLandDevelopment.com

WEB: www.MBLLandDevelopment.com VERT.: DATUM: HORZ.: GRAPHIC SCALE

SITE PLANS TEST PIT LOGS CHAPTER 40B - JOANNA HILLS ESTATES ASSESSORS MAP C7, BLOCK 3 & PLOT 15

MASSACHUSETTS

AVON

C-7.2

PROJ. No.: 2016-002

DATE: AUGUST 8, 2019



165 East Grove Street Middleborough, MA 02346

Tel: 508-946-9231

www.outback-eng.com

Fax: 508-947-8873

JOB#:

OEL-1048-H-Pit 27

LOCATION: Pit 27 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

<u>DATE:</u> 10/17/2019 <u>TEST BY:</u> M.S. <u>CHECK BY:</u> J.A.Y.

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sieve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	100.00%
3/8"	9.5	67.28%
# 4	4.75	55.65%
# 10	2	45.29%

Sieve Analysis of Material Passing #10

Sieve

Sleve_No	Particle Dlameter (mm):	Percent Passing (%):	
# 10	2	100.00%	
# 20	0.85	78.19%	
# 50	0.3	48.69%	SAND
# 100	0.15	31.74%	المحاد
# 140	0.106	23.70%	
# 270	0.053	14.63%	

Hydrometer Analysis of Material Passing

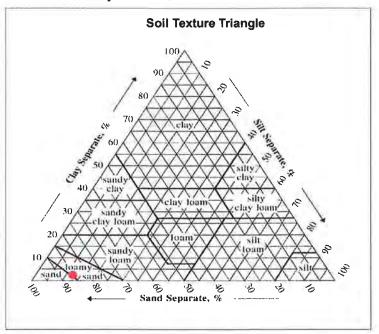
		#2/U SIBVE
	13,86%	0.05735
	13.09%	0.04146
	11.42%	0.03066
	9.24%	0.02286
SILT	7.83%	0.01669
	6.29%	0.01219
	4.75%	0.00918
1	4.36%	0.00772
	2.57%	0.00408
CLAY	2.18%	0.00330
TOLA	2.05%	0.00142

Triangle Classification of Material Passing the #10 Sleve

% Sand 85.37%

% Slit 12.45%

% Clay 2.18%



USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1. Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



JOB #: OEL-1048-H-Pit 27

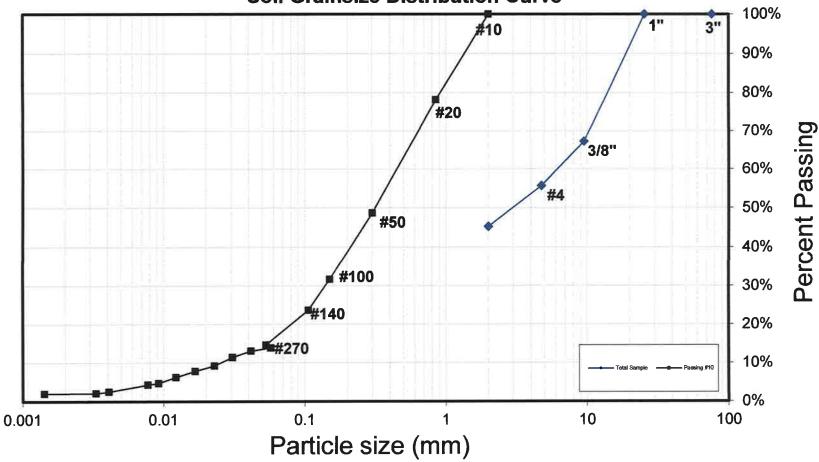
LOCATION: Pit 27 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/17/2019

TEST BY: M.S.
CHECK BY: J.A.Y.







165 East Grove Street Middleborough, MA 02346

Tel: 508-946-9231

www.outback-eng.com

Fax: 508-947-8873

JOB#: OEL-1048-H-Pit 28

CLIENT:

LOCATION: Pit 28 - Joanna Rd. Avon, MA Stonebridge Homes

DATE: 10/21/2019 TEST BY: M.S. CHECK BY: J.A.Y.

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sieve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	100.00%
3/8"	9.5	90.58%
# 4	4.75	83.28%
# 10	2	72.72%

Sleve Analysis of Material Passing #10 Sleve

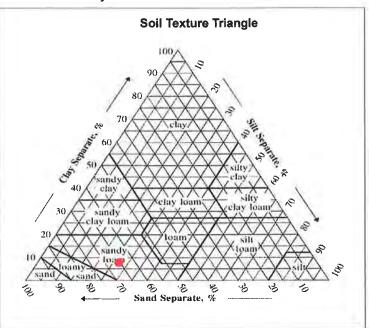
Sieve_No	Particle Diameter (mm):	Percent Passing (%):	
# 10	2	100.00%	
# 20	0.85	83.44%	
# 50	0.3	57.25%	SAND
# 100	0.15	47.09%	
# 140	0.106	40.42%	
# 270	0.053	34.65%	

Hydrometer Analysis of Material Passing #270 Clave

		#2/U Sieve	
	35.03%	0.04293	
	32.74%	0.03096	
	28.17%	0.02273	
	24.36%	0.01655	
SILT	21.32%	0.01236	
	18.27%	0.00892	
	13.70%	0.00650	
	11.42%	0.00467	
	8.75%	0.00346	
CLAY	7.61%	0.00273	
TOLAY	5.33%	0.00081	

Triangle Classification of Material Passing the #10 Sieve % Sand 65.35%

% SIIt 27.04% % Clay 7.61%



USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1. Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



Tel: 508-946-9231 Fax: 508-947-8873 JOB #: OEL-1048-H-Pit 28

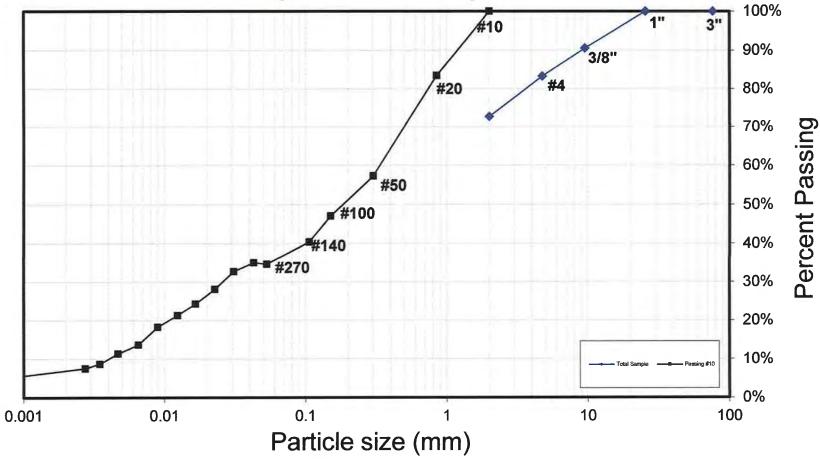
LOCATION: Pit 28 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/21/2019

TEST BY: M.S. CHECK BY: J.A.Y.







165 East Grove Street Middleborough, MA 02346

Tel: 508-946-9231

www.outback-eng.com

Fax: 508-947-8873

JOB #: OEL-1048-H-Pit 29

CLIENT:

LOCATION: Pit 29 - Joanna Rd. Avon, MA

Stonebridge Homes

DATE: 10/11/2019 TEST BY: M.S.

CHECK BY: J.A.Y.

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sieve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	100.00%
3/8"	9.5	92.35%
# 4	4.75	85.81%
# 10	2	72.30%

Sieve Analysis of Material Passing #10 Sieve

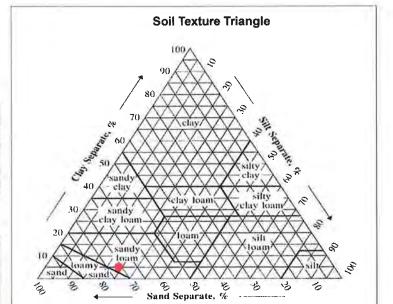
Olova			
	Percent Passing (%):	Particle Diameter (mm):	Sleve_No
	100.00%	2	# 10
	79.76%	0.85	# 20
SAND	53.85%	0.3	# 50
	40.51%	0.15	# 100
	34.15%	0.106	# 140
	28.95%	0.053	# 270
	28.95%	0.053	# 270

Hydrometer Analysis of Material Passing

		#270 Sleve	
	28.54%	0.04264	
	25.20%	0.03126	L.,
	20.65%	0.02313	
	17.61%	0.01683	
SILT	15.18%	0.01255	
	11.54%	0.00915	
	9.71%	0.00657	
1	7.28%	0.00473	
	5.77%	0.00271	
CLAY	4.85%	0.00143	
COAT	2.56%	0.00141	

Triangle Classification of Material Passing the #10 Sleve

% Sand	71.05%
% Silt	24.10%
% Clay	4.85%



USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soli Analysis, Part 1. Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



Tel: 508-946-9231

Middleboro, MA 02340 Fax: 508-947-8873 JOB #: OEL-1048-H-Pit 29

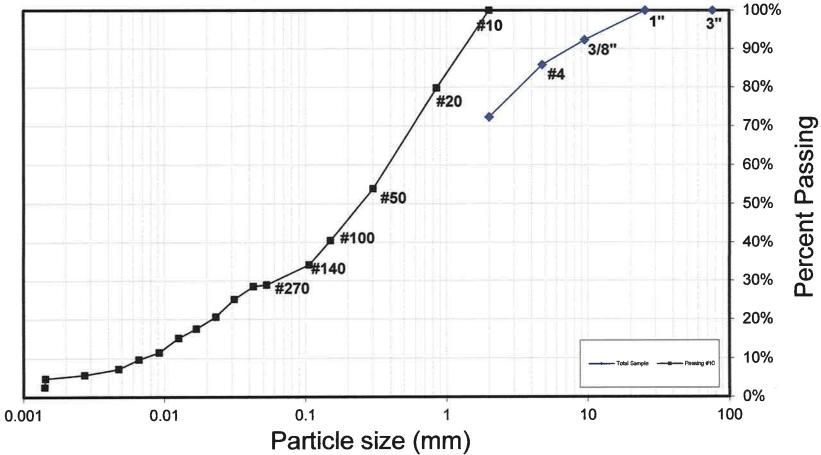
LOCATION: Pit 29 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/11/2019

TEST BY: M.S.
CHECK BY: J.A.Y.







Tel: 508-946-9231

www.outback-eng.com

Fax: 508-947-8873

www.outbac

<u>DATE:</u> 10/14/2019 <u>TE\$T BY:</u> M.S.

LOCATION: Pit 30 - Joanna Rd. Avon, MA
CLIENT: Stonebridge Homes

JOB #:

OEL-1048-H-Pit 30

CHECK BY: J.A.Y.

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sleve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
- 1"	25.4	100.00%
3/8"	9.5	74.27%
# 4	4.75	60.87%
# 10	2	43.58%

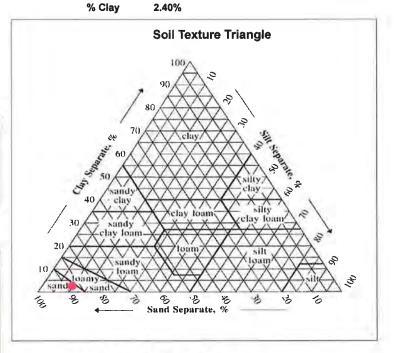
Sieve Analysis of Material Passing #10 Sieve

Sleve_No	Particle Diameter (mm):	Percent Passing (%):	
# 10	2	100.00%	
# 20	0.85	62.08%	
# 50	0.3	31.64%	SAND
# 100	0.15	21.49%	
# 140	0.106	17.68%	
# 270	0.053	12.23%	

Hydrometer Analysis of Material Passing

-	÷	#2/0 Sieve	
	12.54%	0.04264	
	10.67%	0.03156	
	9.34%	0.02300	
	7.74%	0.01683	
SILT	6.67%	0.01255	
	5.60%	0.00906	
	4.27%	0.00657	
	3.73%	0.00506	
	2.53%	0.00301	
CLAY	2.40%	0.00253	
CLAT	2.00%	0.00080	

Triangle Classification of Material Passing the #10 Sieve % Sand 87.77% % Silt 9.83%



USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1. Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



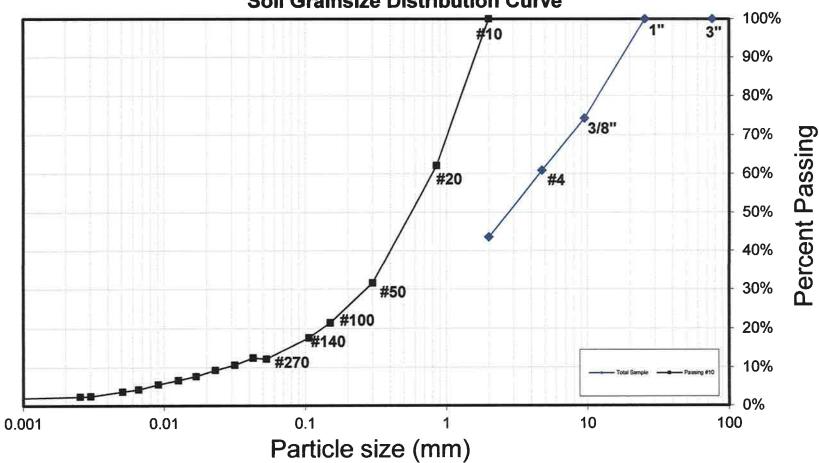
JOB #: OEL-1048-H-Pit 30

LOCATION: Pit 30 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/14/2019







Tel: 508-946-9231

OEL-1048-H-Pit 31

Stonebridge Homes

LOCATION: Pit 31 - Joanna Rd. Avon, MA

JOB#:

CLIENT:

www.outback-eng.com

DATE: 10/24/2019

TEST BY: M.S.
CHECK BY: J.A.Y.

Fax: 608-947-8873

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sleve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	100.00%
3/8"	9.5	71.36%
#4	4.75	57.26%
# 10	2	44.23%

Sleve Analysis of Material Passing #10

Sieve

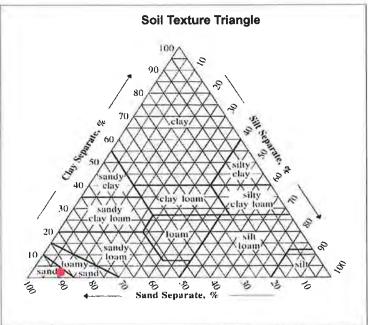
Sleve_No	Particle Diameter (mm):	Percent Passing (%):	
# 10	2	100.00%	
# 20	0.85	66.83%	
# 50	0.3	34.06%	SAND
# 100	0.15	25.23%	
# 140	0.106	21.47%	
# 270	0.053	11.94%	

Hydrometer Analysis of Material Passing

-		#2/U SIBVB	
	12.15%	0.06091	
	11.22%	0.04407	
	9.21%	0.03262	
	7.34%	0.02399	
SILT	4.81%	0.01781	
	4.27%	0.01313	
	3.74%	0.00938	
	2.94%	0.00672	
	2.67%	0.00511	
CLAY	2.27%	0.00276	
TO LAT	2.13%	0.00139	

Triangle Classification of Material Passing the #10 Sieve

% Sand 88.06% % Slit 9.67% % Clay 2.27%



USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1. Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



JOB #: OEL-1048-H-Pit 31

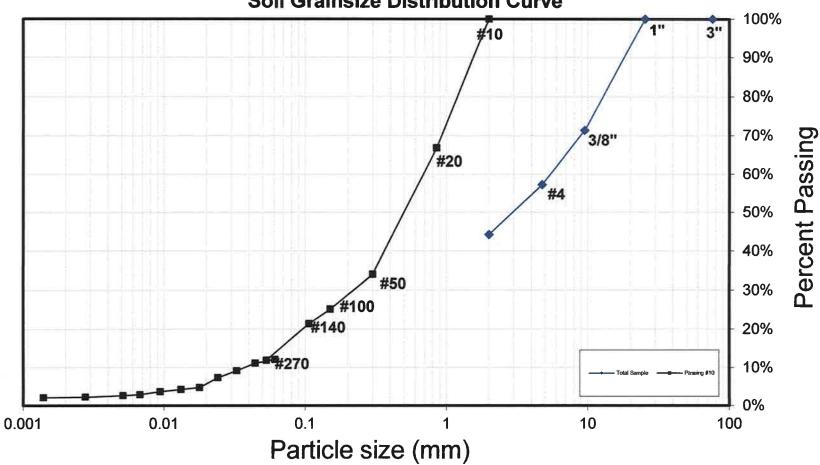
LOCATION: Pit 31 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/24/2019

TEST BY: M.S.
CHECK BY: J.A.Y.

Soil Grainsize Distribution Curve





Tel: 508-946-9231

www.outback-eng.com

Fax: 508-947-8873

JOB #: OEL-1048-H-Pit 32 LOCATION: Pit 32 - Joanna Rd. Avon, MA <u>DATE:</u> 10/14/2019 <u>TEST BY:</u> M.S. <u>CHECK BY:</u> J.A.Y.

CLIENT: Stonebridge Homes

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sieve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	100.00%
3/8"	9.5	81.29%
# 4	4.75	71.19%
# 10	2	61,17%

Sieve Analysis of Material Passing #10

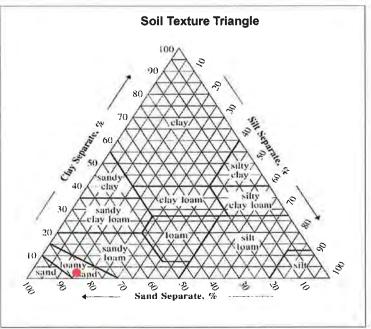
Sieve_No	Particle Diameter (mm):	Percent Passing (%):	
# 10	2	100.00%	
# 20	0.85	84.25%	
# 50	0.3	59.76%	SAND
# 100	0.15	40.58%	JANE
# 140	0.106	31.88%	
# 270	0.053	17.78%	

Hydrometer Analysis of Material Passing #270 Sieve

_		TETU SIGVO	
	16.18%	0.03993	
	13.87%	0.02994	
	11.56%	0.02231	
	9.10%	0.01659	
SILT	7.37%	0.01252	
	5.63%	0.00913	
	4.04%	0.00663	
1	3.18%	0.00475	
	2.45%	0.00294	
CLAY	2.31%	0.00252	
TCLAY	1.73%	0.00080	1 1 1

Triangle Classification of Material Passing the #10 Sleve

% Sand 82.22% % Silt 15.47% % Clay 2.31%



USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1, Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



Tel: 508-946-9231

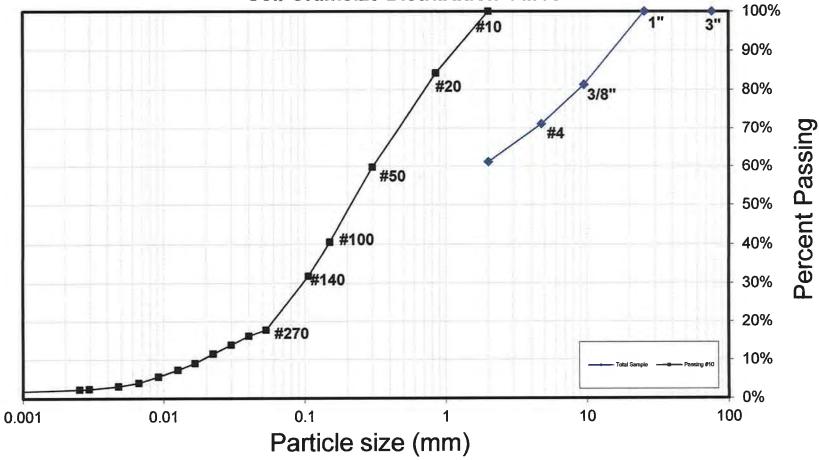
Fax: 508-947-8873

JOB #: OEL-1048-H-Pit 32 LOCATION: Pit 32 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/14/2019







Tel: 508-946-9231

www.outback-eng.com

Fax: 508-947-8873

JOB #: OEL-1048-H-Pit 33

LOCATION: Pit 33 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/22/2019 TEST BY: M.S. CHECK BY: J.A.Y.

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sieve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	100.00%
3/8"	9.5	75.10%
#4	4.75	66.25%
# 10	2	56.52%

Sieve Analysis of Material Passing #10

Sieve

Sieve_No	Particle Diameter (mm):	Percent Passing (%):	
# 10	2	100.00%	
# 20	0.85	83.15%	
# 50	0.3	60.50%	SAND
# 100	0.15	43.38%	
# 140	0.106	32.74%	
# 270	0.053	19.88%	

Hydrometer Analysis of Material Passing

#270 Sieve

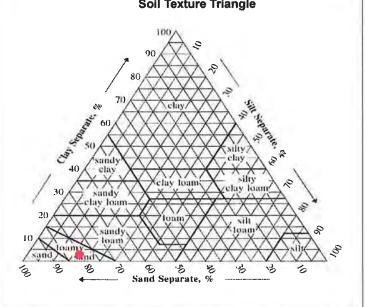
	0.05377	19.90%	
	0.03946	18.46%	
	0.02994	15.41%	
	0.02225	13.00%	
	0.01669	9.79%	SILT
	0.01242	8.67%	
	0.00908	6.58%	
	0.00654	4.81%	
	0.00460	3.85%	
	0.00278	2.73%	CLAY
+	0.00140	2.25%	CLA

Triangle Classification of Material

Passing the #10 Sieve

% Sand 80.12% % Silt 17.15% % Clay 2.73%

Soil Texture Triangle



USDA Soil Textural Triangle

Jason Youngquist, P.E Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1. Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



Tel: 508-946-9231

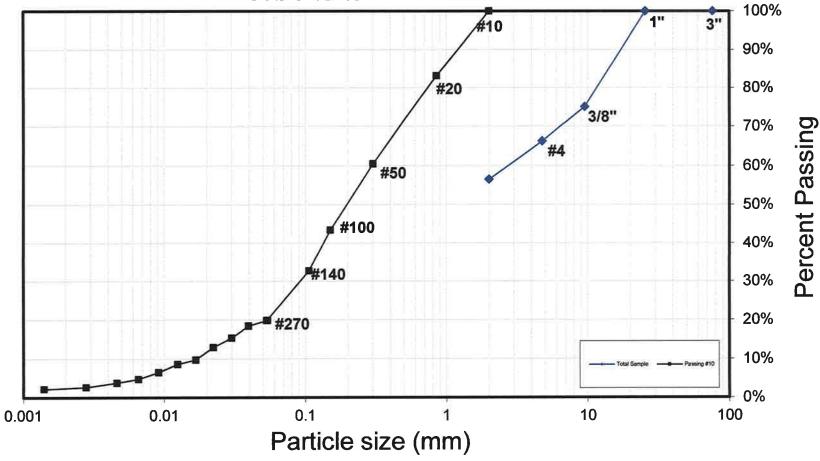
402346 Middleboro, MA 4733-308-947 JOB #: OEL-1048-H-Pit 33

LOCATION: Pit 33 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/22/2019







Tel: 508-946-9231

www.outback-eng.com

Fax: 508-947-8873

JOB #: OEL-1048-H-Pit 34

LOCATION: Pit 34 - Joanna Rd. Avon, MA
CLIENT: Stonebridge Homes

<u>DATE:</u> 10/10/2019 <u>TEST BY:</u> M.S. <u>CHECK BY:</u> J.A.Y.

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sieve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	91.43%
3/8"	9.5	63.51%
# 4	4.75	52.89%
# 10	2	43.11%

Sleve Analysis of Material Passing #10

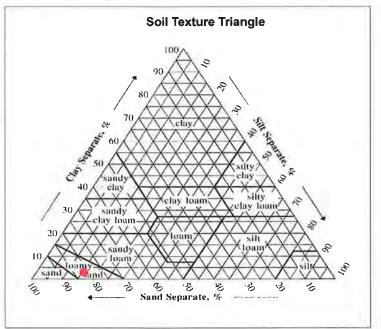
Sleve_No	Particle Diameter (mm):	Percent Pessing (%):	
# 10	2	100.00%	
# 20	0.85	81.71%	
# 50	0.3	55.32%	SAND
# 100	0.15	35.95%	
# 140	0.106	27.95%	
# 270	0.053	18.52%	

Hydrometer Analysis of Material Passing

		#2/0 51979	
	18.12%	0.04293	
	14.97%	0.03195	
	11.62%	0.02373	
	8.66%	0.01746	
SILT	7.48%	0.01294	
	5.91%	0.00933	
	4.72%	0.00669	
	3.74%	0.00479	
	3.35%	0.00304	
CLAY	2.75%	0.00265	
TODAY	2.56%	0.00141	

Triangle Classification of Material Passing the #10 Sieve

% Sand 81.48% % Silt 15.77% % Clay 2.75%



USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1. Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



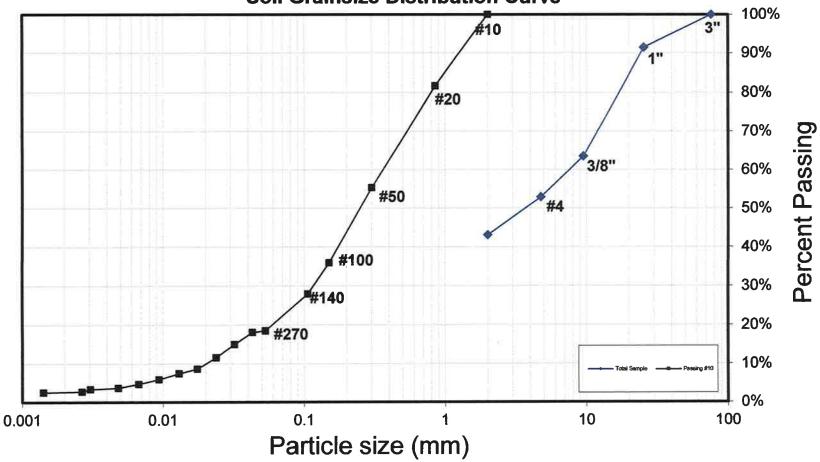
JOB #: OEL-1048-H-Pit 34

LOCATION: Pit 34 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/10/2019







Tel: 508-946-9231

www.outback-eng.com

Fax: 508-947-8873

JOB #:

CLIENT:

OEL-1048-H-Pit 35

LOCATION: Pit 35 - Joanna Rd. Avon, MA Stonebridge Homes

DATE: 10/23/2019 TEST BY: M.S.

CHECK BY: J.A.Y.

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sleve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	100.00%
3/8"	9.5	81.70%
# 4	4.75	69.80%
# 10	2	58.48%

Sieve Analysis of Material Passing #10 Sleve

Sieve_No	Particle Dlameter (mm):	Percent Passing (%):	
# 10	2	100.00%	
# 20	0.85	82.25%	
# 50	0.3	60.41%	SAND
# 100	0.15	44.65%	Jane
# 140	0.106	36.18%	
# 270	0.053	23.33%	

Hydrometer Analysis of Material Passing #270 Sleve

_		WEI U SIEVE	
	22.30%	0.05469	
	20.81%	0.03993	
	17.47%	0.03015	
	14.12%	0.02259	
SILT	10.96%	0.01678	
	8.73%	0.01265	
	5.94%	0.00872	
	4.64%	0.00668	
	3.34%	0.00425	
CLAY	2.97%	0.00373	
TOLAY	2.23%	0.00281	

Triangle Classification of Material Passing the #10 Sieve

% Sand 76.67% % Silt 20.36% % Clay 2.97%

Soil Texture Triangle sand clay loam

USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1, Physical and Mineralogical Methods, 2nd Edition es specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



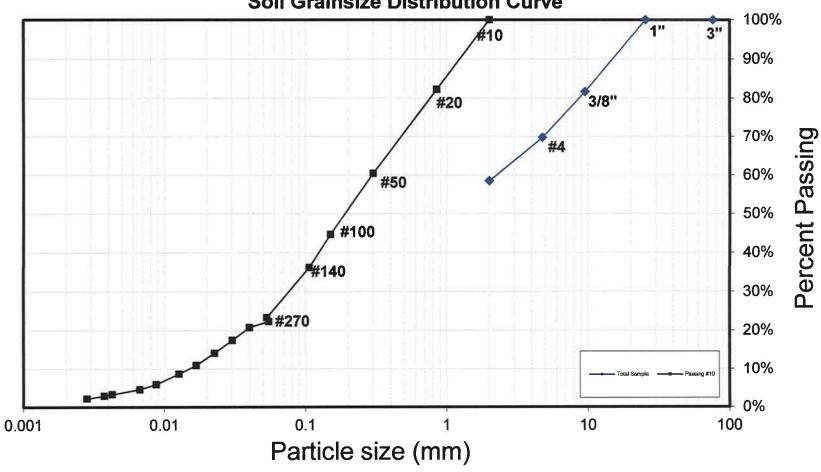
JOB #: OEL-1048-H-Pit 35

LOCATION: Pit 35 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/23/2019







Tel: 508-946-9231

www.outback-eng.com

Fax: 508-947-8873

JOB #: OEL-1048-H-Plt 36

LOCATION: Pit 36 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/11/2019 TEST BY: M.S.

CHECK BY: J.A.Y.

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sieve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	100.00%
3/8"	9.5	77.43%
# 4	4.75	65.62%
# 10	2	54.52%

Sleve Analysis of Material Passing #10

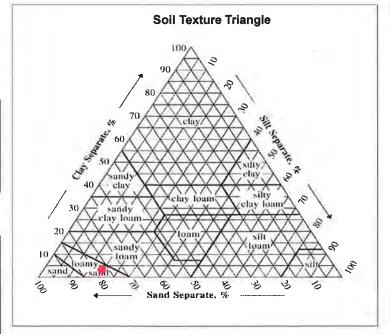
	21040		
Sieve_No	Particle Diameter (mm):	Percent Passing (%):	
# 10	2	100.00%	
# 20	0.85	81.75%	
# 50	0.3	56.98%	SANE
# 100	0.15	41.34%	J 4411
# 140	0.106	34.08%	
# 270	0.053	22.71%	

Hydrometer Analysis of Material Passing #270 Sleve

_		WE10 01040
	21.98%	0.05469
	20.15%	0.04024
	17.59%	0.02994
	14.29%	0.02245
SILT	10.81%	0.01678
	8.79%	0.01262
	6.96%	0.00915
	5.49%	0.00660
	4.21%	0.00495
CLAY	3.11%	0.00276
TCLAY	2.38%	0.00144

Triangle Classification of Material Passing the #10 Sieve

% Sand 77.29% % Silt 19.60% % Clay 3.11%



USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1, Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



Tel: 508-946-9231 Fax: 508-947-8873 JOB #: OEL-1048-H-Pit 36

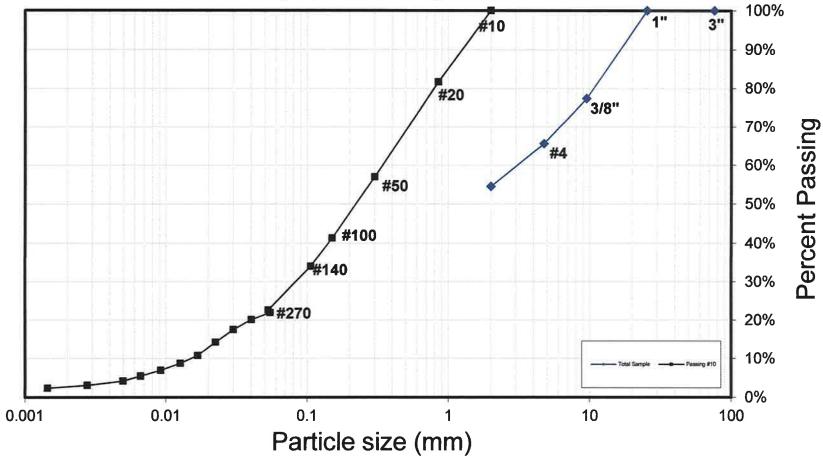
LOCATION: Pit 36 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/11/2019

TEST BY: M.S. CHECK BY: J.A.Y.

Soil Grainsize Distribution Curve





Tel: 508-946-9231

www.outback-eng.com

JOB #: OEL-1048-H-Pit 37 LOCATION: Pit 37 - Joanna Rd. Avon, MA Stonebridge Homes

CLIENT:

DATE: 10/24/2019 TEST BY: M.S.

CHECK BY: J.A.Y.

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sleve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	100.00%
3/8"	9.5	84.12%
#4	4.75	71.83%
# 10	2	60.00%

Sieve Analysis of Material Passing #10 Sleve

Sieve_No	Particle Dlameter (mm):	Percent Passing (%):	
# 10	2	100.00%	
# 20	0.85	79.98%	
# 50	0.3	54.39%	SAND
# 100	0.15	38.73%	J. J.
# 140	0.106	30.26%	
# 270	0.053	19.19%	

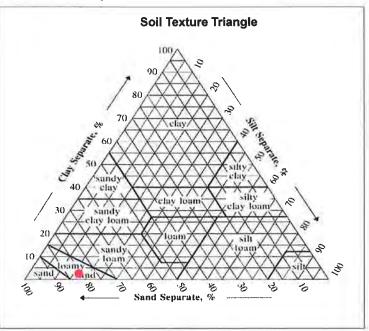
Hydrometer Analysis of Material Passing #270 Sleve

		WE10 01040
	17.68%	0.0396
	14.88%	0.0299
	11.78%	0.0226
	8.84%	0.0159
SILT	7.13%	0.0127
	5.27%	0.0092
	4.03%	0.0067
	2.79%	0.0045
1	2.48%	0.0034
CLAY	2.32%	0.0028
TULAY	1.86%	0.0014

Triangle Classification of Material Passing the #10 Sleve

Fax: 508-947-8873

% Sand 80.81% % Silt 16.87% % Clay 2.32%



USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1, Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



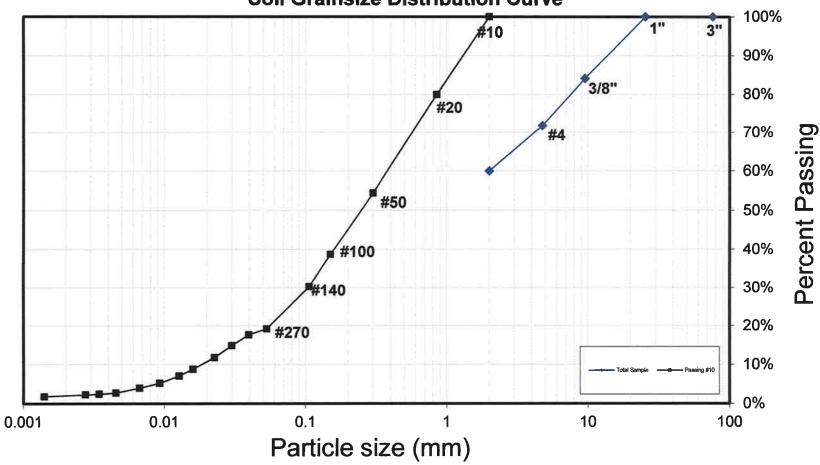
Tel: 508-946-9231 Fax: 508-947-8873 JOB #: OEL-1048-H-Pit 37

LOCATION: Pit 37 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/24/2019







Tel: 508-946-9231

www.outback-eng.com

Fax: 508-947-8873

DATE: 10/10/2019 TEST BY: M.S.

OEL-1048-H-Pit 38 LOCATION: Pit 38 - Joanna Rd. Avon, MA
CLIENT: Stonebridge Homes

JOB #:

CHECK BY: J.A.Y.

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

ve Analysis of Total Sample

Sieve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	96.54%
3/8"	9.5	79.29%
#4	4.75	69.92%
# 10	2	58.81%

Sleve Analysis of Material Passing #10

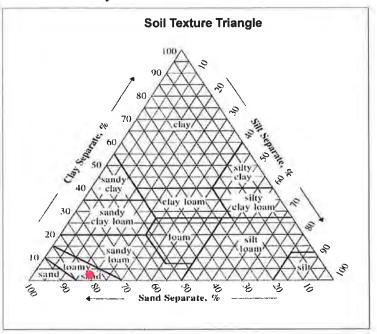
	216/6		
	Percent Passing (%):	Particle Diameter (mm):	Sleve_No
	100.00%	2	# 10
	78.73%	0.85	# 20
SAND	50.97%	0.3	# 50
O/NIVO	36.36%	0.15	# 100
	29.32%	0.106	# 140
	20.91%	0.053	# 270

Hydrometer Analysis of Material Passing #270 Sieve

	19.45%	0.03931
	16.10%	0.02994
	14.08%	0.02203
	10.56%	0.01659
SILT	8.72%	0.01249
	6.37%	0.00915
	5.20%	0.00658
	4.02%	0.00494
	3.02%	0.00295
CLAY	2.35%	0.00268
	2.01%	0.00142

Triangle Classification of Material

Passing the #10 Sleve		
% Sand	79.09%	
% Silt	18.56%	
% Clay	2.35%	



USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1, Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



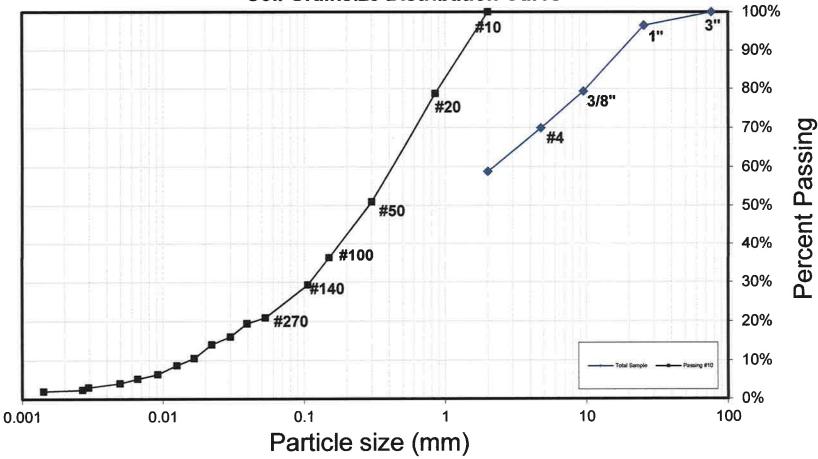
165 East Grove Stree Tel: 508-946-9231 Middleboro, MA 0234 Fax: 508-947-8873 JOB #: OEL-1048-H-Pit 38

LOCATION: Pit 38 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/10/2019







Tel: 508-946-9231

www.outback-eng.com

Fax: 508-947-8873

JOB #: OEL-1048-H-Pit 39 LOCATION: Pit 39 - Joanna Rd. Avon, MA

CLIENT:

Stonebridge Homes

DATE: 10/21/2019

TEST BY: M.S.

CHECK BY: J.A.Y.

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sieve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	100.00%
3/8"	9.5	85.43%
#4	4.75	80.32%
# 10	2	73.20%

Sieve Analysis of Material Passing #10

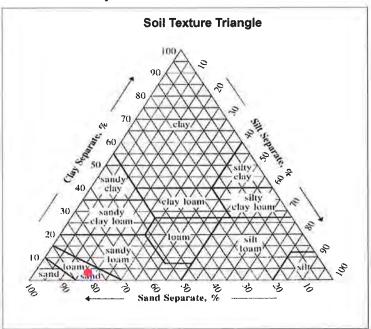
	21848		
	Percent Passing (%):	Particle Diameter (mm):	Sieve_No
	100.00%	2	# 10
	88.72%	0.85	# 20
SAND	66.92%	0.3	# 50
UANT	45.79%	0.15	# 100
	34.63%	0.106	# 140
	20.84%	0.053	# 270

Hydrometer Analysis of Material Passing #270 Sieve

	18.89%	0.03931
1	16.45%	0.02942
	13.52%	0.02211
	10.58%	0.01650
SILT	8.63%	0.01248
	6.67%	0.00908
	5.21%	0.00657
	4.23%	0.00485
	3.74%	0.00354
CLAY	3.09%	0.00276
	2.60%	0.00139

Triangle Classification of Material 0 Sleve

Pass	ing the #1
% Sand	79.16%
% SIIt	17.75%
% Clay	3.09%



USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1. Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



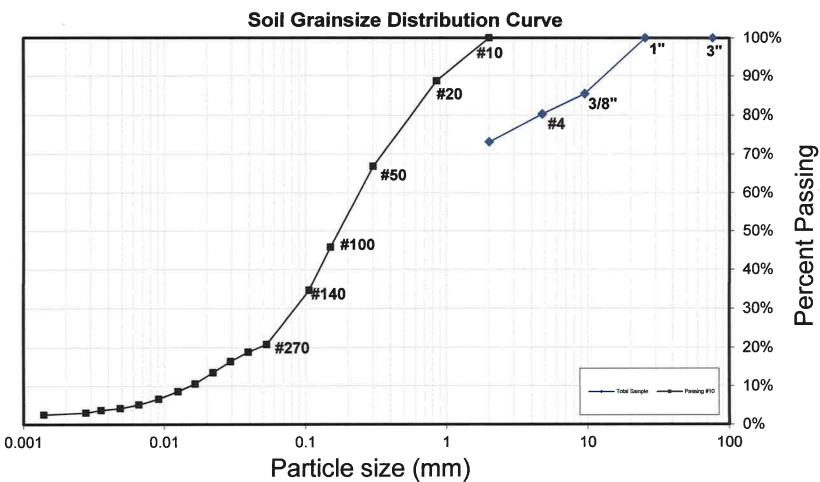
Tel: 508-946-9231

Middleboro, MA 0234 Fax: 508-947-8873 JOB #: OEL-1048-H-Pit 39

LOCATION: Pit 39 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/21/2019





Tel: 508-946-9231

www.outback-eng.com

Fax: 508-947-8873

JOB #: OEL-1048-H-Pit 40

LOCATION: Pit 40 - Joanna Rd. Avon, MA **CLIENT:** Stonebridge Homes

DATE: 10/17/2019 TEST BY: M.S. CHECK BY: J.A.Y.

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sieve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	100.00%
3/8"	9.5	79.80%
#4	4.75	68.76%
# 10	2	55.99%

Sieve Analysis of Material Passing #10 Sleve

Sleve_No	Particle Diameter (mm):	Percent Passing (%):	
# 10	2	100.00%	
# 20	0.85	77.79%	
# 50	0.3	54.79%	SAND
# 100	0.15	41.04%	
# 140	0.106	34.63%	
# 270	0.053	25.08%	

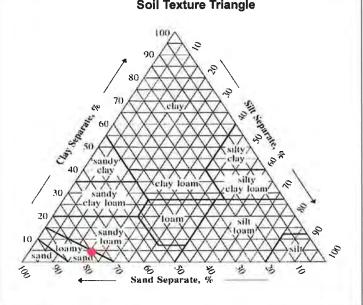
Hydrometer Analysis of Material Passing #270 Sleve

		1121001010
	25.78%	0.05284
	24.17%	0.03867
	20.94%	0.02910
	17.12%	0.02196
SILT	13.89%	0.01631
	12.08%	0.01222
	9.66%	0.00892
	7.25%	0.00699
	4.83%	0.00393
CLAY	4.23%	0.00321
	3.22%	0.00142

Triangle Classification of Material Passing the #10 Sieve

% Sand 74.92% % SIIt 20.85% % Clay 4.23%

Soil Texture Triangle



USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1. Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



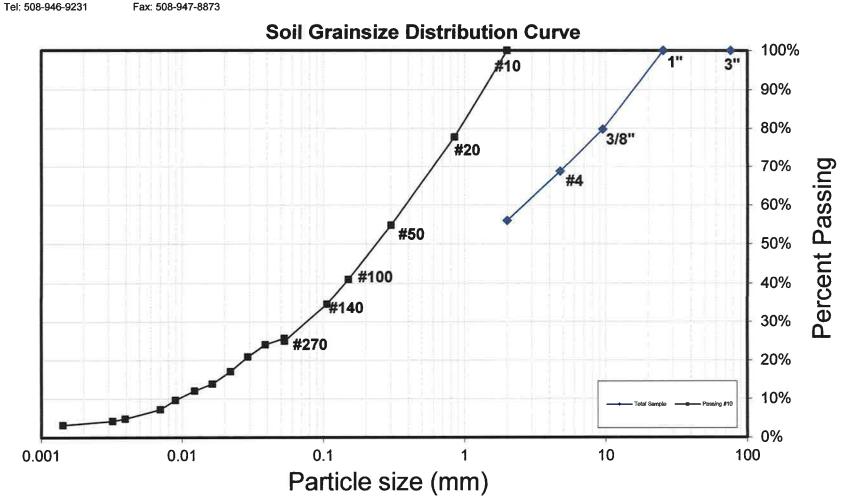
Fax: 508-947-8873

JOB #: OEL-1048-H-Pit 40

LOCATION: Pit 40 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/17/2019





Tel: 508-946-9231

www.outback-eng.com

DATE: 10/18/2019

TEST BY: M.S.
CHECK BY: J.A.Y.

Fax: 608-947-8873

JOB#: OEL-1048-H-Pit 41
LOCATION: Pit 41 - Joanna Rd. Avon, MA
CLIENT: Stonebridge Homes

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sleve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	100.00%
3/8"	9.5	81.54%
# 4	4.75	75.37%
# 10	2	68.04%

Sleve Analysis of Material Passing #10 Sieve

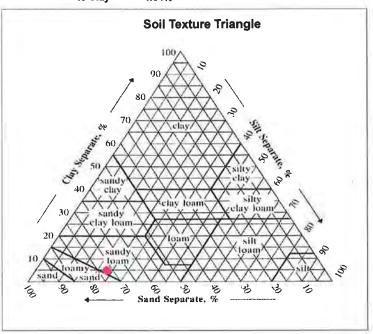
Sleve_No	Particle Diameter (mm):	Percent Passing (%):	
# 10	2	100.00%	
# 20	0.85	85.91%	
# 50	0.3	64.11%	SAND
# 100	0.15	48.61%	
# 140	0.106	39.70%	
# 270	0.053	27.98%	

Hydrometer Analysis of Material Passing #270 Sieve

		#2/0 3/9/9	1
	26.56%	0.04	
SILT	23.46%	0.03	
	20.14%	0.02	
	16.60%	0.02	
	54.44%	0.00	
	10.84%	0.01	
	8.41%	0.01	
	7.30%	0.00	
	5.31%	0.00	
CLAY	4.64%	0.00	
	3.76%	0.00	15.

Triangle Classification of Material Passing the #10 Sieve

% Sand 72.02% % Silt 23.34% % Clay 4.64%



USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1, Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades

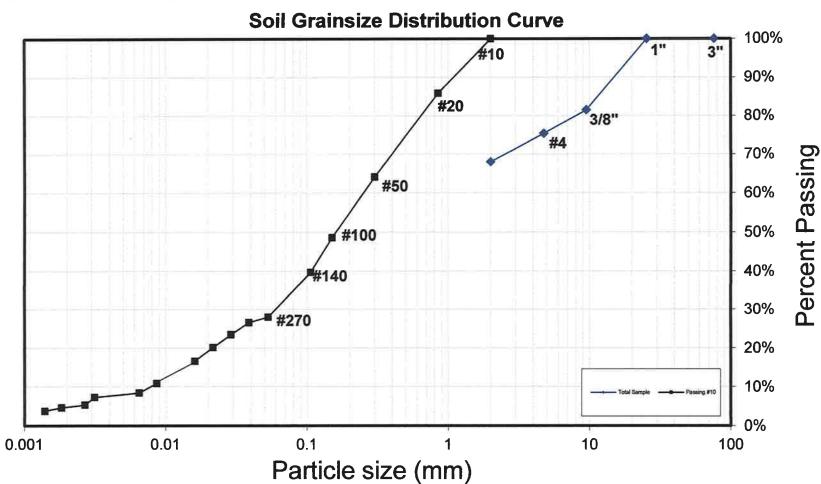


165 East Grove Street Tel: 508-946-9231 Middleboro, MA 02346 Fax: 508-947-8873 JOB #: OEL-1048-H-Pit 41

LOCATION: Pit 41 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/18/2019





Tel: 508-946-9231

OEL-1048-H-Pit 42 LOCATION: Pit 42 - Joanna Rd. Avon, MA

Stonebridge Homes

JOB #:

CLIENT:

www.outback-eng.com

<u>DATE:</u> 10/23/2019 <u>TEST BY:</u> M.S. CHECK BY: J.A.Y.

Fax: 508-947-8873

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sieve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	100.00%
3/8"	9.5	86.89%
#4	4.75	80.25%
# 10	2	72.77%

Sleve Analysis of Material Passing #10

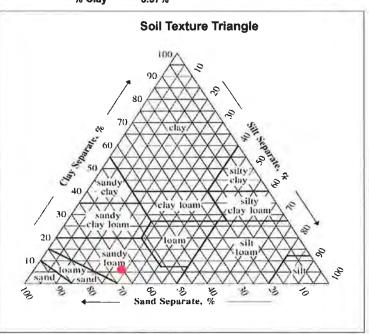
	Sieve		
	Percent Passing (%):	Particle Diameter (mm):	Sieve_No
	100.00%	2	# 10
	86.76%	0.85	# 20
SAN	66.55%	0.3	# 50
	52.21%	0.15	# 100
]	44.10%	0.106	# 140
	34.60%	0.053	# 270

Hydrometer Analysis of Material Passing #270 Slave

_		#2/0 Sieve	
	30.37%	0.0383	
	26.88%	0.0287	7
	22.90%	0.0215	
	19.42%	0.0159	1
SILT	16.93%	0.0119	
	13.19%	0.0088	
	8.96%	0.0063	
	7.71%	0.0040	
	6.97%	0.0035	1
CLAY	5.97%	0.0027	
	4.23%	0.0003	

Triangle Classification of Material Passing the #10 Sieve

rass	rassing the mit		
% Sand	65.40%		
% Slit	28.63%		
% Clay	5.97%		



USDA Soil Textural Triangle

Jason Youngguist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1. Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



165 East Grove Street Tel: 508-946-9231

0.001

Middleboro, MA 02346 Fax: 508-947-8873

0.01

0.1

Particle size (mm)

JOB #: OEL-1048-H-Pit 42

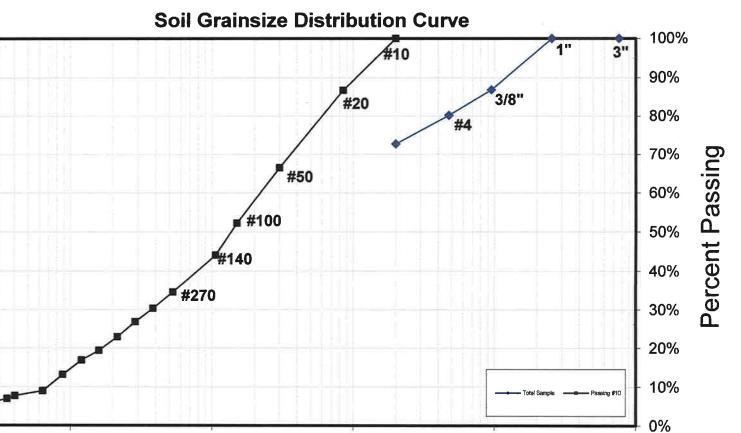
LOCATION: Pit 42 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/23/2019

100

TEST BY: M.S.
CHECK BY: J.A.Y.



10



Tel: 508-946-9231

www.outback-eng.com

Fax: 508-947-8873

JOB #: OEL-1048-H-Pit 43

LOCATION: Pit 43 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/18/2019 **TEST BY:** M.S.

TEST BY: M.S. CHECK BY: J.A.Y.

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sleve_No	Particle Diameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	100.00%
3/8"	9.5	87.59%
#4	4.75	81,28%
# 10	2	74.55%

Sieve Analysis of Material Passing #10

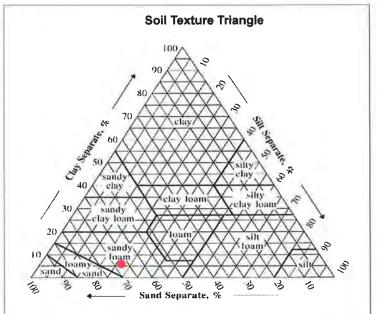
Particle	Percent	1
Diameter (mm):	Passing (%):	
2	100.00%	
0.85	90.39%	
0.3	71.16%	SANI
0.15	53.14%	JAN.
0.106	44.01%	
0.053	32.70%	
	2 0.85 0.3 0.15 0.106	2 100.00% 0.85 90.39% 0.3 71.16% 0.15 53.14% 0.106 44.01%

Hydrometer Analysis of Material Passing #270 Sieve

		#270 Sieve	
	31.65%	0.0377	
	28.39%	0.0281	
	24.62%	0.0210	
]	20.60%	0.0157	
SILT	17.84%	0.0118	
	14.82%	0.0087	
	12.06%	0.0068	
	9.29%	0.0050	11
	6.53%	0.0031	
CLAY	5.77%	0.0027	
	4.02%	0.0014	

Triangle Classification of Material

Passing the #10 Sieve		
% Sand	67.30%	
% SIIt	26.93%	
% Clay	5.77%	



USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1. Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



165 East Grove Street Tel: 508-946-9231

Fax: 508-947-8873

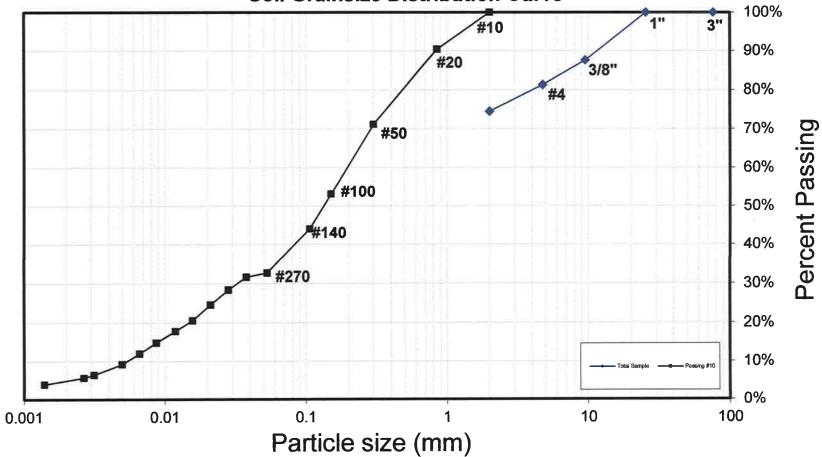
JOB #: OEL-1048-H-Pit 43

LOCATION: Pit 43 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/18/2019







Tel: 508-946-9231

OEL-1048-H-Pit 44 LOCATION: Pit 44 - Joanna Rd. Avon, MA

Stonebridge Homes

JOB #:

CLIENT:

www.outback-eng.com

Fax: 608-947-8873

DATE: 10/22/2019

TEST BY: M.S. CHECK BY: J.A.Y.

Title V Particle Size Analysis (Combined Sieve & Hydrometer) Report

Sieve Analysis of Total Sample

Sieve_No	Particle Dlameter (mm):	Percent Passing (%):
3"	76.2	100.00%
1"	25.4	100.00%
3/8"	9.5	91.30%
#4	4.75	85.02%
# 10	2	77.21%

Sleve Analysis of Material Passing #10 Sieve

Sieve_No	Particle Diameter (mm):	Percent Passing (%):	
# 10	2	100.00%	
# 20	0.85	86.19%	
# 50	0.3	65.90%	SAND
# 100	0.15	49.42%	l on it
# 140	0.106	41.47%	
# 270	0.053	29.11%	

Hydrometer Analysis of Material Passing #270 Sieve

	29.60%	0.05331
	28.22%	0.03865
	25.37%	0.02867
	21.61%	0.02146
SILT	17.38%	0.01607
	14.09%	0.01222
	11.27%	0.00892
	8.92%	0.00647
	7.05%	0.00475
CLAY	4.93%	0.00275
TULKI	3.99%	0.00139

Triangle Classification of Material Passing the #10 Sieve % Sand 70.89% % SIIt 24.18%

% Clay 4.93%

	Soil Texture Triangle
	199
	A 20
	90
	80
	(XXXX) ·= '
	ou 70 chay
	60 X X X X X X X X X X X X X X X X X X X
ف	S. (XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
19	sundy clay
40	clay XXXXXXXXXX
30 /	sandy X X X X Clay loam
\wedge	clay loam
20	A A A A North A A Sill A A A
10	\\ \sandy \\ \\ \loum \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\
sand toan	3 × × × × × × × × × × × × × × × × × × ×
19 B	8 8 8 8 8 8 6
70	8 8 8 8 8 8 6 6 4 8 6 6 € Sand Separate, %

USDA Soil Textural Triangle

Jason Youngquist, P.E. Laboratory Supervisor

Particle Size Analysis Methodology taken from Gee and Bauder (1986) Methods of Soil Analysis, Part 1. Physical and Mineralogical Methods, 2nd Edition as specified in Appendix 2 of Mass DEP Policy #:BRP/DWM/PeP-P00-4; Title 5 Alternative to Percolation Testing Policy for System Upgrades



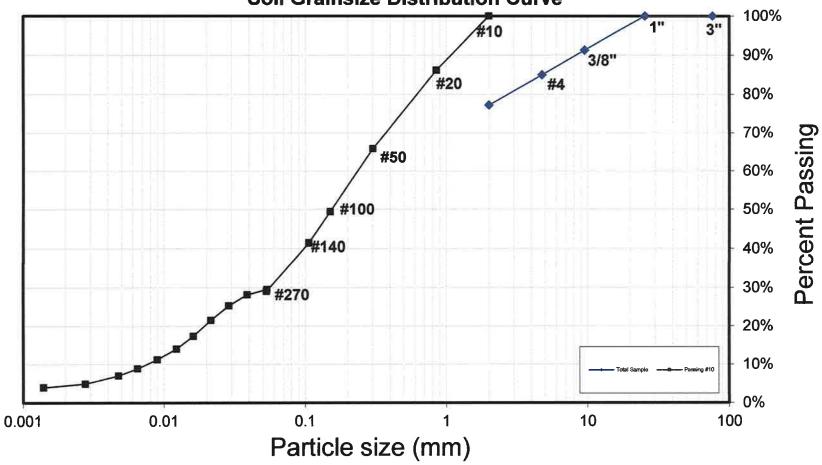
JOB #: OEL-1048-H-Pit 44

LOCATION: Pit 44 - Joanna Rd. Avon, MA

CLIENT: Stonebridge Homes

DATE: 10/22/2019





APPENDIX K: RIPRAP SIZING CALCULATIONS

Riprap Sizing Spreadsheet

Project No. 2019-002

Project: Joanna Hills Estates **Location:** Joanna Road, Avon, MA

Date: March 6, 2020

Riprap is used for erosion control, to prevent scour, and to minimize sediment transport into wetland resource areas. A stable riprap rock size is desired. The Isbash equation computes the smallest diameter stone, D, having a specific gravity, S, that if dropped in water flowing at velocity, V, will settle and remain stationary.

$D = V^2 / (2gC^2(S-1))$

Where:

<u>D</u>: Median diameter of spherical stone or rock. Also known as D₅₀. (Meters)

C: Isbash Constant. C=0.86 for highly turbulent conditions or 1.2 for low turbulence conditions.

<u>a</u>: Acceleration due to gravity, 9.8066 m/s²

<u>S</u>: Specific gravity of the riprap. Typically varies from 2.56 to 2.92 depending on the rock material. A commonly used value is 2.65.

 $\underline{\mathbf{V}}$: Water velocity approaching the riprap (m/s). This is calculated using the 100-yr peak flow rate for the basin and dividing it by the area of the spillway at the peak elevation of the 100-yr storm.

Location	С	g (m/s²)	S	Q ₁₀₀ (cfs)	A ₁₀₀ (ft ²)	V (ft/s)	V (m/s)	D (m)	D (Inches)
Outlet Basin A	0.86	9.81	2.65	0.07	0.25	0.28	0.09	0.00	0.01
Outlet Basin B	0.86	9.81	2.65	4.58	4.25	1.08	0.33	0.00	0.18
Outlet Basin C	0.86	9.81	2.65	4.19	4	1.05	0.32	0.00	0.17
Outlet Basin D	0.86	9.81	2.65	1.38	2	0.69	0.21	0.00	0.07
Outlet Basin E	0.86	9.81	2.65	4.27	4	1.07	0.33	0.00	0.17

Riprap Design Values					
D	Weight	Weight			
(Inches)	(Kg)	(lbs)			
6	5	10			
9	16	35			
12	39	85			
18	125	275			
24	227	500			

MassDOT specification M2.02.0 for Riprap requires a minimum weight of 50lb (25 kg)

APPENDIX L: TURF MANAGEMENT PLAN

TURF MANAGEMENT PLAN – LOT 10

The following is a guideline detailing proper turf management techniques, maintenance schedules, and tasks to be completed to reduce the amount of pollutants entering the stormwater system from turfed areas.

FERTILIZER

<u>First Application</u>: To establish final turf at the end of construction activities, phosphate-free organic nitrogen slow-release fertilizer should be used. Fertilizers should not be used within 100 feet of wetlands.

<u>Semi-Annual Maintenance (Spring and Fall)</u>: Inspect turf for any bare soil or thin grass areas. Slow-release fertilizer should be applied where bare spots or thin areas are present to assist with the reestablishment of the turf. All fertilizer shall be kept in a wrapped or sealed container and stored at least 100 feet from the wetland at all times.

SEEDING

<u>First Application</u>: To establish final turf at the end of construction activities, a seed mix of 65% fine Fescue, 20% perennial ryegrass, and 15% Kentucky Bluegrass should be applied at a rate of 4-6 LBS/1000FT². Seeding should take place in late summer.

<u>Semi-Annual Maintenance (Spring and Fall)</u>: Inspect turf for any bare soil or thin grass areas. Where bare spots or thin areas are present, over-seed the area(s) with the grass seed mix to allow for turf reestablishment.

MOWING

General Maintenance: Turf should be mowed as needed and shall be cut to a minimum of 3 inches in height. If at the time of mowing the grass clippings are less than 1 inch in height, grass clippings can be left on the turf to serve as a natural fertilizer. If grass clippings are collected, they shall be removed from the property and disposed of properly.

WATERING

<u>Unscheduled Maintenance</u>: An irrigation system shall be installed to ensure proper watering of the turf on site. The system should be equipped with a rain gauge to prevent the system from running after a recent rain storm.

PEST CONTROL

See the attached Integrated Pest Management Guide for guidelines on lawn care pest control.